

LOCATION MAP
N.T.S.

LAND DEVELOPMENT PLANS FOR PLANNING AND ZONING SPECIAL PERMIT APPLICATION

PROPOSED RETAIL DEVELOPMENT

1100 BOSTON TURNPIKE
BOLTON, CONNECTICUT

PREPARED FOR:
GARRETT HOMES, LLC
59 FIELD STREET
TORRINGTON, CT 06790



VICINITY MAP
SCALE: 1"=800'

CONTENTS

	TITLE SHEET
	ALTA/NSPS LAND TITLE SURVEY (BY OTHERS)
	2-LOT SUBDIVISION PLAN (BY OTHERS)
MP-1	MASTER PLAN
GN-1	GENERAL NOTES
DM-1	DEMOLITION PLAN
SP-1	SITE PLAN
TT-1	TRUCK TURNING PLAN - WB-67
GD-1	GRADING AND DRAINAGE PLAN
SU-1	SITE UTILITIES PLAN
EC-1	SEDIMENT AND EROSION CONTROL PLAN
EC-2	SEDIMENT AND EROSION CONTROL NOTES
LL-1	LANDSCAPING PLAN
LL-2	LANDSCAPING DETAILS AND NOTES
SD-1	INTERSECTION SITE DISTANCE PLAN
DN-1 TO DN-6	DETAILS SHEET
	SITE LIGHTING PHOTOMETRICS PLAN (BY OTHERS)
	ARCHITECTURAL CONCEPTUAL ELEVATIONS (BY OTHERS)
	ARCHITECTURAL CONCEPTUAL FLOOR PLAN (BY OTHERS)

PREPARED BY:



100 CONSTITUTION PLAZA, 10TH FLOOR
HARTFORD, CONNECTICUT 06103
(860) 249-2200
(860) 249-2400 Fax

FOR PERMITTING PURPOSES ONLY
NOT RELEASED FOR CONSTRUCTION

BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT

DATE APPROVED _____ DATE OF EXPIRATION _____

CHAIRMAN _____

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON _____

DEVELOPER: GARRETT HOMES, LLC 59 FIELD STREET TORRINGTON, CT 06790	OWNER: 1100 BOSTON TRUNPIKE LLC C/O JOEL ROSENLICHT 483 MIDDLE TURNPIKE WEST, SUITE 102 MANCHESTER, CT 06040
---	--



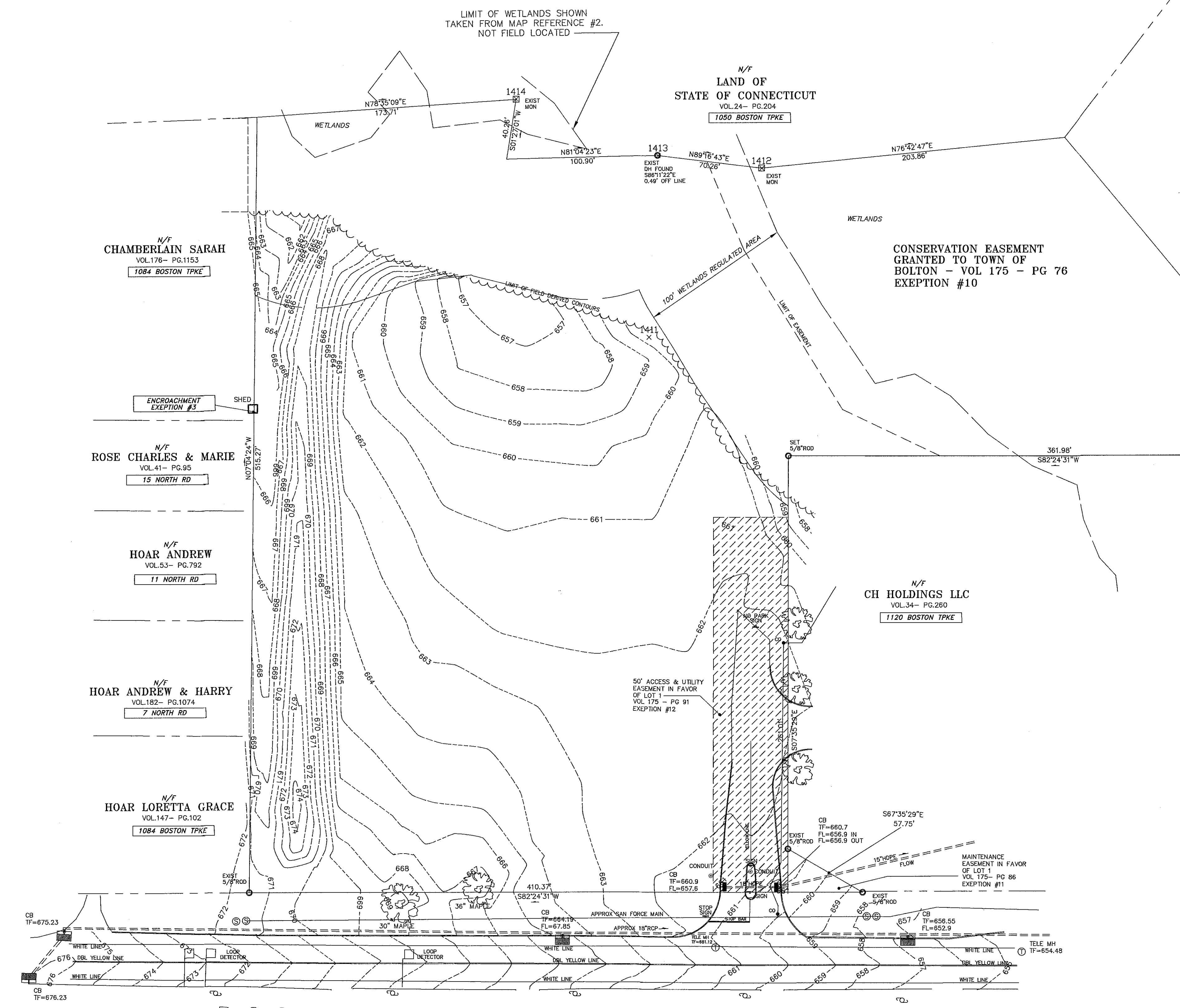
DATES

ISSUE DATE:	APRIL 2, 2021	
REVISION:	MAY 5, 2021	(REVISED PER TOWN COMMENTS)

LEGEND

- STORM SEWER
- CONTOUR LINE
- FOLIAGE LINE
- FENCE
- IRON PIN
- CURB CATCHBASIN
- UTILITY POLE
- TELEPHONE MANHOLE
- SANITARY MANHOLE
- CONDUIT
- SIGN

LEGAL DESCRIPTION - 1100 BOSTON TURNPIKE:
 COMMENCING AT AN EXISTING 5/8" ROD IN THE NORTHERLY HIGHWAY LINE OF RT 44 ALSO KNOWN AS BOSTON TURNPIKE, MARKING THE SOUTHEAST CORNER OF THE PROPERTY BEING DESCRIBED HEREIN AND THE SOUTHWEST CORNER OF LAND NOW OR FORMALLY OF CH HOLDINGS LLC; THENCE ALONG THE NORTHERLY HIGHWAY LINE OF SAID BOSTON TURNPIKE S82°24'31"W, 410.30' TO AN EXISTING 5/8" ROD MARKING THE SOUTHEASTERN CORNER OF LAND NOW OR FORMERLY OF LORETTA GRACE HOAR AND THE SOUTHWESTERLY CORNER OF LAND BEING DESCRIBED HEREIN; THENCE ALONG LANDS OF HARRY AND ANDREW HOAR, ANDREW HOAR, MARIE AND CHARLES ROSE, AND SARAH CHAMBERLIN PARTLY BY EACH N07°04'24"W, 515.27' TO A POINT MARKING NORTHEAST CORNER OF LAND NOW OR FORMERLY OF SARAH CHAMBERLAIN AND THE NORTHWEST CORNER OF LAND BEING DESCRIBED HEREIN; THENCE ALONG THE SOUTHERLY LINE OF LAND NOW OR FORMERLY OF STATE OF CONNECTICUT THE FOLLOWING FIVE COURSES:
 N78°35'09"E, 173.71, S01°27'01"W, 40.26', N81°04'23"E, 100.90', N89°16'43"E, 70.26', N76°42'47"E, 203.86, TO A POINT MARKING THE NORTHWEST CORNER OF LAND NOW OR FORMERLY OF MISSIONARY SOCIETY FOR THE DIOCESE AND THE NORTHEAST CORNER OF LAND BEING DESCRIBED HEREIN; THENCE ALONG LAND OF SAID MISSIONARY FOR THE DIOCESE OF CONNECTICUT S47°34'01"E, 275.82', TO A POINT MARKING THE NORTHEAST CORNER OF SAID CH HOLDING LLC; THENCE ALONG SAID CH HOLDINGS LLC S82°24'31"W, 361.98', TO A POINT MARKING THE NORTHWEST CORNER OF SAID CH HOLDING LLC, THENCE ALONG SAID CH HOLDING LLC S07°35'29"E, 261.01', TO AN EXISTING 5/8" ROD; THENCE CONTINUING ALONG CH HOLDING LLC S67°35'29"E, 57.75', TO THE POINT AND PLACE OF BEGINNING CONTAINING 236,912.34 OR 5.44 ACRES.



N/F
 THE MISSIONARY SOCIETY FOR THE
 DIOCESE OF CONNECTICUT
 VOL.34 - PG.260
 1150 BOSTON TPKE

NOTES:

1. THIS SURVEY HAS BEEN PREPARED PURSUANT TO THE REGULATIONS OF CONNECTICUT STATE AGENCIES SECTIONS 20-300b-20 AND THE "STANDARDS FOR SURVEYS AND MAPS IN THE STATE OF CONNECTICUT" AS ADOPTED BY THE CONNECTICUT ASSOCIATION OF LAND SURVEYORS, INCORPORATED ON SEPTEMBER 26, 1996.
2. TYPE OF SURVEY = PROPERTY SURVEY
3. BOUNDARY DETERMINATION CATEGORY = DEPENDENT RE-SURVEY
4. OWNERS OF RECORD - 1100 BOSTON TURNPIKE LLC (Vol. 141- Pg 790)
5. TOTAL AREA - 236912 S.F. OR 5.439 Ac.
6. ZONE - RMUZ
7. ELEVATIONS BASED ON NAVD 88 DATUM (MAP REFERENCE #2)
8. NO EVIDENCE OF RECENT EARTH MOVING WORK, BUILDING CONSTRUCTION OR BUILDING ADDITIONS OBSERVED.
9. NO INFORMATION OF PROPOSED CHANGES IN STREET RIGHT OF WAY LINES HAS BEEN MADE AVAILABLE TO SURVEYOR. NO EVIDENCE OF RECENT STREET OR SIDEWALK CONSTRUCTION OR REPAIRS OBSERVED.
10. NO WETLAND DELINEATION OBSERVED
11. PROPERTY LIES WITHIN FLOOD ZONE X - AREA OF MINIMAL FLOOD HAZARD, AS SHOWN ON MAP ENTITLED "FIRM - FLOOD INSURANCE RATE MAP - TOWN OF BOLTON, CONNECTICUT- TOLLAND COUNTY - PANEL 1 OF 3 COMMUNITY PANEL NUMBER 090109 0001 B WITH AN EFFECTIVE DATE OF JUNE 1 1981 AND PREPARED BY FEDERAL EMERGENCY MANAGEMENT AGENCY.
12. REFER TO VOL 72 - PG 443 FOR POSSIBLE EFFECTS OF SPECIAL PERMIT GRANTED ON APRIL 4, 1990 - EXEPTION #9
13. UNDERGROUND UTILITIES, STRUCTURE AND FACILITY LOCATIONS DEPICTED AND NOTED HEREON HAVE BEEN COMPILED, IN PART, FROM RECORD MAPPING SUPPLIED BY THE RESPECTIVE UTILITY COMPANIES OR GOVERNMENTAL AGENCIES, FROM PAROLE TESTIMONY AND FROM OTHER SOURCES. THESE LOCATIONS ARE APPROXIMATE AND OTHER SUCH FEATURES MAY EXIST UNKNOWN TO DUFOR SURVEYING AND OTHER SUCH FEATURES LOCATION AND EXISTENCE OF ALL SUCH FEATURES MUST BE FIELD DETERMINED AND VERIFIED BY THE APPROPRIATE AUTHORITIES PRIOR TO CONSTRUCTION. CALL BEFORE YOU DIG 1-800-922-4455

FIRST AMERICAN TITLE INSURANCE COMPANY - COMMITMENT FOR TITLE INSURANCE : COMMITMENT No. CT5142976 - DATED JANUARY 8, 2021 CERTIFIED TO: CALITO DEVELOPMENT LLC FIRST AMERICAN TITLE INSURANCE COMPANY

THAT THIS MAP OR PLAT AND THE SURVEY ON WHICH IT IS BASED WERE MADE IN ACCORDANCE WITH THE 2016 MINIMUM STANDARD DETAIL REQUIREMENTS FOR ALTA/ACSM LAND TITLE SURVEYS, JOINTLY ESTABLISHED AND ADOPTED BY ALTA AND NSPS, AND INCLUDES TABLE A ITEMS 2,3,4,5,8,11,13,16,17,18 AND 19.

SCHEDULE B, PART II, EXCEPTIONS:
 1. NON-SURVEY ISSUE
 2. NON SURVEY ISSUE
 3. PLOTTED
 4. NON SURVEY ISSUE
 5. NON SURVEY ISSUE
 6. NON SURVEY ISSUE
 7. NON SURVEY ISSUE
 8. NON SURVEY ISSUE
 9. PLOTTED - NOTE 12
 10. PLOTTED
 11. PLOTTED
 12. PLOTTED

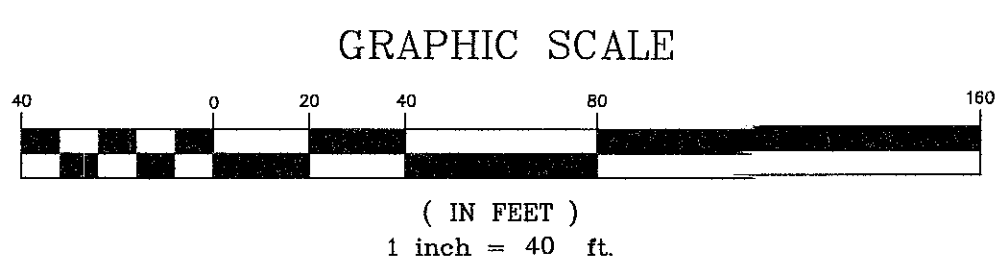
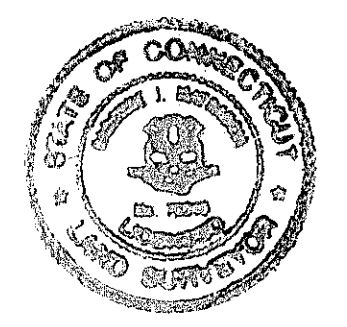
FIELD WORK WAS COMPLETED ON FEBRUARY 17, 2021

DATE: 3/31/2021
 CARMINE J. MATRASCIA - LS#70219

MAP REFERENCES :

1. "LOT SPLIT PLAN & EASEMENT PLAN - PREPARED FOR - 1100 BOSTON TURNPIKE, LLC - 1100 BOSTON TURNPIKE - BOLTON, CT - MAP 05 LOT 81 - ZONE: RMUZ", scale 1" = 40', dated 7-18-17, revised to 9-28-17 and prepared by J R Russo & Associates LLC, Surveyors - Engineers.
2. "AS-BUILT PLAN - BOLTON COMETIC & FAMILY DENTISTRY - 1120 BOSTON TURNPIKE - BOLTON, CT - MAP 05 LOT 81-1 ZONE: RMUZ", scale 1" = 20', dated 7-24-18 and prepared by JR Russo & Associates, LLD Surveyors - Engineers
3. "CONNECTICUT - STATE HIGHWAY DEPARTMENT - RIGHT OF WAY MAP - TOWN OF BOLTON - HARTFORD-WILLMANTIC ROAD - FROM THE COVENTRY TOWN LINE - WESTERLY ABOUT 6,300 FEET - ROUTES U.S. 6 & U.S. 44", scale 1" = 40', dated Oct. 31, 1935 and prepared by Connecticut State Highway Department
4. "SOME LAND - OF THE ESTATE OF - ALBERT N. SKINNER - TOWN OF BOLTON CONN." - scale 1"=50', dated Nov. 20, 1968 and prepared by Everett O. Gardner L.S 4349

BOSTON TURNPIKE (RTE. 44)



TO MY KNOWLEDGE AND BELIEF, THIS MAP IS SUBSTANTIALLY CORRECT AS NOTED HEREON. THIS MAP AND SURVEY WERE PREPARED IN ACCORDANCE WITH THE STANDARDS OF A CLASS A-2 & T-2 SURVEY AS DEFINED IN THE CODE OF PRACTICE FOR STANDARDS OF ACCURACY OF SURVEYS AND MAPS, ADOPTED SEPT. 26, 1996 AS AMENDED BY THE CONNECTICUT ASSOCIATION OF LAND SURVEYORS, INCORPORATED.

Carmine J. Matrascia
 CARMINE J. MATRASCIA - L.S. #70219
 NOT VALID WITHOUT EMBOSSED SEAL



ALTA/NSPS LAND TITLE SURVEY		
PREPARED FOR: CALITO DEVELOPMENT, LLC		
1100 BOSTON TPKE, RTE 44, BOLTON, CT		
SCALE: 1" = 40'	APPROVED: CARMINE J. MATRASCIA - L.S. #70219	
DATE: 02-18-2021	JOB NO.: 21-05	FILE NO.: 21-05
DUFOR SURVEYING LLC 575 NORTH MAIN STREET BRISTOL, CONNECTICUT 860-314-0502 860-738-0222		

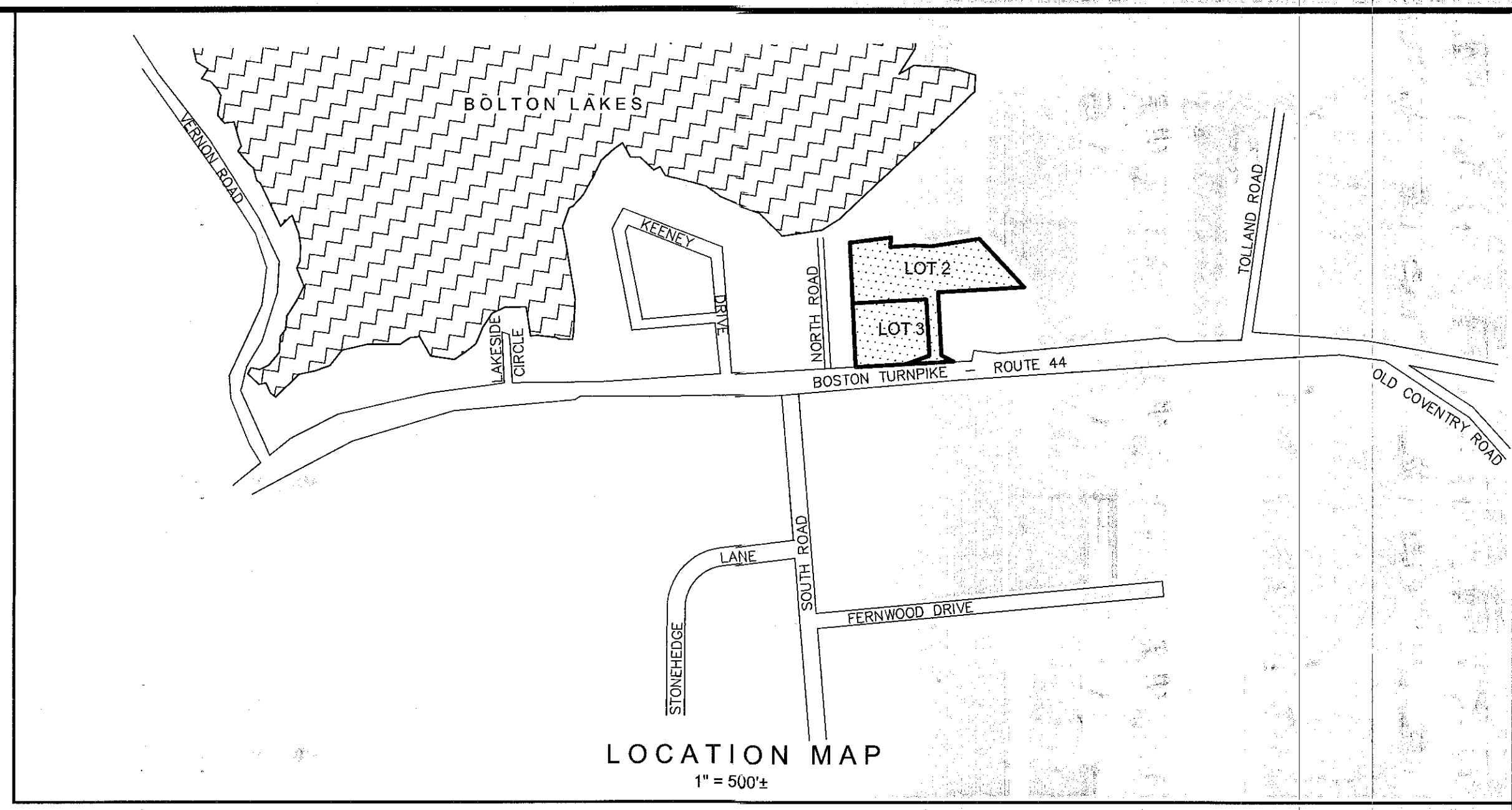
REVISED 3/31/2021 : ADD WETLANDS LIMITS
 REVISED 3/22/2021 : CORRECTED ZONE RMUZ

LEGEND

- STONE WALL
- SANITARY SEWER
- ELECTRIC LINE
- PROPANE LINE
- FENCE
- STORM SEWER
- CONTOUR LINE
- FOLIAGE LINE
- IRON PIN
- SURVEY MONUMENT
- DRILL HOLE
- CURB CATCHBASIN
- CURBLESS CATCHBASIN
- UTILITY POLE
- UTILITY POLE WITH GUY
- WATER GATE VALVE
- GAS GATE VALVE
- HYDRANT

ZONING REQUIREMENTS

ZONE - RMUZ
MINIMUM LOT AREA = 80,000 S.F.
MINIMUM LOT WIDTH = 150'
MINIMUM SETBACKS -
FRONT = 25'
SIDE = 25' (50' WHEN ABUTTING RESIDENTIAL ZONE)
REAR = 25' (50' WHEN ABUTTING RESIDENTIAL ZONE)
MINIMUM LANDSCAPE AREA = 30%
MAXIMUM BUILDING HEIGHT = 35' or 2.5 STORIES
MINIMUM FLOOR AREA - 600 S.F.(GROUND FLOOR)
MAXIMUM LOT COVERAGE = 25%
MAXIMUM IMPERVIOUS SURFACE = 50%



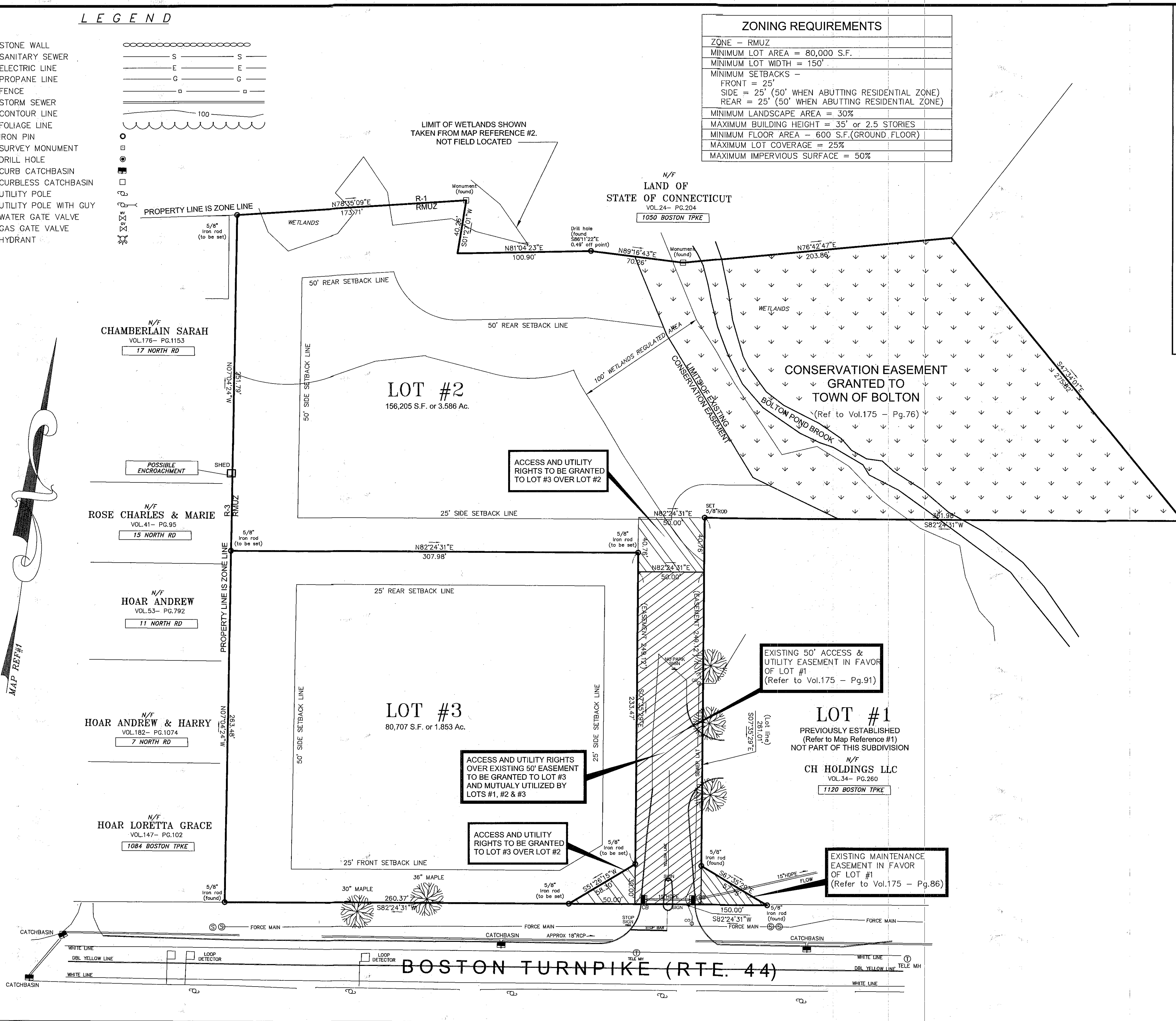
MAP REFERENCES:

- LOT SPLIT PLAN & EASEMENT PLAN - PREPARED FOR - 1100 BOSTON TURNPIKE, LLC - 1100 BOSTON TURNPIKE - BOLTON, CT - MAP 05 LOT 81 - ZONE: RMUZ, scale 1" = 40', dated 7-18-17, revised to 9-28-17 and prepared by J R Russo & Associates LLC, Surveyors - Engineers.
- AS-BUILT PLAN - BOLTON COMETIC & FAMILY DENTISTRY - 1120 BOSTON TURNPIKE - BOLTON, CT - MAP 05 LOT 81-1 ZONE: RMUZ, scale 1" = 20', dated 7-24-18 and prepared by JR Russo & Associates, LLD Surveyors - Engineers
- CONNECTICUT - STATE HIGHWAY DEPARTMENT - RIGHT OF WAY MAP - TOWN OF BOLTON - HARTFORD-WILLIMANTIC ROAD - FROM THE COVENTRY TOWN LINE - WESTERLY ABOUT 6,300 FEET - ROUTES U.S. 6 & U.S. 44", scale 1" = 40', dated Oct. 31, 1935 and prepared by Connecticut State Highway Department
- SOME LAND - OF THE ESTATE OF - ALBERT N. SKINNER - TOWN OF BOLTON CONN. - scale 1"=50', dated Nov. 20, 1968 and prepared by Everett O. Gardner L.S 4349

THE MISSINOARY SOCIETY FOR THE DIOCESE OF CONNECTICUT
VOL.34 - PG.260
1150 BOSTON TPKE

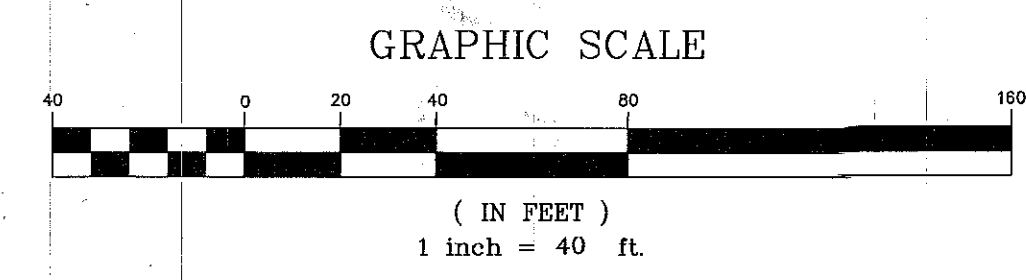
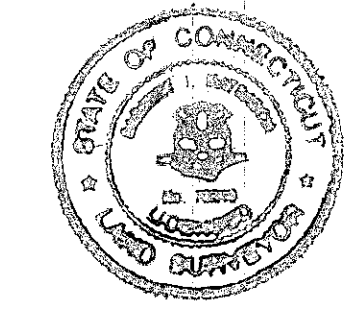
NOTES:

- THIS SURVEY HAS BEEN PREPARED PURSUANT TO THE REGULATIONS OF CONNECTICUT STATE AGENCIES SECTIONS 20-300b-20 AND THE "STANDARDS FOR SURVEYS AND MAPS IN THE STATE OF CONNECTICUT" AS ADOPTED BY THE CONNECTICUT ASSOCIATION OF LAND SURVEYORS, INCORPORATED ON SEPTEMBER 28, 1996.
- TYPE OF SURVEY = PROPERTY SURVEY
- BOUNDARY DETERMINATION CATEGORY = DEPENDENT RE-SURVEY
- OWNERS OF RECORD - 1100 BOSTON TURNPIKE LLC (Vol. 141- Pg 790)
- TOTAL AREA - 236912 S.F. or 5.439 Ac.
- ZONE - RMUZ
- PROPERTY LIES WITHIN FLOOD ZONE X - AREA OF MINIMAL FLOOD HAZARD, AS SHOWN ON MAP ENTITLED "FIRM - FLOOD INSURANCE RATE MAP - TOWN OF BOLTON, CONNECTICUT- TOLLAND COUNTY - PANEL 1 OF 3 COMMUNITY PANEL NUMBER 090109 0001 B WITH AN EFFECTIVE DATE OF JUNE 1 1981 AND PREPARED BY FEDERAL EMERGENCY MANAGEMENT AGENCY.
- UNDERGROUND UTILITIES, STRUCTURE AND FACILITY LOCATIONS DEPICTED AND NOTED HEREON HAVE BEEN COMPILED, IN PART, FROM RECORD MAPPING SUPPLIED BY THE RESPECTIVE UTILITY COMPANIES OR GOVERNMENTAL AGENCIES, FROM PAROLE TESTIMONY AND FROM OTHER SOURCES. THESE LOCATIONS ARE APPROXIMATE AND OTHER SUCH FEATURES MAY EXIST UNKNOWN TO DUFOUR SURVEYING ASSOCIATES. THE SIZE, LOCATION AND EXISTENCE OF ALL SUCH FEATURES MUST BE FIELD DETERMINED AND VERIFIED BY THE APPROPRIATE AUTHORITIES PRIOR TO CONSTRUCTION. CALL BEFORE YOU DIG 1-800-922-4455



BOSTON TURNPIKE (RTE. 44)

- N/F JASON C. & KERI A. FULLER 1055 BOSTON TURNPIKE
- N/F NICHOLAS & JESSICA ROBINSON 1061 BOSTON TURNPIKE
- N/F ROGER A. & KATHLEEN D. RUNKIS 1065 BOSTON TURNPIKE
- N/F JOHN B. STEVENS 1069 BOSTON TURNPIKE
- N/F DOROTHY S. LARSON 1071 BOSTON TURNPIKE
- N/F FREDERICK DAVIS 1079 BOSTON TURNPIKE



ALL CONSTRUCTION OF PUBLIC FACILITIES REQUIRED FOR THIS SUBDIVISION SHALL BE COMPLETED WITHIN FIVE YEARS AFTER THE APPROVAL OF THIS SUBDIVISION PLAN AND THIS FIVE YEAR PERIOD EXPIRES ON: _____

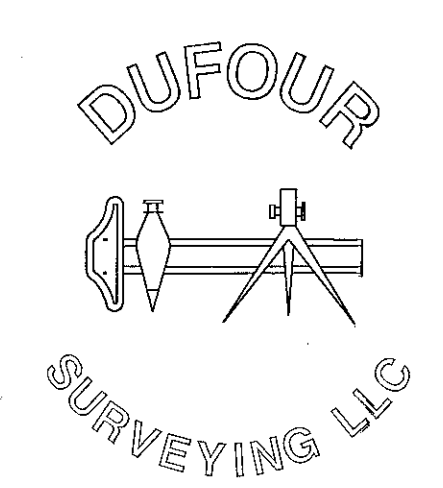
APPROVED BY THE BOLTON PLANNING COMMISSION

CHAIRMAN / SECRETARY _____ DATE: _____

REVISED 3/31/2021 : PER STAFF COMMENTS

TO MY KNOWLEDGE AND BELIEF, THIS MAP IS SUBSTANTIALLY CORRECT AS NOTED HEREON. THIS MAP AND SURVEY WERE PREPARED IN ACCORDANCE WITH THE STANDARDS OF A CLASS A-2 SURVEY AS DEFINED IN THE CODE OF PRACTICE FOR STANDARDS OF ACCURACY OF SURVEYS AND MAPS, ADOPTED SEPT. 28, 1996 AS AMENDED BY THE CONNECTICUT ASSOCIATION OF LAND SURVEYORS, INCORPORATED.

Carmine J. Matrascia
CARMINE J. MATRASCIA - L.S. #70219
NOT VALID WITHOUT EMBOSSED SEAL



2 - LOT SUBDIVISION PLAN

PREPARED FOR: CALITTO DEVELOPMENT LLC

1100 BOSTON TURNPIKE, ROUTE 44, BOLTON, CONNECTICUT

SCALE: 1" = 40'

DATE: 03-18-2021

APPROVED: CARMINE J. MATRASCIA - L.S. #70219

JOB NO.: 21-05

FILE NO.: 121-05

DUFOUR SURVEYING LLC
575 NORTH MAIN STREET
BRISTOL, CONNECTICUT
860-314-0902 860-738-0222

ZONING INFORMATION

LOCATION: BOLTON, TOLLAND COUNTY, CONNECTICUT					
ZONE: RURAL MIXED USE ZONE (RMUZ)					
USE: RETAIL (PERMITTED BY SPECIAL PERMIT)					
ITEM #	ITEM	REQUIREMENTS	PROPOSED LOT 3	FUTURE LOT 2	VARIANCE
1	MINIMUM LOT AREA	80,000 S.F.	80,707 S.F. (1.85 AC.)	82,061 S.F. (1.88 AC.) [2]	NO
2	MINIMUM LOT WIDTH	NONE REQUIRED	308 FEET	560 FEET	NO
3	MINIMUM LOT FRONTAGE	150 FEET	260.4 FEET	150 FEET	NO
4	MINIMUM FRONT SETBACK	NONE REQUIRED	71.9 FEET	343 FEET	NO
5	MINIMUM SIDE SETBACK	25 FEET (50 FEET) [1]	72.8 FEET	118.6 FEET	NO
6	MINIMUM REAR SETBACK	25 FEET (50 FEET) [1]	51.4 FEET	89.3 FEET	NO
7	MAXIMUM BUILDING HEIGHT	35 FEET/2.5 STORIES	25.6 FEET	<35 FEET/2.5 STORIES	NO
8	MAXIMUM BUILDING COVERAGE	25 PERCENT	13.2 PERCENT	12.2 PERCENT	NO
9	MAXIMUM IMPERVIOUS COVERAGE	50 PERCENT	39.9 PERCENT	33.5 PERCENT	NO

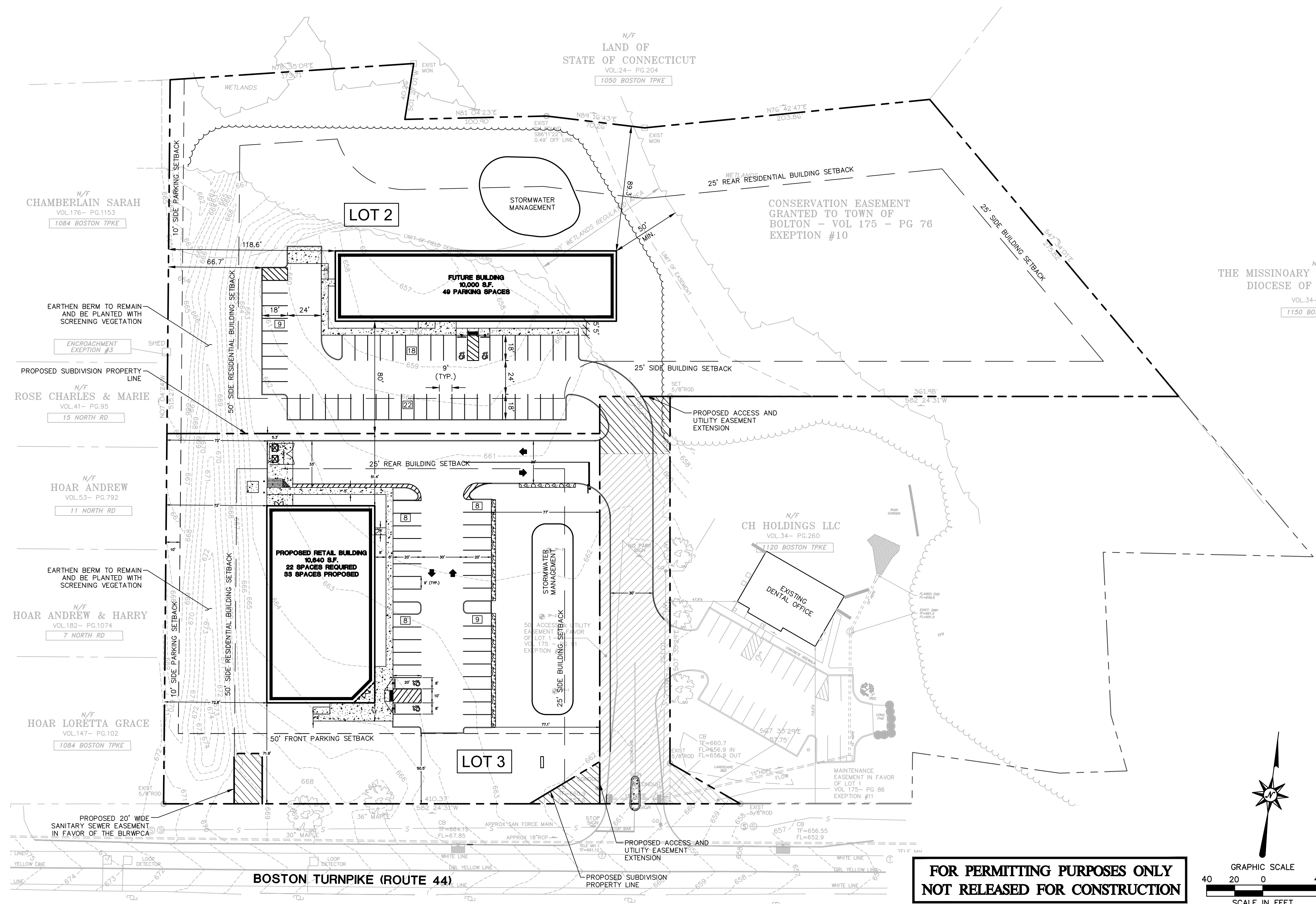
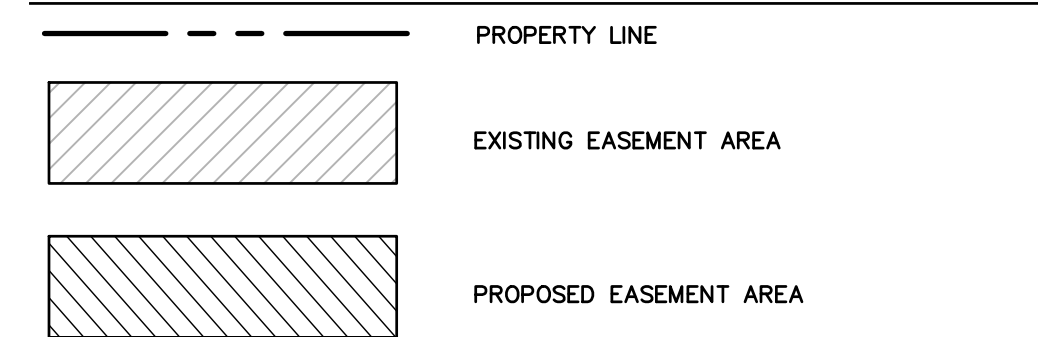
[1] MINIMUM SIDE AND REAR SETBACKS - 50 FEET WHEN ABUTTING A RESIDENTIAL DISTRICT
 [2] LOT AREA FOR LOT 2 DOES NOT INCLUDE ACCESS STRIP, CONSERVATION EASEMENT, OR WETLAND AREAS.

PARKING INFORMATION

ITEM #	ITEM	REQUIREMENTS	PROPOSED LOT 3	FUTURE LOT 2	VARIANCE
1	BUILDING SIZE	600 S.F.	10,640 S.F.	10,000 S.F.	NO
2	PARKING REQUIRED	RETAIL: MINIMUM - 2 SPACES PER 1,000 S.F. OF GFA (10,640/10,000 S.F.) MINIMUM REQUIRED = 22 / 20 SPACES MAXIMUM - 5 SPACES PER 1,000 S.F. OF GFA (10,640/10,000 S.F.) MAXIMUM ALLOWED = 54 / 50 SPACES	33 SPACES	49 SPACES	NO
3	MINIMUM HANDICAPPED PARKING SPACES REQUIRED	2 SPACES	2 SPACES	2 SPACES	NO
4	MINIMUM PARKING DIMENSIONS	9 FEET X 18 FEET	9 FEET X 20 FEET	9 FEET X 18 FEET	NO
5	MINIMUM LOADING DIMENSIONS	10 FEET X 25 FEET X 14 FEET	33 FEET X 71 FEET X > 14 FEET	10 FEET X 25 FEET X > 14 FEET	NO
6	MINIMUM AISLE WIDTH	22 FEET - 2-WAY FEET - 1-WAY	30 FEET - 2-WAY	24 FEET - 2-WAY	NO
7	MINIMUM FRONT SETBACK	50 FEET [3]	50.5 FEET	273.4 FEET	NO
8	MINIMUM SIDE SETBACK	NONE REQUIRED [3]	77.1 FEET	66.7 FEET	NO
9	MINIMUM REAR SETBACK	NONE REQUIRED [3]	5.3 FEET	124 FEET	NO
10	BICYCLE PARKING REQUIRED	1 BICYCLE PARKING SPACE PER 25 PARKING STALLS (2 REQUIRED)	2 BICYCLE PARKING SPACES	2 BICYCLE PARKING SPACES	NO

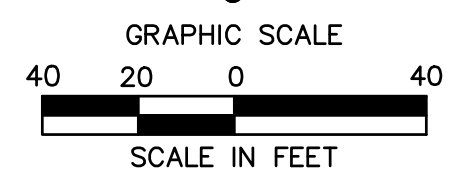
[3] 10 FEET LANDSCAPED BUFFER STRIP REQUIRED WHERE ABUTTING A RESIDENCE DISTRICT

SITE PLAN LEGEND



BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT	
DATE APPROVED _____	DATE OF EXPIRATION _____
CHAIRMAN	

**FOR PERMITTING PURPOSES ONLY
 NOT RELEASED FOR CONSTRUCTION**



100 Constitution Plaza
 10th Floor
 Hartford, CT 06103
 (860) 249-2200
 (860) 249-2400 Fax



PROPOSED RETAIL DEVELOPMENT
 1100 BOSTON TURNPIKE
 BOLTON, CONNECTICUT

REVISIONS

No.	Date	Desc.
1.	05/05/2021	REVISED PER TOWN COMMENTS

Designed: C.J.L.
 Drawn: C.J.L.
 Reviewed: K.M.M.
 Scale: 1"=40'
 Project No.: 2002032
 Date: 04/02/2021
 CAD File: MP200203201

MASTER PLAN

Sheet No.

MP-1

DEMOLITION LEGEND

- PROPERTY LINE
- LOD LIMIT OF DISTURBANCE AND SITEWORK
- CONTRACT LIMIT LINE
- SAWCUT LINE
- XXXXXXXXXXXXXXXXXXXXX REMOVE AND DISPOSE OF CURB, FENCE, ETC.
- P P PROTECT EXISTING UTILITY LINE
- ~ ~ ~ ~ ~ LIMIT OF TREE AND VEGETATION CLEARING
- X REMOVE AND DISPOSE OF SIGN, HYDRANT, FIXTURE, ETC.
- [Hatched Box] REMOVE AND DISPOSE OF EXISTING BITUMINOUS CONCRETE PAVEMENT STRUCTURE
- [Circle with X] REMOVE AND DISPOSE OF EXISTING TREE AND STUMP
- [Circle with S] PROTECT EXISTING TREE TO REMAIN

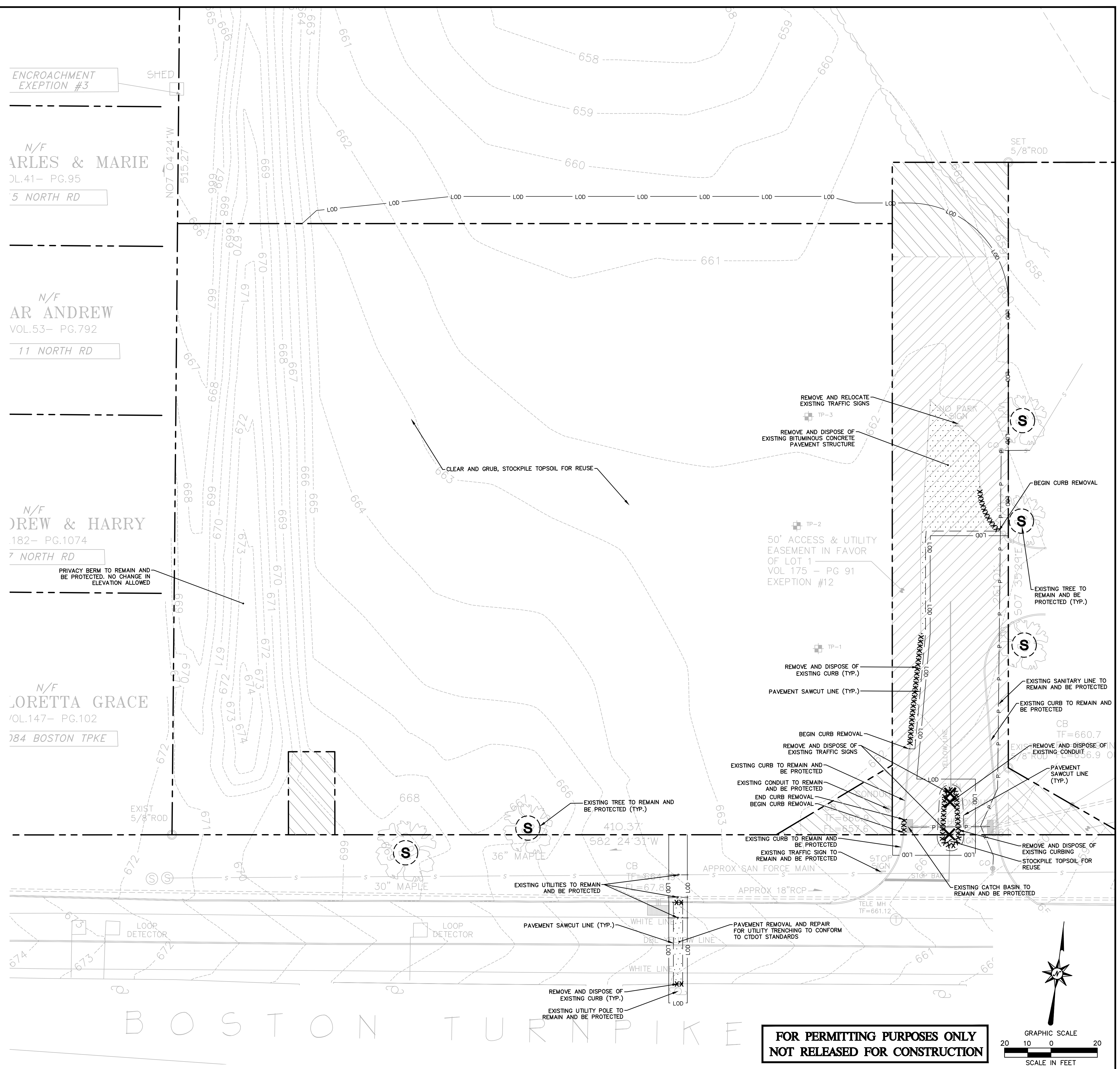
ENCROACHMENT EXEPTION #3

N/F ARLES & MARIE
VOL.41- PG.95
5 NORTH RD

N/F AR ANDREW
VOL.53- PG.792
11 NORTH RD

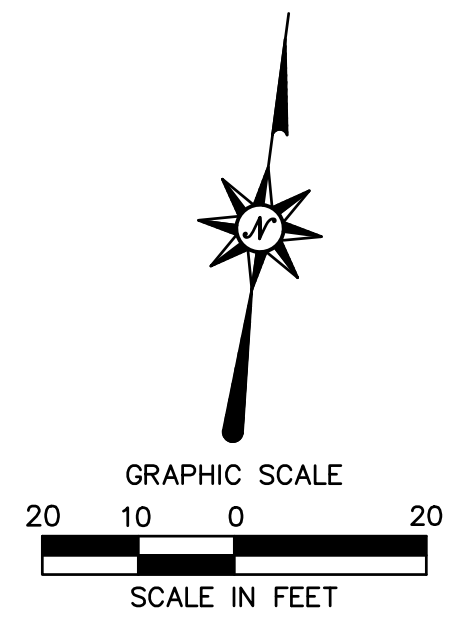
N/F DREW & HARRY
VOL.182- PG.1074
7 NORTH RD

N/F LORETTA GRACE
VOL.147- PG.102
184 BOSTON TPKE



BOSTON TURNPIKE

**FOR PERMITTING PURPOSES ONLY
NOT RELEASED FOR CONSTRUCTION**



BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT

DATE APPROVED _____ DATE OF EXPIRATION _____

CHAIRMAN _____

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPRES ON _____



100 Constitution Plaza
10th Floor
Hartford, CT 06103
(860) 249-2200
(860) 249-2400 Fax



**PROPOSED RETAIL DEVELOPMENT
1100 BOSTON TURNPIKE
BOLTON, CONNECTICUT**

REVISIONS

No.	Date	Desc.
1	05/05/2021	REVISED PER TOWN COMMENTS

Designed _____ S.E.L.
Drawn _____ S.E.L.
Reviewed _____ K.M.M.
Scale 1"=20'
Project No. 2002032
Date 04/02/2021
CAD File: DM20203201

DEMOLITION PLAN

Sheet No.

DM-1

5/4/2021 - C:\Users\blc\OneDrive\Documents\2021\DM20203201 - DM20203201.dwg (14/04/2021) 1:24:04 PM 2021

ZONING INFORMATION

LOCATION: BOLTON, TOLLAND COUNTY, CONNECTICUT				
ZONE: RURAL MIXED USE ZONE (RMUZ)				
USE: RETAIL (PERMITTED BY SPECIAL PERMIT)				
ITEM #	ITEM	REQUIREMENTS	PROPOSED	VARIANCE
1	MINIMUM LOT AREA	80,000 S.F.	80,707 S.F. (1.85 AC.)	NO
2	MINIMUM LOT WIDTH	NONE REQUIRED	308 FEET	NO
3	MINIMUM LOT FRONTAGE	150 FEET	260.4 FEET	NO
4	MINIMUM FRONT SETBACK	NONE REQUIRED	71.9 FEET	NO
5	MINIMUM SIDE SETBACK	25 FEET (50 FEET) [1]	72.8 FEET	NO
6	MINIMUM REAR SETBACK	25 FEET [1]	51.4 FEET	NO
7	MAXIMUM BUILDING HEIGHT	35 FEET/2.5 STORIES	25.6 FEET	NO
8	MAXIMUM BUILDING COVERAGE	25 PERCENT	13.2 PERCENT	NO
9	MAXIMUM IMPERVIOUS COVERAGE	50 PERCENT	39.9 PERCENT	NO

[1] MINIMUM SIDE AND REAR SETBACKS - 50 FEET WHEN ABUTTING A RESIDENTIAL DISTRICT

PARKING INFORMATION

ITEM #	ITEM	REQUIREMENTS	PROPOSED	VARIANCE
1	BUILDING SIZE	600 S.F.	10,640 S.F.	NO
2	PARKING REQUIRED	RETAIL - MINIMUM - 2 SPACES PER 1,000 S.F. OF GFA (10,640 S.F.) MINIMUM REQUIRED = 22 SPACES MAXIMUM - 5 SPACES PER 1,000 S.F. OF GFA (10,640 S.F.) MAXIMUM ALLOWED = 54 SPACES	33 SPACES	NO
3	MINIMUM HANDICAPPED PARKING SPACES REQUIRED	2 SPACES	2 SPACES	NO
4	MINIMUM PARKING DIMENSIONS	9 FEET X 18 FEET	9 FEET X 20 FEET	NO
5	MINIMUM LOADING DIMENSIONS	10 FEET X 25 FEET X 14 FEET	33 FEET X 71 FEET X > 14 FEET	NO
6	MINIMUM AISLE WIDTH	22 FEET - 2-WAY 11 FEET - 1-WAY	30 FEET - 2-WAY	NO
7	MINIMUM FRONT SETBACK	50 FEET [2]	50.5 FEET	NO
8	MINIMUM SIDE SETBACK	NONE REQUIRED [2]	77.1 FEET	NO
9	MINIMUM REAR SETBACK	NONE REQUIRED [2]	5.3 FEET	NO
10	BICYCLE PARKING REQUIRED	1 BICYCLE PARKING SPACE PER 25 PARKING SPACES (2 REQUIRED)	2 BICYCLE PARKING SPACES	NO

[2] 10 FEET LANDSCAPED BUFFER STRIP REQUIRED WHERE ABUTTING A RESIDENCE DISTRICT

SITE PLAN LEGEND

	PROPERTY LINE
	LIMIT OF DISTURBANCE AND SITWORK CONTRACT LIMIT LINE
	SAWCUT LINE
	PROVIDE AND INSTALL CONCRETE PAVEMENT STRUCTURE, REINFORCED CONCRETE SIDEWALK, OR MONOLITHIC CONCRETE CURB AND SIDEWALK
	PROVIDE AND INSTALL FULL DEPTH HEAVY DUTY BITUMINOUS CONCRETE PAVEMENT STRUCTURE
	PROVIDE AND INSTALL FULL DEPTH STANDARD DUTY BITUMINOUS CONCRETE PAVEMENT STRUCTURE
	PROVIDE AND INSTALL SIGN

SIGN LEGEND

SIGN NO.	C-DOT NO.	LEGEND
A	31-0552Z	30"
B	31-0629	
C	31-0648	

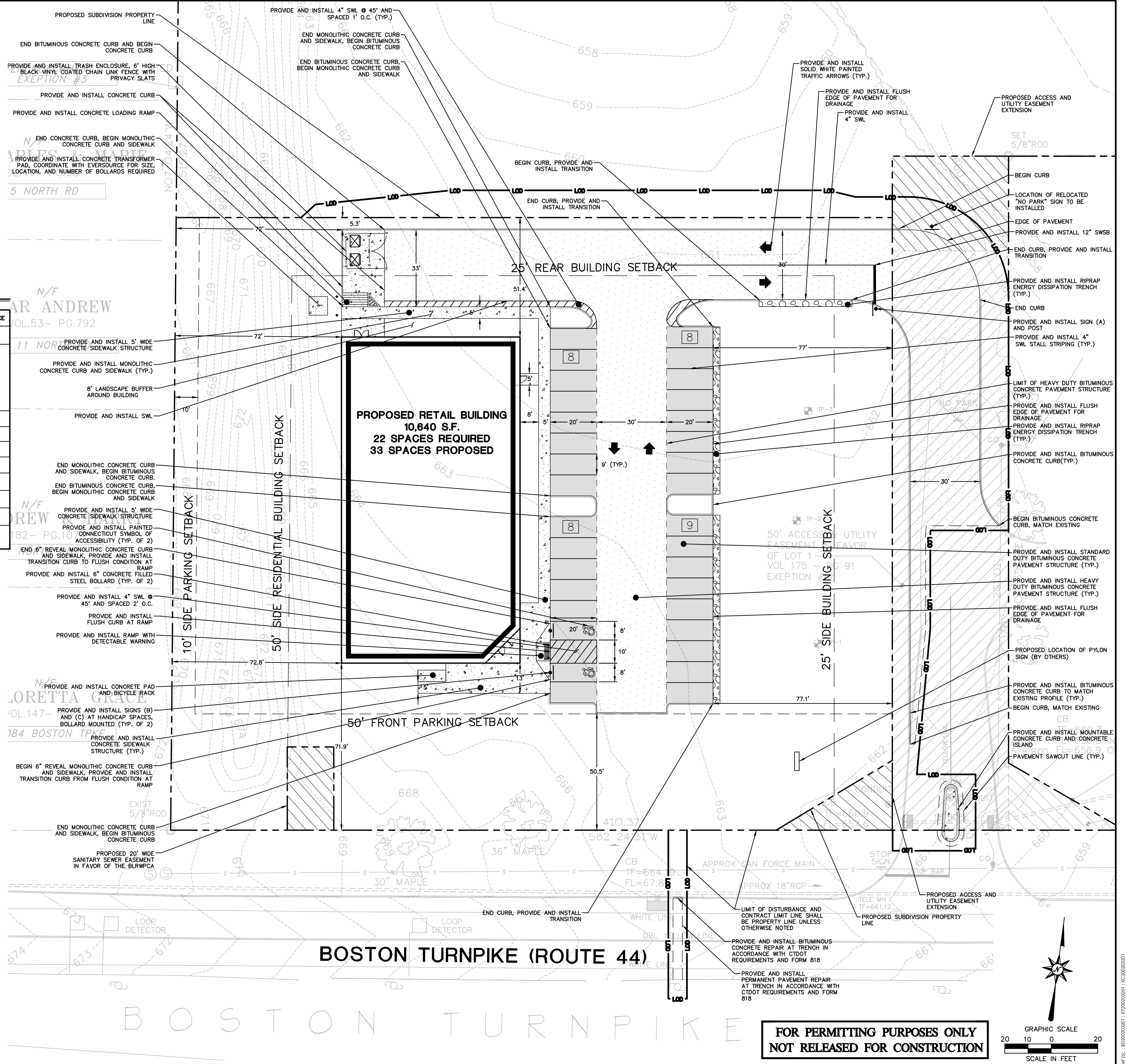
NOTE:
1. HANDICAPPED SIGNS TO BE INSTALLED IN PIPE BOLLARDS (SEE DETAIL). ALL HANDICAP SIGNAGE TO CONFORM TO LATEST BUILDING CODE.
2. SIGNS INSTALLED IN THE STATE RIGHT-OF-WAY MUST BE INSTALLED IN ACCORDANCE WITH THE DEPARTMENT'S TYPICAL DETAIL SHEETS (I.E. HEIGHT, BREAKAWAY POSTS, ETC.)

BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT

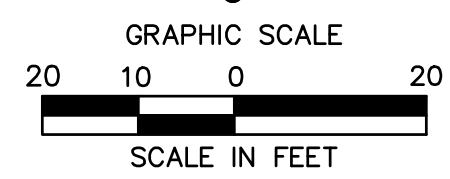
DATE APPROVED _____ DATE OF EXPIRATION _____

CHAIRMAN _____

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON _____



FOR PERMITTING PURPOSES ONLY
NOT RELEASED FOR CONSTRUCTION



PROPOSED RETAIL DEVELOPMENT

1100 BOSTON TURNPIKE
BOLTON, CONNECTICUT

REVISIONS

No.	Date	Desc.
1	05/05/2021	REVISED PER TOWN COMMENTS

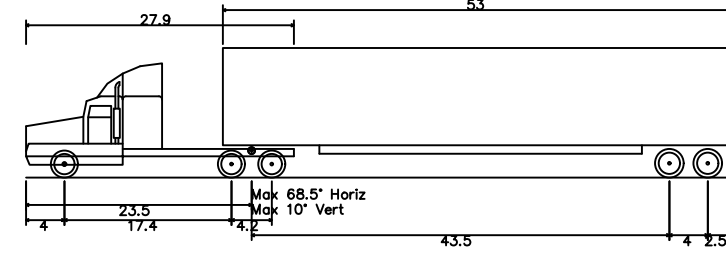
Designed S.E.L.
Drawn S.E.L.
Reviewed K.M.M.
Scale 1"=20'
Project No. 2002032
Date 04/02/2021
CAD File: SP200203201

Title

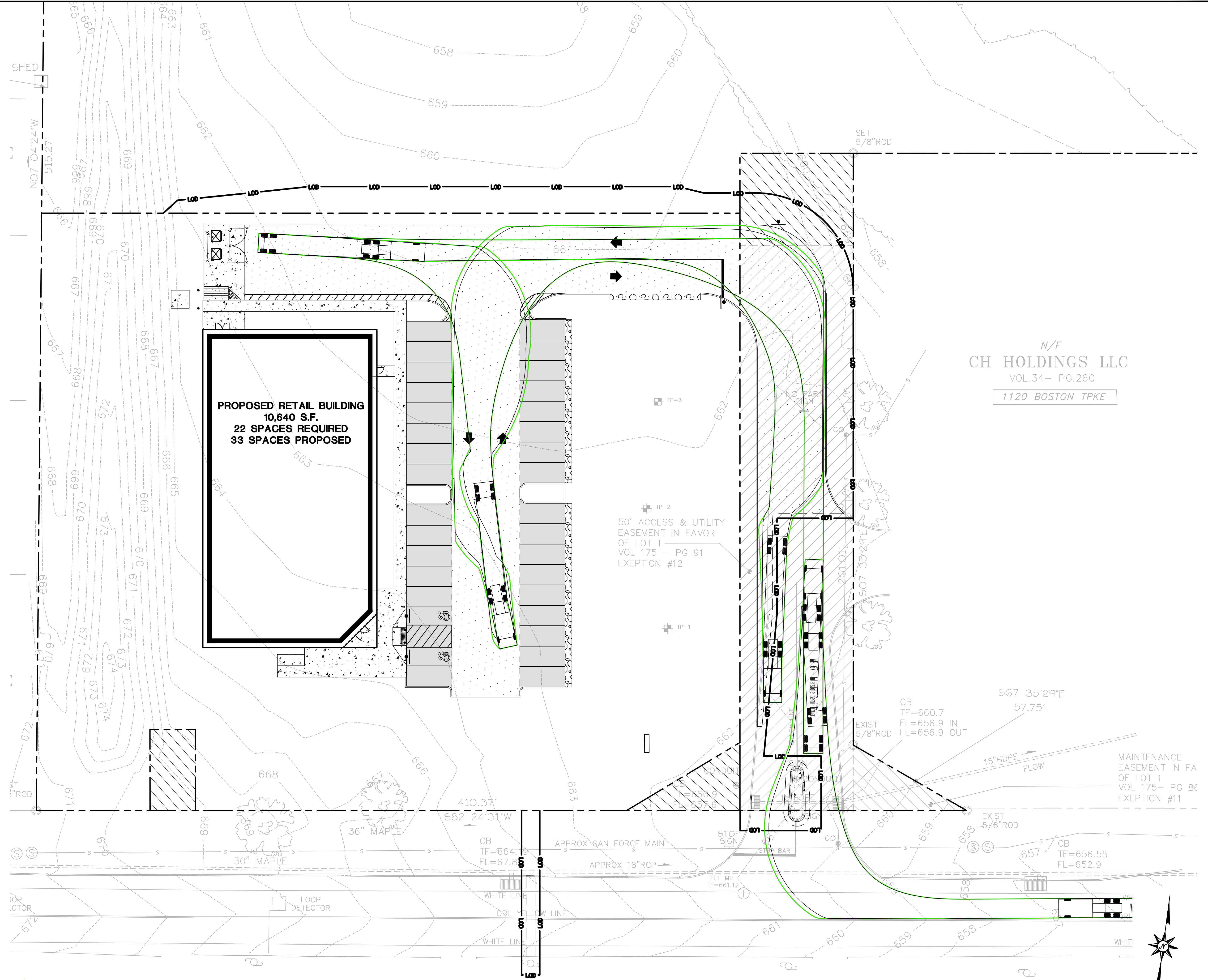
SITE PLAN

Sheet No.

SP-1



WB-67 - Interstate Semi-Trailer
 Overall Length 73.50ft
 Overall Width 8.50ft
 Overall Body Height 13.50ft
 Min Body Ground Clearance 13.54ft
 Max Track Width 8.50ft
 Lock-to-lock time 6.00s
 Max Steering Angle (Virtual) 28.4°



PROPOSED RETAIL BUILDING
 10,640 S.F.
 22 SPACES REQUIRED
 33 SPACES PROPOSED

N/F
CH HOLDINGS LLC
 VOL.34- PG.260
 1120 BOSTON TPKE

TP-2
 50' ACCESS & UTILITY
 EASEMENT IN FAVOR
 OF LOT 1
 VOL 175 - PG 91
 EXEPTION #12

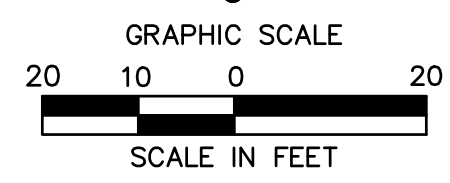
MAINTENANCE
 EASEMENT IN FA
 OF LOT 1
 VOL 175- PG 91
 EXEPTION #11

CB
 TF=660.7
 FL=656.9 IN
 FL=656.9 OUT

EXIST
 5/8"ROD
 CB
 TF=656.55
 FL=652.9

BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT
 DATE APPROVED _____ DATE OF EXPIRATION _____
 _____ CHAIRMAN
 THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON _____

FOR PERMITTING PURPOSES ONLY
NOT RELEASED FOR CONSTRUCTION



100 Constitution Plaza
 10th Floor
 Hartford, CT 06103
 (860) 249-2200
 (860) 249-2400 Fax



PROPOSED RETAIL DEVELOPMENT
 1100 BOSTON TURNPIKE
 BOLTON, CONNECTICUT

REVISIONS

No.	Date	Desc.
1.	05/05/2021	REVISED PER TOWN COMMENTS

Designed	SEL
Drawn	SEL
Reviewed	K.M.M.
Scale	1"=20'
Project No.	2002032
Date	04/02/2021
CAD File:	TT200203201

Title
TRUCK TURNING PLAN - WB-67

Sheet No.

TT-1

5/4/2021 - C:\Users\blc\OneDrive\Documents\17000203201\DWG\TT1.dwg - 1:48:28 AM

Sheet No.: 180200203201 - 17000203201 - 17000203201

SITE UTILITIES LEGEND

---	PROPERTY LINE
---	LIMIT OF DISTURBANCE AND SITEWORK
---	CONTRACT LIMIT LINE
---	SAWCUT LINE
E	ELECTRIC LINE
G	GAS LINE
W	WATER LINE
S	SANITARY SEWER LINE
SFM	SANITARY SEWER FORCE MAIN
T	TELECOMMUNICATIONS LINE
ETC	ELECTRIC AND TELECOMMUNICATIONS LINE
---	STORM LINE

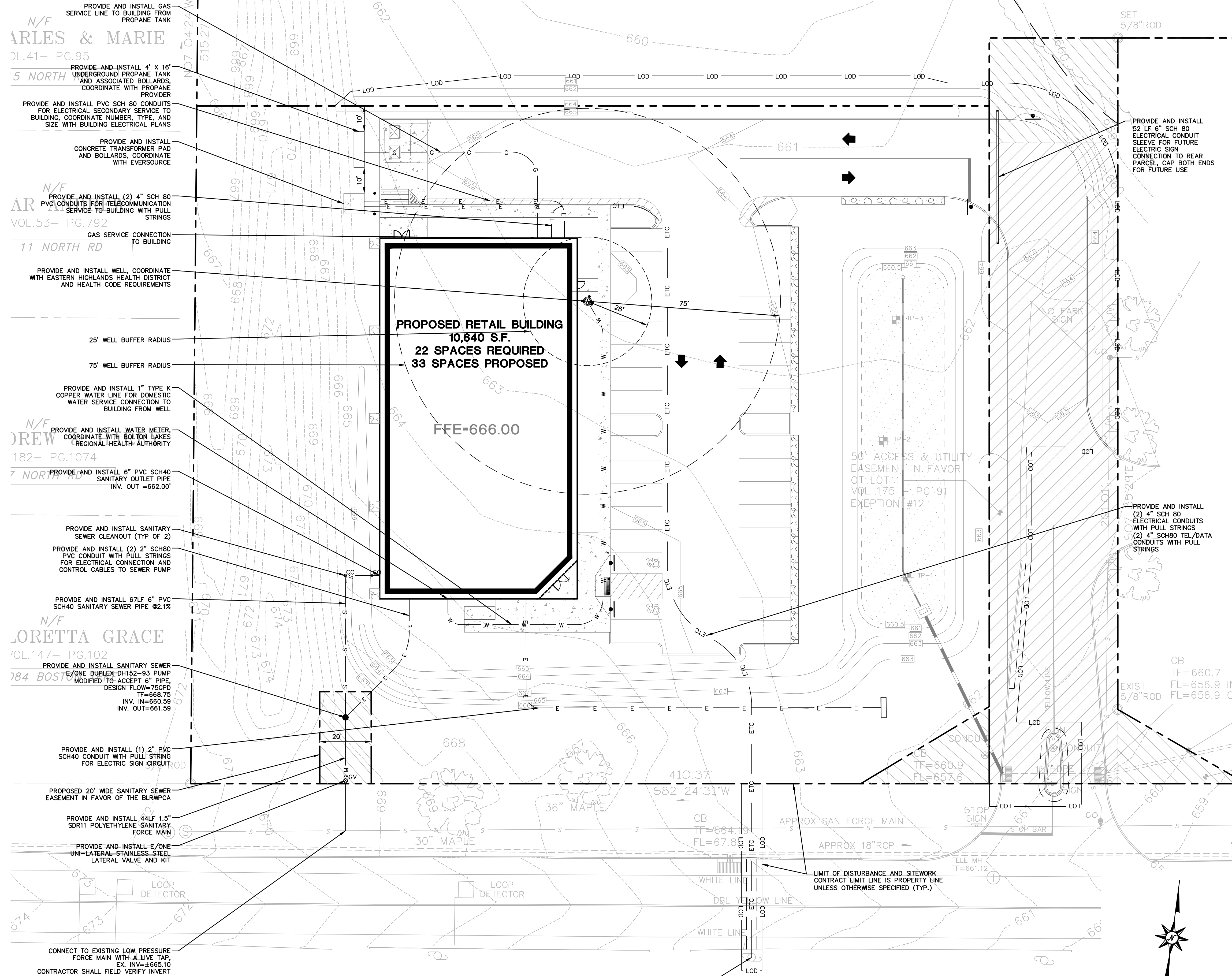
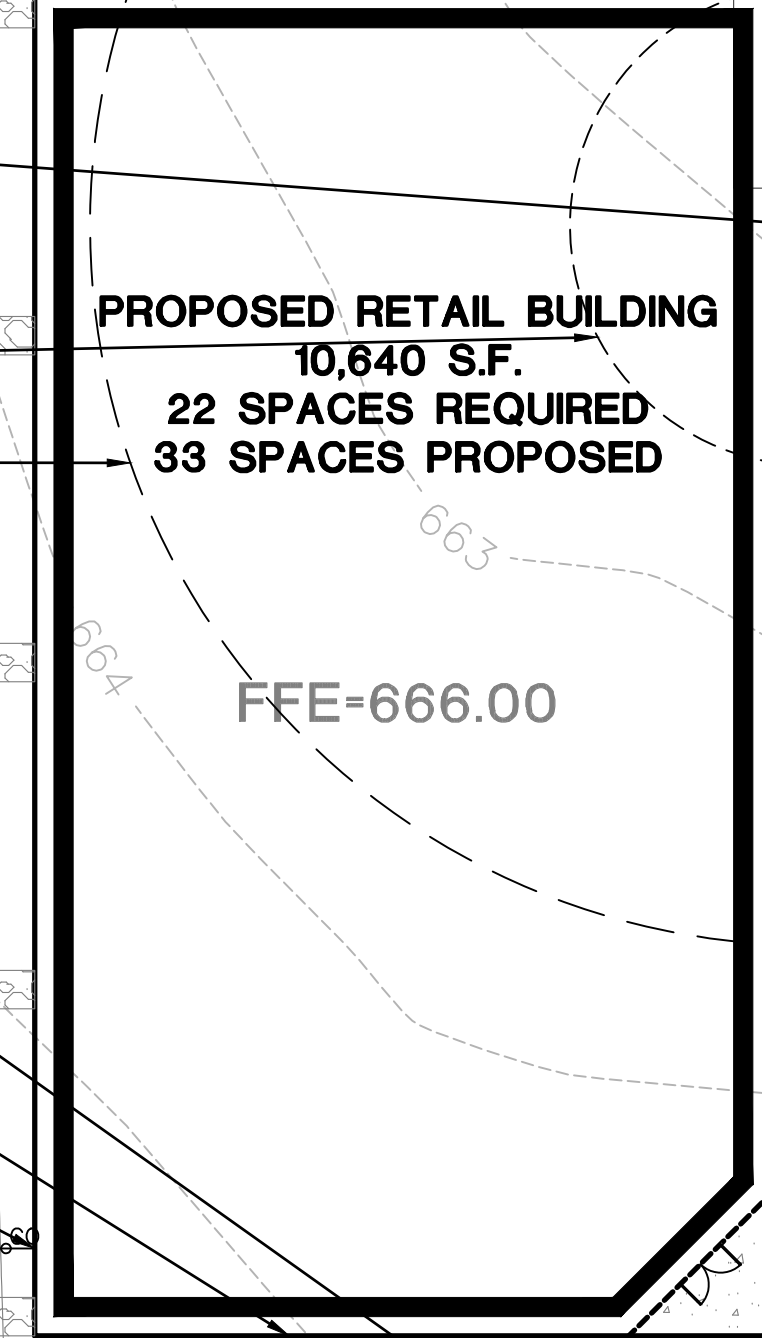
ENCROACHMENT EXEPTION #3

N/F CHARLES & MARIE
VOL.41- PG.95
5 NORTH

N/F AR...
VOL.53- PG.792
11 NORTH RD

N/F DREW...
182- PG.1074
7 NORTH RD

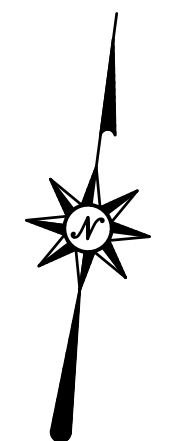
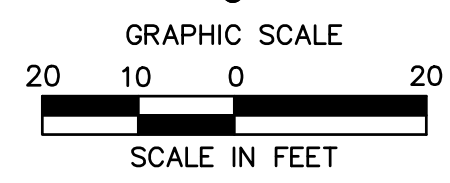
N/F LORETTA GRACE
VOL.147- PG.102
184 BOSTON TURNPIKE



CONNECT TO EXISTING LOW PRESSURE FORCE MAIN WITH A LIVE TAP. EX. INV.=±665.10 CONTRACTOR SHALL FIELD VERIFY INVERT AND INFORM SEWER PUMP MANUFACTURER OF REQUIRED HEAD LOSS

PROVIDE AND INSTALL RIGID METAL CONDUIT DOWN POLE TO UNDERGROUND UTILITIES. COORDINATE ELECTRIC AND TELECOMMUNICATIONS CONNECTIONS WITH UTILITY PROVIDERS

**FOR PERMITTING PURPOSES ONLY
NOT RELEASED FOR CONSTRUCTION**



BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT	
DATE APPROVED _____	DATE OF EXPIRATION _____
CHAIRMAN	

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON _____



100 Constitution Plaza
10th Floor
Hartford, CT 06103
(860) 249-2200
(860) 249-2400 Fax



PROPOSED RETAIL DEVELOPMENT
1100 BOSTON TURNPIKE
BOLTON, CONNECTICUT

REVISIONS

No.	Date	Desc.	REVISED PER TOWN COMMENTS
1.	03/05/2021		

Designed	S.E.L.
Drawn	S.E.L.
Reviewed	K.M.M.
Scale	1"=20'
Project No.	2002032
Date	04/02/2021
CAD File:	SU200203201

SITE UTILITIES PLAN

Sheet No.

SU-1

EROSION CONTROL LEGEND

- PROPERTY LINE
- LOD LIMIT OF DISTURBANCE AND SITEWORK CONTRACT LIMIT LINE
- SAWCUT LINE
- SF- SILT FENCE BARRIER
- SS SILT SACK INLET PROTECTION
- CONCRETE WASH PIT
- TEMPORARY MATERIAL STOCKPILE
- EROSION CONTROL BLANKET
- CONSTRUCTION ENTRANCE

ENCROACHMENT EXEPTION #3

N/F
ARLES & MARIE
DL.41- PG.95
5 NORTH RD

N/F
AR ANDREW
VOL.53- PG.792
11 NORTH RD

N/F
DREW & HARRY
182- PG.1074
7 NORTH RD

N/F
LORETTA GRACE
VOL.147- PG.102
184 BOSTON TPKE

PROPOSED RETAIL BUILDING
10,640 S.F.
22 SPACES REQUIRED
33 SPACES PROPOSED

FFE-666.00

DESIGNATED LOCATION FOR CONCRETE WASHPIT

DESIGNATED LOCATION FOR MATERIAL STOCKPILE, PROVIDE AND INSTALL DOUBLE ROW OF SILT FENCE BARRIER AROUND ENTIRE STOCKPILE

PROVIDE AND INSTALL EROSION CONTROL BLANKET ON ALL SLOPES 3:1 OR STEEPER (TYP.)

PROVIDE AND INSTALL SILT FENCE BARRIER (TYP.)

PROVIDE AND INSTALL STONE CONSTRUCTION ENTRANCE (MINIMUM 30'X50')

PROVIDE AND INSTALL SILT SACK INLET PROTECTION (TYP.)

LIMIT OF DISTURBANCE AND SITE CONTRACT LIMIT LINE SHALL BE PROPERTY LINE UNLESS OTHERWISE NOTED

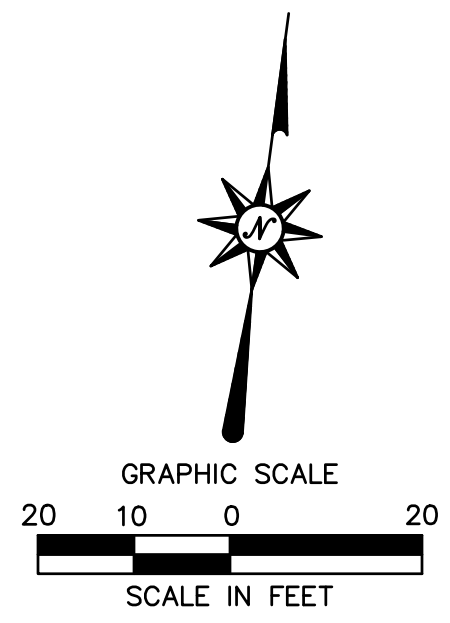
BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT

DATE APPROVED _____ DATE OF EXPIRATION _____

CHAIRMAN

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON _____

FOR PERMITTING PURPOSES ONLY
NOT RELEASED FOR CONSTRUCTION



B O S T O N T U R N P I K E



PROPOSED RETAIL DEVELOPMENT
1100 BOSTON TURNPIKE
BOLTON, CONNECTICUT

REVISIONS

No.	Date	Desc.	REVISED PER TOWN COMMENTS
1.	05/05/2021		

Designed	S.E.L.
Drawn	S.E.L.
Reviewed	K.M.M.
Scale	1"=20'
Project No.	2002032
Date	04/02/2021
CAD File:	EC200203201

Title
SEDIMENT AND EROSION CONTROL PLAN

Sheet No.

EC-1

SEDIMENT AND EROSION CONTROL NOTES

SEDIMENT & EROSION CONTROL NARRATIVE
THE SEDIMENT AND EROSION CONTROL PLAN WAS DEVELOPED TO PROTECT THE EXISTING ROADWAY AND STORM DRAINAGE SYSTEMS, ADJACENT PROPERTIES, AND ANY ADJACENT WETLAND AREA AND ANY ADJACENT WATER COURSE FROM SEDIMENT LADEN SURFACE RUNOFF AND EROSION. A CONSTRUCTION SEQUENCE IS PROVIDED TO PROVIDE SURFACE RUNOFF EROSION CONTROLS PRIOR TO THE BEGINNING OF PROJECT DEMOLITION AND/OR CONSTRUCTION.

CONSTRUCTION SCHEDULE
THE ANTICIPATED STARTING DATE FOR CONSTRUCTION IS SPRING 2021 WITH COMPLETION ANTICIPATED FALL 2021. APPROPRIATE SEDIMENT AND EROSION CONTROL MEASURES AS DESCRIBED HEREIN SHALL BE INSTALLED BY THE CONTRACTOR PRIOR TO THE COMMENCEMENT OF ALL DEMOLITION OR CONSTRUCTION ACTIVITY. SCHEDULE WORK TO MINIMIZE THE LENGTH OF TIME THAT BARE SOIL WILL BE EXPOSED.

CONTINGENCY EROSION PLAN
THE CONTRACTOR SHALL INSTALL ALL SPECIFIED SEDIMENT AND EROSION CONTROL MEASURES AND WILL BE REQUIRED TO MAINTAIN THEM IN THEIR INTENDED FUNCTIONING CONDITION. THE AGENTS OF THE MUNICIPALITY OR INLAND WETLANDS COMMISSION AND/OR CIVIL ENGINEER SHALL HAVE THE AUTHORITY TO REQUIRE SUPPLEMENTAL MAINTENANCE OR ADDITIONAL MEASURES IF FIELD CONDITIONS ARE ENCOUNTERED BEYOND WHAT WOULD NORMALLY BE ANTICIPATED.

CONSTRUCTION SEQUENCE
THE FOLLOWING CONSTRUCTION SEQUENCE IS RECOMMENDED:

- 1. CONTACT MUNICIPALITY OR INLAND WETLANDS COMMISSION AGENT AT LEAST FORTY-EIGHT (48) HOURS PRIOR TO COMMENCEMENT OF ANY DEMOLITION, CONSTRUCTION OR REGULATED ACTIVITY ON THIS PROJECT.
- 2. CLEARING LIMITS SHALL BE PHYSICALLY MARKED IN THE FIELD AND APPROVED BY THE MUNICIPALITY OR INLAND WETLANDS COMMISSION AGENT PRIOR TO THE START OF WORK ON THE SITE. INSTALL TREE PROTECTION AND PERIMETER SILT FENCE.
- 3. CONSTRUCT STONE CONSTRUCTION ENTRANCE ANTI-TRACKING PADS AT CONSTRUCTION ENTRANCES/EXITS AND INSTALL FILTER FABRIC AROUND GRATES OF CATCH BASINS OR INSTALL SILT SACKS ON CATCH BASIN INLETS ON OFF SITE ROADS. INSTALL SILT FENCE AND OTHER EROSION CONTROL DEVICES INDICATED ON THESE PLANS AT PERIMETER OF PROPOSED SITE. DISTURBANCE AND INSTALL ALL EROSION CONTROL MEASURES AND TREE PROTECTION INDICATED ON THESE PLANS. INSTALL SEDIMENT BASINS AND SEDIMENT TRAPS IF REQUIRED AT LOW AREAS OF SITE OR AS ORDERED BY THE ENGINEER OR AS SHOWN ON THESE PLANS.
- 4. CLEAR AND GRUB SITE. STOCKPILE CHIPS. STOCKPILE TOPSOIL. INSTALL SEDIMENT AND EROSION CONTROLS AT STOCKPILES.
- 5. ANY BUILDING AND SITE DEMOLITION AND REMOVAL. PAVEMENT REMOVAL.
- 6. INSTALL SILT FENCE, CONSTRUCT ANY DIVERSION SWALES AND SEDIMENT BASINS AND SEDIMENT TRAPS. COMMENCE INSTALLATION OF STORM DRAINAGE SYSTEM.
- 7. COMMENCE EARTHWORK. INSTALL ADDITIONAL SEDIMENT AND EROSION CONTROLS AS WORK PROGRESSES AND CONTINUE STORM DRAINAGE SYSTEM CONSTRUCTION, TOPSOIL AND SEED SLOPES WHICH HAVE ACHIEVED FINAL SITE GRADING.
- 8. CONSTRUCTION STAKING OF ALL BUILDING CORNERS, UTILITIES, ACCESS DRIVES, AND PARKING AREAS.
- 9. ROUGH GRADING AND FILLING OF SUBGRADES AND SLOPES.
- 10. IMMEDIATELY UPON DISCOVERING UNFORESEEN CIRCUMSTANCES POSING THE POTENTIAL FOR ACCELERATED EROSION AND/OR SEDIMENT POLLUTION, THE OPERATOR SHALL IMPLEMENT APPROPRIATE BEST MANAGEMENT PRACTICES TO ELIMINATE THE POTENTIAL FOR ACCELERATED EROSION AND/OR SEDIMENT POLLUTION.
- 11. BEFORE DISPOSING OF SOIL OR RECEIVING BORROW FOR THE SITE, THE CONTRACTOR MUST PROVIDE EVIDENCE THAT EACH SPOIL OR BORROW AREA HAS A SEDIMENT AND EROSION CONTROL PLAN APPROVED BY THE MUNICIPALITY AND/OR INLAND WETLANDS COMMISSION AND WHICH IS BEING IMPLEMENTED AND MAINTAINED. THE CONTRACTOR SHALL ALSO NOTIFY THE MUNICIPALITY AND/OR INLAND WETLANDS COMMISSION IN WRITING OF ALL RECEIVING SPOIL AND BORROW AREAS WHEN THEY HAVE BEEN IDENTIFIED.
- 12. CONTINUE INSTALLATION OF STORM DRAINAGE AS SUBGRADE ELEVATIONS ARE ACHIEVED.
- 13. BUILDING FOUNDATION SUBGRADE AND PAD SUBGRADE PREPARATION.
- 14. BUILDING FOUNDATION CONSTRUCTION. BEGIN BUILDING SUPERSTRUCTURE.
- 15. THROUGHOUT CONSTRUCTION SEQUENCE, REMOVE SEDIMENT FROM BEHIND ANY SILT FENCES, HAY BALES AND OTHER EROSION CONTROL DEVICES, AND FROM SEDIMENT BASINS AND SEDIMENT TRAPS AS REQUIRED. REMOVAL SHALL BE ON A PERIODIC BASIS (EVERY SIGNIFICANT RAINFALL OF 0.25 INCH OR GREATER). INSPECTION OF SEDIMENT AND EROSION CONTROL MEASURES SHALL BE ON A WEEKLY BASIS AND AFTER EACH RAINFALL OF 0.25 INCHES OR GREATER. SEDIMENT COLLECTED SHALL BE DEPOSITED AND SPREAD EVENLY UPLAND ON SLOPES DURING CONSTRUCTION.
- 16. INSTALL SANITARY LATERAL AND UTILITIES. COMPLETE STORM DRAINAGE SYSTEM.
- 17. INSTALL SITE LIGHTING AND TRASH ENCLOSURE.
- 18. COMPLETE GRADING TO SUBGRADES AND CONSTRUCT PARKING AREA SUBGRADE.
- 19. CONSTRUCT CURBS, PAVEMENT STRUCTURE AND SIDEWALKS.
- 20. CONDUCT FINE GRADING.
- 21. PAVING OF PARKING AREAS AND DRIVEWAYS.
- 22. FINAL FINE GRADING OF SLOPE AND NON-PAVED AREAS.
- 23. PLACE 4" TOPSOIL ON SLOPES AFTER FINAL GRADING IS COMPLETED. FERTILIZE SEED AND MULCH. SEED MIXTURE TO BE INSTALLED APRIL 15 - JUNE 1 OR AUGUST 15 - OCTOBER 15. USE EROSION CONTROL BLANKETS AS REQUIRED OR ORDERED FOR SLOPES GREATER THAN 3:1 AND AS SHOWN ON LANDSCAPE PLANS OR EROSION CONTROL PLANS. FOR TEMPORARY STABILIZATION BEYOND SEEDING DATES USE ANNUAL RYE AT 4.0 LBS./1,000 S.F. FERTILIZE WITH 10-10-10 AT 1.0 LBS. OF NITROGEN PER 1,000 S.F. AND LIME AT 100 LBS./1,000 S.F. (MAX.).
- 24. LANDSCAPE ISLANDS, INTERIOR NON-PAVED AREAS, AND PERIMETER AREAS.
- 25. INSTALL SIGNING AND PAVEMENT MARKINGS.
- 26. CLEAN STORM DRAINAGE PIPE STRUCTURES, DETENTION SYSTEMS AND WATER QUALITY DEVICES OF DEBRIS AND SEDIMENT.
- 27. UPON DIRECTION OF THE MUNICIPALITY AND/OR INLAND WETLANDS COMMISSION AGENT, SEDIMENT AND EROSION CONTROL MEASURES SHALL BE REMOVED FOLLOWING STABILIZATION OF THE SITE.

OPERATION REQUIREMENTS

CLEARING AND GRUBBING OPERATIONS
1. ALL SEDIMENT AND EROSION CONTROL MEASURES, INCLUDING THE CONSTRUCTION OF TEMPORARY SEDIMENTATION BASINS AND STONE CONSTRUCTION ENTRANCE ANTI-TRACKING PADS, WILL BE INSTALLED PRIOR TO THE START OF CLEARING AND GRUBBING AND DEMOLITION OPERATIONS.

- 2. FOLLOWING INSTALLATION OF ALL SEDIMENT AND EROSION CONTROL MEASURES, THE CONTRACTOR SHALL NOT PROCEED WITH GRADING, FILLING OR OTHER CONSTRUCTION OPERATIONS UNTIL THE ENGINEER HAS INSPECTED AND APPROVED ALL INSTALLATIONS.
- 3. THE CONTRACTOR SHALL TAKE EXTREME CARE DURING CLEARING AND GRUBBING OPERATIONS SO AS NOT TO DISTURB UNPROTECTED WETLAND AREAS OR SEDIMENT AND EROSION CONTROL DEVICES.
- 4. FOLLOWING THE COMPLETION OF CLEARING AND GRUBBING OPERATIONS, ALL AREAS SHALL BE STABILIZED WITH TOPSOIL AND SEEDING OR CRUSHED STONE AS SOON AS PRACTICAL.

ROUGH GRADING OPERATIONS

- 1. DURING THE REMOVAL AND/OR PLACEMENT OF EARTH AS INDICATED ON THE GRADING PLAN, TOPSOIL SHALL BE STRIPPED AND APPROPRIATELY STOCKPILED FOR REUSE.
- 2. ALL STOCKPILED TOPSOIL SHALL BE SEED, MULCHED WITH HAY, AND ENCLOSED BY A SILTATION FENCE.

FILLING OPERATIONS

- 1. PRIOR TO FILLING, ALL SEDIMENT AND EROSION CONTROL DEVICES SHALL BE PROPERLY IMPLEMENTED, MAINTAINED AND FULLY INSTALLED, AS DIRECTED BY THE ENGINEER AND AS SHOWN ON THIS PLAN.
- 2. ALL FILL MATERIAL ADJACENT TO ANY WETLAND AREAS, IF APPLICABLE TO THIS PROJECT, SHALL BE GOOD QUALITY, WITH LESS THAN 5% FINES PASSING THROUGH A #200 SIEVE (BANK RUN), SHALL BE PLACED IN LIFT THICKNESSES NOT GREATER THAN THAT SPECIFIED IN PROJECT SPECIFICATIONS AND/OR THE PROJECT GEOTECHNICAL REPORT. LIFTS SHALL BE COMPACTED TO 95% MAX. DRY DENSITY MODIFIED PROCTOR OR AS SPECIFIED IN THE CONTRACT SPECIFICATIONS OR IN THE GEOTECHNICAL REPORT.
- 3. AS GENERAL GRADING OPERATIONS PROGRESS, ANY TEMPORARY DIVERSION DITCHES SHALL BE RAISED OR LOWERED, AS NECESSARY, TO DIVERT SURFACE RUNOFF TO THE SEDIMENT BASINS OR SEDIMENT TRAPS.

PLACEMENT OF DRAINAGE STRUCTURES, UTILITIES, AND BUILDING CONSTRUCTION OPERATIONS.

- 1. SILT FENCES SHALL BE INSTALLED AT THE DOWNHILL SIDES OF BUILDING EXCAVATIONS, MUD PUMP DISCHARGES, AND UTILITY TRENCH MATERIAL STOCKPILES. HAY BALES/STRAW BALES MAY BE USED IF SHOWN ON THE SEDIMENT AND EROSION CONTROL PLANS OR IF DIRECTED BY THE CIVIL ENGINEER.

FINAL GRADING AND PAVING OPERATIONS

- 1. ALL INLET AND OUTLET PROTECTION SHALL BE PLACED AND MAINTAINED AS SHOWN ON SEDIMENT AND EROSION CONTROL PLANS AND DETAILS, AND AS DESCRIBED IN SPECIFICATIONS AND AS DESCRIBED HEREIN.
- 2. NO CUT OR FILL SLOPES SHALL EXCEED 2:1 EXCEPT WHERE STABILIZED BY ROCK FACED EMBANKMENTS OR EROSION CONTROL BLANKETS, OR JUTE MESH AND VEGETATION. ALL SLOPES SHALL BE SEED, AND ANY ROAD OR DRIVEWAY SHOULDER AND BANKS SHALL BE STABILIZED IMMEDIATELY UPON COMPLETION OF FINAL GRADING UNTIL TURF IS ESTABLISHED.
- 3. PAVEMENT SUB-BASE AND BASE COURSES SHALL BE INSTALLED OVER AREAS TO BE PAVED AS SOON AS FINAL SUB-GRADES ARE ESTABLISHED AND UNDERGROUND UTILITIES AND STORM DRAINAGE SYSTEMS HAVE BEEN INSTALLED.
- 4. AFTER CONSTRUCTION OF PAVEMENT, TOPSOIL, FINAL SEED, MULCH AND LANDSCAPING, REMOVE ALL TEMPORARY SEDIMENT AND EROSION CONTROL DEVICES ONLY AFTER ALL AREAS HAVE BEEN PAVED AND/OR GRASS HAS BEEN WELL ESTABLISHED AND THE SITE IS STABLE AND HAS BEEN INSPECTED AND APPROVED BY THE MUNICIPALITY AND/OR INLAND WETLANDS COMMISSION.

INSTALLATION OF SEDIMENTATION AND EROSION CONTROL MEASURES

- I. SILTATION FENCE
A. DIG A SIX INCH TRENCH ON THE UPHILL SIDE OF THE DESIGNATED FENCE LINE LOCATION.
B. POSITION THE POST AT THE BACK OF THE TRENCH (DOWNHILL SIDE), AND HAMMER THE POST AT LEAST 1.5 FEET INTO THE GROUND.
C. LAY THE BOTTOM SIX INCHES OF THE FABRIC INTO THE TRENCH TO PREVENT UNDERMINING BY STORM WATER RUN-OFF.
D. BACKFILL THE TRENCH AND COMPACT.
- II. HAY BALES/STRAW BALES
A. BALES SHALL BE PLACED IN A SINGLE ROW, LENGTHWISE, ORIENTED PARALLEL TO THE CONTOUR, WITH ENDS OF ADJACENT BALES TIGHTLY ABUTTING ONE ANOTHER.
B. BALES SHALL BE ENTRENCHED AND BACKFILLED. A TRENCH SHALL BE EXCAVATED THE WIDTH OF A BALE AND THE LENGTH OF THE PROPOSED BARRIER TO A MINIMUM DEPTH OF FOUR INCHES. AFTER THE BALES ARE STAKED, THE EXCAVATED SOIL SHALL BE BACKFILLED AGAINST THE BARRIER.
C. EACH BALE SHALL BE SECURELY ANCHORED BY AT LEAST TWO (2) STAKES.
D. THE GAPS BETWEEN BALES SHALL BE WEDGED WITH STRAW TO PREVENT WATER LEAKAGE.
E. THE BARRIER SHALL BE EXTENDED TO SUCH A LENGTH THAT THE BOTTOMS OF THE END BALES ARE HIGHER IN ELEVATION THAN THE TOP OF THE LOWEST MIDDLE BALE, TO ENSURE THAT RUN-OFF WILL FLOW EITHER THROUGH OR OVER THE BARRIER, BUT NOT AROUND IT.

OPERATION AND MAINTENANCE OF SEDIMENT AND EROSION CONTROL MEASURES

- I. SILTATION FENCE
A. ALL SILTATION FENCES SHALL BE INSPECTED AS A MINIMUM WEEKLY OR AFTER EACH RAINFALL. ALL DETERIORATED FABRIC AND DAMAGED POSTS SHALL BE REPLACED AND PROPERLY REPOSITIONED IN ACCORDANCE WITH THIS PLAN.
B. SEDIMENT DEPOSITS SHALL BE REMOVED FROM BEHIND THE FENCE WHEN THEY REACH A MAXIMUM HEIGHT OF ONE FOOT.
- II. HAY BALES/STRAW BALES
A. ALL HAY BALE/STRAW BALE RINGS SHALL BE INSPECTED FOLLOWING EACH RAINFALL. REPAIR OR REPLACEMENT SHALL BE PROMPTLY MADE AS NEEDED.
B. DEPOSITS SHALL BE REMOVED AND CLEANED-OUT IF ONE HALF OF THE ORIGINAL HEIGHT OF THE BALES BECOMES FILLED WITH SEDIMENT.
- III. SEDIMENT BASINS/SEDIMENT TRAPS
A. CONTRACTOR TO KEEP WEEKLY CHECKLIST LOSS FOR INSPECTIONS OF ALL SEDIMENT AND EROSION CONTROL DEVICES AND HAVE THEM READILY AVAILABLE ON-SITE AT ALL TIMES FOR INSPECTION BY DEEP, LOCAL AUTHORITIES OR ENGINEER.
B. ALL SEDIMENT BASINS AND/OR SEDIMENT TRAPS SHALL BE INSPECTED FOLLOWING EACH RAINFALL. REPAIR OF SLOPES SHALL BE PROMPTLY MADE AS NEEDED.
C. SEDIMENT DEPOSITS SHALL BE REMOVED FROM SEDIMENT BASINS AND/OR SEDIMENT TRAPS WHEN THEY REACH A MAXIMUM HEIGHT OF ONE FOOT UNLESS OTHERWISE INDICATED ON THE EROSION CONTROL PLANS AND DETAILS TO BE AT A SPECIFIC ELEVATION PER CLEAN OUT MARKERS.
D. SEDIMENT SHALL BE DISPOSED OF ON-SITE OR AS DIRECTED BY THE ENGINEER AND LOCAL GOVERNING OFFICIALS. SEE SEDIMENT AND EROSION CONTROL NOTES HEREIN REGARDING DISPOSAL REQUIREMENTS FOR OFF SITE SPOIL DISPOSAL.

SEDIMENT AND EROSION CONTROL PLAN

- 1. ALL SEDIMENT AND EROSION CONTROL MEASURES WILL BE INSTALLED AT ALL CULVERT OUTLETS IF CULVERT OUTLETS ARE APPLICABLE TO THIS PROJECT AND SILTATION FENCE INSTALLED ALONG THE TOE OF ALL CRITICAL CUT AND FILL SLOPES.
- 2. CULVERT DISCHARGE AREAS WILL BE PROTECTED WITH RIP RAP CHANNELS. EROSION DISSIPATORS WILL BE INSTALLED AS SHOWN ON THESE PLANS AND AS NECESSARY.
- 3. CATCH BASINS WILL BE PROTECTED WITH HAY BALE/STRAW BALE FILTERS, SILT SACKS, SILTATION FENCE, OR OTHER INLET PROTECTION DEVICES PER DETAILS, THROUGHOUT THE CONSTRUCTION PERIOD AND UNTIL ALL DISTURBED AREAS ARE THOROUGHLY STABILIZED.
- 4. ALL SEDIMENT AND EROSION CONTROL MEASURES WILL BE INSTALLED IN ACCORDANCE WITH THE STANDARDS AND SPECIFICATIONS OF THE 2002 CONNECTICUT GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL, LATEST EDITION.
- 5. SEDIMENT AND EROSION CONTROL MEASURES WILL BE INSTALLED PRIOR TO DEMOLITION AND/OR CONSTRUCTION WHENEVER POSSIBLE.
- 6. ALL CONTROL MEASURES WILL BE MAINTAINED IN EFFECTIVE CONDITION THROUGHOUT THE DEMOLITION AND CONSTRUCTION PERIOD UNTIL THE SITE IS DETERMINED TO BE STABILIZED BY THE AUTHORITY HAVING JURISDICTION.
- 7. ADDITIONAL CONTROL MEASURES WILL BE INSTALLED DURING THE CONSTRUCTION PERIOD, IF NECESSARY OR REQUIRED OR AS DIRECTED BY THE CIVIL ENGINEER OR BY THE AUTHORITY HAVING JURISDICTION.
- 8. SEDIMENT REMOVED FROM EROSION CONTROL STRUCTURES WILL BE DISPOSED IN A MANNER WHICH IS CONSISTENT WITH THE INTENT AND REQUIREMENTS OF THE SEDIMENT AND EROSION CONTROL PLANS, NOTES, AND DETAILS.
- 9. THE CONTRACTOR IS ASSIGNED THE RESPONSIBILITY FOR IMPLEMENTING THIS SEDIMENT AND EROSION CONTROL PLAN. THIS RESPONSIBILITY INCLUDES THE INSTALLATION AND MAINTENANCE OF CONTROL MEASURES, INFORMING ALL PARTIES ENGAGED ON THE CONSTRUCTION SITE OF THE REQUIREMENTS AND OBJECTIVES OF THE PLAN, NOTIFICATION OF THE MUNICIPALITY AND/OR INLAND WETLANDS COMMISSION OFFICE OR AUTHORITY HAVING JURISDICTION OF ANY TRANSFER OF THIS RESPONSIBILITY AND FOR CONVEYING A COPY OF THE SEDIMENT AND EROSION CONTROL PLAN IF THE TITLE TO THE LAND IS TRANSFERRED.

SEDIMENT AND EROSION CONTROL NOTES

- 1. THE SEDIMENT AND EROSION CONTROL PLAN IS ONLY INTENDED TO DESCRIBE THE SEDIMENT AND EROSION CONTROL TREATMENT FOR THIS SITE. SEE SEDIMENT AND EROSION CONTROL DETAILS AND CONSTRUCTION SEQUENCE. REFER TO SITE PLAN FOR GENERAL INFORMATION AND OTHER CONTRACT PLANS FOR APPROPRIATE INFORMATION.
- 2. THE CONTRACTOR IS RESPONSIBLE FOR IMPLEMENTING THIS SEDIMENT AND EROSION CONTROL PLAN. THIS RESPONSIBILITY INCLUDES THE PROPER INSTALLATION AND MAINTENANCE OF SEDIMENT AND EROSION CONTROL MEASURES, INFORMING ALL PARTIES ENGAGED WITH CONSTRUCTION ON THE SITE OF THE REQUIREMENTS AND OBJECTIVES OF THIS PLAN, INFORMING THE AUTHORITY HAVING JURISDICTION OR COUNTY OR INLAND WETLANDS AGENCY OF ANY TRANSFER OF THIS RESPONSIBILITY, AND FOR CONVEYING A COPY OF THE SEDIMENT & EROSION CONTROL PLAN IF THE TITLE TO THE LAND IS TRANSFERRED.
- 3. AN EROSION CONTROL BOND MAY BE REQUIRED TO BE POSTED WITH THE MUNICIPALITY TO ENSURE IMPLEMENTATION OF THE SEDIMENT AND EROSION CONTROL MEASURES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE POSTING OF THIS BOND AND FOR INQUIRIES TO THE MUNICIPALITY FOR INFORMATION ON THE METHOD, TYPE AND AMOUNT OF THE BOND POSTING UNLESS OTHERWISE DIRECTED BY THE OWNER.
- 4. VISUAL SITE INSPECTIONS SHALL BE CONDUCTED WEEKLY, AND AFTER EACH MEASURABLE PRECIPITATION EVENT OF 0.25 INCHES OR GREATER BY QUALIFIED PERSONNEL, TRAINED AND EXPERIENCED IN SEDIMENT AND EROSION CONTROL, TO ASCERTAIN THAT THE SEDIMENT AND EROSION CONTROL (E&S) BMPs ARE OPERATIONAL AND EFFECTIVE IN PREVENTING POLLUTION. A WRITTEN REPORT OF EACH INSPECTION SHALL BE KEPT, AND INCLUDE:
A) A SUMMARY OF THE SITE CONDITIONS, E&S BMPs, AND COMPLIANCE; AND
B) THE DATE, TIME, AND THE NAME OF THE PERSON CONDUCTING THE INSPECTION
- 5. THE CONTRACTOR SHALL CONSTRUCT ALL SEDIMENT AND EROSION CONTROLS IN ACCORDANCE WITH 2002 CONNECTICUT GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL, LATEST EDITION IN ACCORDANCE WITH THE CONTRACT DOCUMENTS, AND AS DIRECTED BY THE MUNICIPALITY AND/OR INLAND WETLANDS COMMISSION. THE CONTRACTOR SHALL KEEP A COPY OF THE GUIDELINES ON-SITE FOR REFERENCE DURING CONSTRUCTION.
- 6. ADDITIONAL AND/OR ALTERNATIVE SEDIMENT AND EROSION CONTROL MEASURES MAY BE INSTALLED DURING THE CONSTRUCTION PERIOD IF FOUND NECESSARY BY THE CONTRACTOR, OWNER, SITE ENGINEER, MUNICIPALITY AND/OR INLAND WETLANDS COMMISSION, OR GOVERNING AGENCIES. THE CONTRACTOR SHALL CONTACT THE OWNER AND APPROPRIATE GOVERNING AGENCIES FOR APPROVAL IF ALTERNATIVE CONTROLS OTHER THAN THOSE SHOWN ON THE PLANS ARE PROPOSED.
- 7. THE CONTRACTOR SHALL INSPECT ALL SEDIMENT AND EROSION CONTROLS BEFORE AND AFTER EACH STORM (0.25 INCHES OR GREATER RAINFALL), OR AT LEAST WEEKLY, TO VERIFY THAT THE CONTROLS ARE OPERATING PROPERLY AND MAKE REPAIRS WHERE NECESSARY.
- 8. THE CONTRACTOR SHALL KEEP A SUPPLY OF SEDIMENT AND EROSION CONTROL MATERIAL (ANY HAY BALES, SILT FENCE,

JUTE MESH, RIP RAP, ETC.) ON-SITE FOR MAINTENANCE AND EMERGENCY REPAIRS.

- 9. PROTECT EXISTING TREES THAT ARE TO BE SAVED BY FENCING AT THE DRIP LINE OR AS SHOWN WITH SNOW FENCE, ORANGE SAFETY FENCE, OR EQUIVALENT FENCING. ANY LIMB TRIMMING SHOULD BE DONE BEFORE CONSTRUCTION BEGINS IN THAT AREA; FENCING SHALL BE MAINTAINED AND REPAIRED DURING CONSTRUCTION.
- 10. INSTALL PERIMETER SEDIMENT AND EROSION CONTROLS PRIOR TO CLEARING OR CONSTRUCTION. ALL CONSTRUCTION SHALL BE CONTAINED WITHIN THE LIMIT OF DISTURBANCE, WHICH SHALL BE MARKED WITH SILT FENCE, SAFETY FENCE, HAY BALES, ORANGE SAFETY FENCE, OR EQUIVALENT FENCING. CONSTRUCTION ACTIVITY SHALL REMAIN ON THE UPHILL SIDE OF THE SILT FENCE UNLESS WORK IS SPECIFICALLY CALLED FOR ON THE DOWNHILL SIDE OF THE FENCE.
- 11. ANY STONE CONSTRUCTION ENTRANCE ANTI-TRACKING PADS SHALL BE INSTALLED AT START OF CONSTRUCTION AND MAINTAINED THROUGHOUT THE DURATION OF CONSTRUCTION. THE LOCATION OF THE TRACKING PADS MAY CHANGE AS VARIOUS PHASES OF CONSTRUCTION ARE COMPLETED.
- 12. TOPSOIL SHALL BE STRIPPED AND STOCKPILED FOR USE IN FINAL LANDSCAPING. ALL EARTH STOCKPILES SHALL HAVE HAY BALES OR SILT FENCE AROUND THE LIMIT OF PILE. PILES SHALL BE TEMPORARILY SEEDDED IF PILE IS TO REMAIN IN PLACE FOR MORE THAN ONE (1) MONTH.
- 13. ANY SEDIMENT BASINS AND SEDIMENT TRAPS SHALL PROVIDE 134 CUBIC YARDS OF SEDIMENT STORAGE PER ACRE CONTRIBUTING TO THE BASIN. PROVIDE BASIN VOLUMES FOR ALL DISTURBANCE ON SITE.
- 14. COMPLY WITH REQUIREMENTS OF CGS SECTION 22A 430B, FOR STORMWATER DISCHARGE FROM CONSTRUCTION ACTIVITIES AND WITH DEEP RECORD KEEPING AND INSPECTION REQUIREMENTS.
- 15. ANY STONE CONSTRUCTION ENTRANCE ANTI-TRACKING PADS SHALL BE INSTALLED PRIOR TO ANY ON SITE EXCAVATION AND SHALL BE MAINTAINED DURING ALL DEMOLITION, EXCAVATION AND CONSTRUCTION ACTIVITIES.
- 16. MINIMIZE LAND DISTURBANCES. SEED AND MULCH DISTURBED AREAS WITH TEMPORARY MIX AS SOON AS PRACTICABLE (ONE WEEK MAXIMUM UNSTABILIZED PERIOD) USING PERENNIAL RYEGRASS AT 40 LBS PER ACRE. MULCH ALL CUT AND FILL SLOPES AND SWALES WITH LOOSE HAY AT A RATE OF 2 TONS PER ACRE. IF NECESSARY, REPLACE LOOSE HAY ON SLOPES WITH EROSION CONTROL BLANKETS OR JUTE CLOTH. MODERATELY GRADED AREAS, ISLANDS, AND TEMPORARY CONSTRUCTION STAGING AREAS MAY BE HYDROSEEDDED WITH TACKIFIER.
- 17. MAINTAIN EXISTING PAVED AREAS FOR CONSTRUCTION STAGING FOR AS LONG AS POSSIBLE.
- 18. SILT FENCE AND OTHER SEDIMENT AND EROSION CONTROL MEASURES SHALL BE INSTALLED IN ACCORDANCE WITH CONTRACT DRAWINGS AND MANUFACTURER'S RECOMMENDATIONS PRIOR TO WORK IN ANY UPLAND AREAS.
- 19. EXCAVATED MATERIAL FROM TEMPORARY SILT TRAPS MUST BE STOCKPILED ON UPHILL SIDE OF SILT FENCE.
- 20. INSTALL SILT FENCE ACCORDING TO MANUFACTURER'S INSTRUCTION, PARTICULARLY, BURY LOWER EDGE OF FABRIC INTO GROUND. SILT FENCE SHALL BE TENCATE ENVIROFENCE, PROPEX GETOX OR EQUIVALENT APPROVED BY THE CIVIL ENGINEER. FILTER FABRIC USED SHALL BE TENCATE 140N OR 170N, OR APPROVED EQUIVALENT. SEE SPECIFICATIONS FOR FURTHER INFORMATION.
- 21. WHERE INDICATED ON SEDIMENT AND EROSION CONTROL PLANS USE NEW HAY/STRAW BALES AND REPLACE THEM (ONE WEEK MAXIMUM UNSTABILIZED PERIOD) USING PERENNIAL RYEGRASS AT 40 LBS PER ACRE. MULCH ALL CUT AND FILL SLOPES AND SWALES WITH LOOSE HAY AT A RATE OF 2 TONS PER ACRE. IF NECESSARY, REPLACE LOOSE HAY ON SLOPES WITH EROSION CONTROL BLANKETS OR JUTE CLOTH. MODERATELY GRADED AREAS, ISLANDS, AND TEMPORARY CONSTRUCTION STAGING AREAS MAY BE HYDROSEEDDED WITH TACKIFIER.
- 22. INSTALL ANY TEMPORARY DIVERSION DITCHES, PLUNGE POOLS, SEDIMENT BASINS, SEDIMENT TRAPS, CONCRETE WASH PITS AND DEWATERING PITS AS SHOWN AND AS NECESSARY DURING VARIOUS PHASES OF CONSTRUCTION TO CONTROL RUNOFF UNTIL UPHILL AREAS ARE DETERMINED TO BE STABILIZED BY THE AUTHORITY HAVING JURISDICTION. LOCATION OF TEMPORARY SEDIMENT BASINS WILL REQUIRE REVIEW AND APPROVAL BY THE CIVIL ENGINEER AND AUTHORITY HAVING JURISDICTION.
- 23. DIRECT ALL DEWATERING PUMP DISCHARGE TO A SEDIMENT CONTROL DEVICE SUCH AS TEMPORARY PITS, SEDIMENT TRAP, SEDIMENT BASINS OR GRASS FILTERS WITHIN THE APPROVED LIMIT OF DISTURBANCE. DISCHARGE TO STORM DRAINAGE SYSTEM OR SURFACE WATERS FROM SEDIMENT CONTROLS SHALL BE CLEAR.
- 24. BLOCK THE OPEN UPSTREAM ENDS OF DETENTION BASIN/SEDIMENTATION BASIN OUTLET CONTROL ORIFICE UNTIL SITE IS STABILIZED. BLOCK END OF STORM SEWERS IN EXPOSED TRENCHES WITH BOARDS AND SANDBAGS AT THE END OF EACH WORKING DAY WHEN RAIN IS EXPECTED.
- 25. SWEEP AFFECTED PORTIONS OF OFF SITE ROADS ONE OR MORE TIMES A DAY (OR LESS FREQUENTLY IF TRACKING IS NOT A PROBLEM) DURING CONSTRUCTION. OTHER DUST CONTROL MEASURES TO BE USED AS NECESSARY INCLUDE WATERING DOWN DISTURBED AREAS, USING CALCIUM CHLORIDE, AND COVERING LOADS ON DUMP TRUCKS.
- 26. PERIODICALLY CHECK ACCUMULATED SEDIMENT LEVELS IN ANY SEDIMENT BASINS AND SEDIMENT TRAPS DURING CONSTRUCTION AND CLEAN AS SHOWN AND AS NECESSARY DURING VARIOUS PHASES OF CONSTRUCTION TO CONTROL RUNOFF OR PER SPECIFIC CLEANOUT MARKER ELEVATION. CLEAN ACCUMULATED SEDIMENT FROM CATCH BASIN SUMPS AS NECESSARY AND AS DIRECTED BY THE CIVIL ENGINEER OR OWNER'S CONSTRUCTION REPRESENTATIVE. REMOVE ACCUMULATED SEDIMENT FROM BEHIND HAY/STRAW BALES AND SILT FENCE WHEN LEVEL REACHES HALF THE HEIGHT OF THE BALE OR ONE FOOT AT SILT FENCE. DISPOSE OF SEDIMENT LEGALLY EITHER ON OR OFF SITE.
- 27. IMMEDIATELY UPON DISCOVERING UNFORESEEN CIRCUMSTANCES POSING THE POTENTIAL FOR ACCELERATED EROSION AND/OR SEDIMENT POLLUTION, THE OPERATOR SHALL IMPLEMENT APPROPRIATE BEST MANAGEMENT PRACTICES TO ELIMINATE THE POTENTIAL FOR ACCELERATED EROSION AND/OR SEDIMENT POLLUTION.
- 28. ALL PUMPING OF SEDIMENT LADEN WATER SHALL BE THROUGH A SEDIMENT CONTROL BMP, SUCH AS A PUMPED WATER FILTER BAG OR EQUIVALENT SEDIMENT REMOVAL FACILITY, OVER UNDISTURBED VEGETATED AREAS.
- 29. ALL EXCAVATED MATERIAL SHALL BE PLACED ON THE HIGH SIDE OF UTILITY AND STORM PIPE TRENCHES SO AS TO ALLOW THE TRENCH TO INTERCEPT ALL SILT LADEN RUNOFF.
- 30. CONTRACTOR SHALL ONLY EXCAVATE AS MUCH UTILITY AND STORM PIPE TRENCH WORK AS CAN BE COMPLETED, BACKFILLED AND STABILIZED IN ONE DAY SO AS TO LIMIT THE AMOUNT OF OPEN, DISTURBED TRENCHING.
- 31. ANY STOCKPILES OF STRIPPED MATERIALS ARE TO BE PERIODICALLY SPRAYED WITH WATER OR A CRUSTING AGENT TO STABILIZE POTENTIALLY WIND-BLOWN MATERIAL. HAUL ROADS BOTH INTO AND AROUND THE SITE ARE TO BE SPRAYED AS NEEDED TO SUPPRESS DUST. TRUCKS HAULING IMPORT FILL MATERIAL ARE TO BE TARPED TO AID IN THE CONTROL OF AIRBORNE DUST. DURING HIGH WIND EVENTS (20 TO 30 MPH SUSTAINED) CONSTRUCTION ACTIVITY SHALL BE LIMITED OR CEASED IF DUST CANNOT BE CONTROLLED BY WETTING.
- 32. AN AREA SHALL BE CONSIDERED TO HAVE ACHIEVED FINAL STABILIZATION WHEN IT HAS A MINIMUM OF 70% UNIFORM PERENNIAL VEGETATIVE COVER OR OTHER PERMANENT NON-VEGETATIVE COVER WITH A DENSITY SUFFICIENT TO RESIST ACCELERATED SURFACE EROSION AND SUBSURFACE CHARACTERISTICS SUFFICIENT TO RESIST SLIDING OR OTHER MOVEMENTS UNLESS OTHERWISE DETERMINED BY THE AUTHORITY HAVING JURISDICTION.
- 33. MAINTAIN ALL PERMANENT AND TEMPORARY EROSION AND SEDIMENT CONTROL DEVICES IN EFFECTIVE CONDITION THROUGHOUT THE CONSTRUCTION PERIOD. UPON COMPLETION OF WORK SWEEP PARKING LOT AND REMOVE ALL TEMPORARY EROSION AND SEDIMENT CONTROLS WHEN AUTHORIZED BY AUTHORITY HAVING JURISDICTION. FILE NOT (NOTICE OF TERMINATION) WITH AUTHORITY HAVING JURISDICTION RESPONSIBLE FOR REGULATING STORM WATER DISCHARGES FROM CONSTRUCTION ACTIVITIES PER NPDES.

STATE SPECIAL CONCERN SPECIES

- 1. CONTRACTOR IS RESPONSIBLE FOR HIRING A QUALIFIED HERPETOLOGIST TO WORK WITH CONSTRUCTION CREW TO ENSURE THAT TURTLES WILL NOT BE UNINTENTIONALLY KILLED DURING THE MOVING OF HEAVY EQUIPMENT, ESPECIALLY IN THE MONTH OF JUNE.
- 2. THE LIMIT OF DISTURBANCE SHALL BE FENCED WITH EXCLUSIONARY FENCING THAT IS SECURED AND IN CONTACT WITH THE GROUND AND AT LEAST 30 INCHES HIGH. THE FENCE SHALL BE MAINTAINED BI-WEEKLY AND AFTER MAJOR WEATHER EVENTS. DO NOT USE PLASTIC NETTED OR NETTED SILT FENCE.
- 3. ALL STAGING AND STORAGE AREAS, OUTSIDE OF PREVIOUSLY PAVED LOCATIONS, REGARDLESS OF THE DURATION OF TIME THEY WILL BE UTILIZED, MUST BE REVIEWED TO REMOVE INDIVIDUALS AND EXCLUDE THEM FROM RE-ENTRY.
- 4. ALL CONSTRUCTION PERSONNEL WORKING WITHIN THE TURTLE HABITAT MUST BE APPRISED OF THE SPECIES DESCRIPTION AND THE POSSIBLE PRESENCE OF A LISTED SPECIES, AND INSTRUCTED TO RELOCATE TURTLES FOUND INSIDE WORK AREAS OR NOTIFY THE APPROPRIATE AUTHORITIES TO RELOCATE INDIVIDUALS.
- 5. ANY TURTLES ENCOUNTERED WITHIN THE IMMEDIATE WORK AREA SHALL BE CAREFULLY MOVED TO AN ADJACENT AREA OUTSIDE OF THE EXCLUDED AREA AND FENCING SHOULD BE INSPECTED TO IDENTIFY AND REMOVE ACCESS POINT.
- 6. IN AREAS WHERE SILT FENCE IS USED FOR EXCLUSION, IT SHALL BE REMOVED AS SOON AS THE AREA IS STABLE TO ALLOW FOR REPTILE AND AMPHIBIAN PASSAGE TO RESUME.
- 7. NO HEAVY MACHINERY OR VEHICLES MAY BE PARKED IN ANY TURTLE HABITAT.
- 8. SPECIAL PRECAUTIONS MUST BE TAKEN TO AVOID DEGRADATION OF WETLAND HABITATS INCLUDING ANY WET MEADOWS AND SEASONAL POOLS.
- 9. THE CONTRACTOR AND CONSULTING HERPETOLOGIST MUST SEARCH THE WORK AREA EACH MORNING PRIOR TO ANY WORK BEING DONE.
- 10. WHEN FELLING TREES ADJACENT TO BROOKS AND STREAMS PLEASE CUT THEM TO FALL AWAY FROM THE WATERWAY AND DO NOT DRAG TREES ACROSS THE WATERWAY OR REMOVE STUMPS FROM BANKS.
- 11. AVOID AND LIMIT ANY EQUIPMENT USE WITHIN 50 FEET OF STREAMS AND BROOKS.
- 12. ANY CONFIRMED TURTLE SIGHTINGS SHOULD BE REPORTED TO THE NATURAL DIVERSITY DATA BASE AT (nddbrequestdep@ct.gov) USING REPORTING FORMS FOUND ON THE NDDB WEBSITE



100 Constitution Plaza
10th Floor
Hartford, CT 06103
(860) 249-2200
(860) 249-2400 Fax



PROPOSED RETAIL DEVELOPMENT
1100 BOSTON TURNPIKE
BOLTON, CONNECTICUT

REVISIONS

No.	Date	Desc.
1.	05/05/2021	REVISED PER TOWN COMMENTS

Designed	S.E.L.
Drawn	S.E.L.
Reviewed	K.M.M.
Scale	NONE
Project No.	2002032
Date	04/02/2021
CAD File:	EC200203201

Title
SEDIMENT AND EROSION CONTROL NOTES
Sheet No.

**FOR PERMITTING PURPOSES ONLY
NOT RELEASED FOR CONSTRUCTION**

EC-2

BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT

DATE APPROVED _____	DATE OF EXPIRATION _____
CHAIRMAN _____	

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPRES ON _____

LANDSCAPE ZONING INFORMATION

LOCATION: BOLTON, TOLLAND COUNTY, CONNECTICUT
 ZONE: RURAL MIXED USE ZONE (RMUZ)
 USE: RETAIL (PERMITTED BY SPECIAL PERMIT)

ITEM #	ITEM	REQUIREMENTS	PROPOSED	VARIANCE
1	LANDSCAPE AREA (SEC.11.J)	NO LESS THAN 30% OF AN RMUZ ZONE SHALL BE LANDSCAPED. SIDEWALKS ARE EXCLUDED FROM LANDSCAPE AREA	GREATER THAN 30% LANDSCAPED	NO
2	LANDSCAPE PARKING (SEC.15.H)	INTERIOR LANDSCAPING SHALL BE PROVIDED AT A RATE OF 20 SF PER PARKING SPACE. LANDSCAPING SHALL BE WITHIN RAISED, CURBED ISLANDS. (20 SF X 33 SPACES = 660 SF)	730 SF PROPOSED	NO
3	LANDSCAPE PARKING (SEC.15.H)	PARKING AREAS ABUTTING A RESIDENTIAL ZONE SHALL BE SCREENED BY A 10' WIDTH EVERGREEN ROW. PLANTS TO BE 4' HT AND 4' O.C. AT TIME OF PLANTING.	COMPLIES	NO
4	LANDSCAPE REQUIREMENTS (SEC.16A.3.q.3)	INTERIOR LANDSCAPE AREAS SHALL BE 100 SF MIN AND 8' WIDTH MIN.	COMPLIES	NO
5	LANDSCAPE REQUIREMENTS (SEC.16A.3.q.3)	INTERIOR AREAS SHALL HAVE 1 TREE PER 20 PARKING SPACES	COMPLIES	NO
6	LANDSCAPE REQUIREMENTS (SEC.16A.3.q.3)	PARKING PERIMETER LANDSCAPE AREA SHALL BE 5' WIDTH MIN. WITH 1 TREE PER 50 LF	COMPLIES	NO
7	LANDSCAPE REQUIREMENTS (SEC.16A.3.q.3)	TREES TO BE 3" CAL. AND 10' HT. MIN. AT TIME OF PLANTING	COMPLIES	NO
8	STREET PLANTINGS (SEC.16A.3.q.4)	LANDSCAPE ADJACENT TO STREET TO BE 30' WIDTH WITH 1 TREE PER 40' LOT LINE FRONTAGE (260 LF FRONTAGE ÷ 40 = 6.5 TREES)	5 TREES PROPOSED, 2 TREES TO REMAIN	NO
9	LANDSCAPE DESIGN (SEC.16A.3.x.3.g.10)	FOR EVERY 5 PARKING SPACES, 1 TREE SHALL BE PROVIDED (33 PARKING SPACES ÷ 5 = 6.6 TREES)	GREATER THAN 7 TREES PROVIDED	NO
10	LANDSCAPE DESIGN GUIDELINES (CH.8.1.3)	PLANT MATERIAL TO BE INDIGENOUS TO THE AREA, OR IF NOT NATIVE, THAN HARDY AND NON-INVASIVE	COMPLIES	NO
11	LANDSCAPE DESIGN GUIDELINES (CH.8.1.20&21)	ALL PLANTINGS SHALL BE GUARANTEED FOR 2 YEARS MINIMUM. A COPY OF THE GUARANTEE CONTRACT SHALL BE SUBMITTED TO THE TOWN.	SEE LANDSCAPE NOTE #4 ON SHEET LL-2	NO
12	LANDSCAPE DESIGN GUIDELINES (CH.8.1.22)	FLOWERING TREES TO BE 2"-2.5" CAL./DECIDUOUS TREES 3"-3.5" CAL./EVERGREEN TREES TO BE 5'-7' HT. MIN./DECIDUOUS SHRUBS 24" HT./EVERGREEN SHRUBS 18" HT./PERENNIALS 1 GAL. CONT.	COMPLIES	NO

LANDSCAPE PLANT SCHEDULE

KEY	QTY	BOTANICAL NAME	COMMON NAME	ROOT	SIZE AT INSTALL	SIZE AT MATURITY	COMMENTS
TREES							
AR	3	<i>Acer rubrum</i> 'Frankred'	RED SUNSET RED MAPLE	B&B	3" CAL. MIN.	45' x 35'	7' BRANCH HT. MIN.
AS	3	<i>Acer saccharum</i>	SUGAR MAPLE	B&B	3" CAL. MIN.	45' x 40'	7' BRANCH HT. MIN.
BN	3	<i>Betula nigra</i> 'Cully'	HERITAGE RIVER BIRCH	B&B	10' HT. MIN.	40' x 30'	MULTI-STEM
CO	4	<i>Celtis occidentalis</i> 'Prairie Pride'	PRAIRIE PRIDE HACKBERRY	B&B	3" CAL. MIN.	45' x 35'	7' BRANCH HT. MIN.
PG	3	<i>Picea glauca</i>	WHITE SPRUCE	B&B	6' HT. MIN.	50' x 15'	FULL BRANCHING TO GROUND
PS	8	<i>Pinus strobus</i>	EASTERN WHITE PINE	B&B	6' HT. MIN.	60' x 30'	FULL BRANCHING TO GROUND
PA	4	<i>Platanus x acerifolia</i> 'Morton's Circle'	EXCLAMATION! PLANETREE	B&B	3" CAL. MIN.	55' x 35'	7' BRANCH HT. MIN.
QR	3	<i>Quercus rubra</i>	RED OAK	B&B	3" CAL. MIN.	50' x 45'	7' BRANCH HT. MIN.
QP	3	<i>Quercus palustris</i>	PIN OAK	B&B	3" CAL. MIN.	55' x 40'	7' BRANCH HT. MIN.
TG	20	<i>Thuja</i> 'Green Giant'	GREEN GIANT ARBORVITAE	B&B	6' HT. MIN.	50' x 15'	FULL BRANCHING TO GROUND
SHRUBS							
CS	7	<i>Cornus sericea</i> 'Arctic Fire'	ARCTIC FIRE REDTWG DOGWOOD	CONT.	24" HT. MIN.	3.5' x 3.5'	PLANT 4' O.C.
IG	17	<i>Ilex glabra</i>	INKBERRY	CONT.	4' HT. MIN.	7' x 6'	PLANT 4' O.C.
IGC	27	<i>Ilex glabra</i> 'Compacta'	COMPACT INKBERRY	CONT.	24" HT. MIN.	4' x 5'	PLANT 4' O.C.
MP	7	<i>Myrica pensylvanica</i>	BAYBERRY	CONT.	30" HT. MIN.	8' x 8'	PLANT 5' O.C.
RC	8	<i>Rhododendron</i> 'Cunningham's White'	CUNNINGHAM'S WHITE RHODODENDRON	CONT.	24" HT. MIN.	3' x 4'	PLANT 4' O.C.
RH	6	<i>Rhododendron</i> 'Lavender Princess'	LAVENDER PRINCESS RHODODENDRON	CONT.	24" HT. MIN.	4' x 5'	PLANT 4' O.C.
ORNAMENTAL GRASSES							
PV	24	<i>Panicum virgatum</i> 'Shenandoah'	SHENANDOAH SWITCHGRASS	CONT.	24" HT. MIN.	4' x 2'	PLANT 30" O.C.
SH	27	<i>Sporobolus heterolepis</i>	PRARIE DROPSEED	CONT.	12" HT. MIN.	2.5' x 2.5'	PLANT 30" O.C.
PERENNIALS AND GROUNDCOVERS							
AM	15	<i>Aronia melanocarpa</i> 'UConnAM165'	LOW SCAPE MOUND CHOKEBERRY	CONT.	12" HT. MIN.	2' x 3'	PLANT 30" O.C.
CV	10	<i>Coreopsis verticillata</i> 'Grandiflora'	GRANDIFLORA COREOPSIS	CONT.	8" HT. MIN./1 GAL. CONT.	2.5' x 2.5'	PLANT 30" O.C.
RF	16	<i>Rudbeckia fulgida</i> 'Goldstrum'	BLACK-EYED SUSAN	CONT.	8" HT. MIN./1 GAL. CONT.	2.5' x 2.5'	PLANT 30" O.C.

NOTES:
 1) ALL SUBSTITUTIONS MUST RECEIVE APPROVAL FROM THE LANDSCAPE ARCHITECT PRIOR TO DELIVERY TO SITE.
 2) PROVIDE AND INSTALL ALL PLANTS SHOWN ON THE PLANTING PLAN DRAWINGS; THE QUANTITIES IN THE PLANT LIST ARE PROVIDED FOR THE CONTRACTOR'S CONVENIENCE ONLY. IF DISCREPANCIES OCCUR, THE LARGER QUANTITY SHALL APPLY.
 3) IF THERE IS A DISCREPANCY BETWEEN BOTANICAL AND COMMON NAME, BOTANICAL NAME PREVAILS.

SEE SHEET LL-2 FOR LANDSCAPE NOTES AND DETAILS

BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT

DATE APPROVED _____ DATE OF EXPIRATION _____

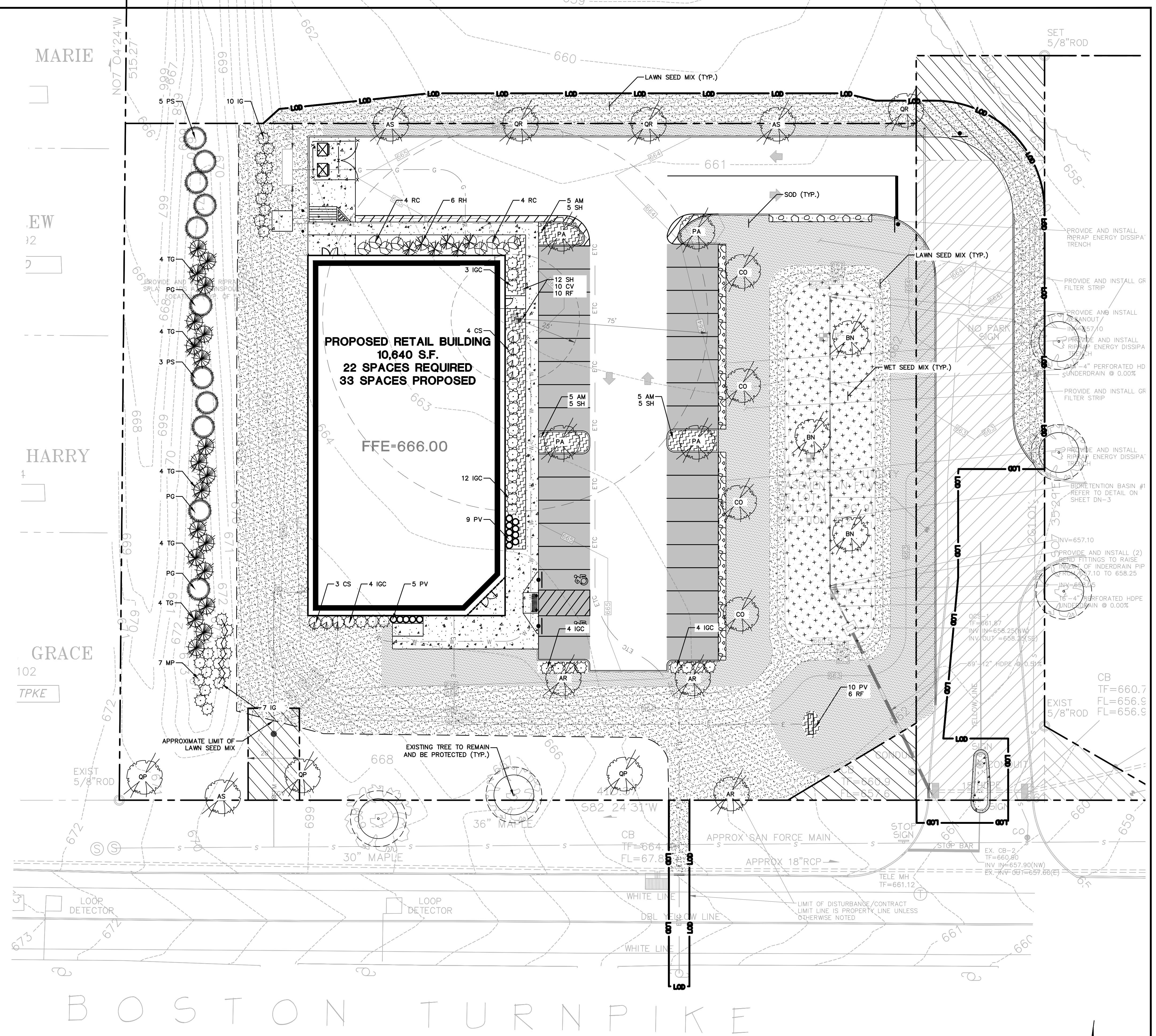
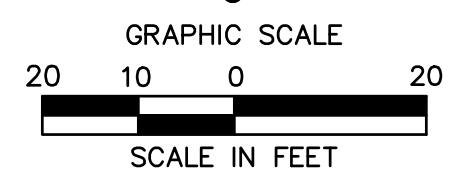
CHAIRMAN _____

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON _____

LEGEND

PATTERN	DESCRIPTION	PATTERN	DESCRIPTION	PATTERN	DESCRIPTION
[Pattern]	SOD (REFER TO SEED MIXES ON SHEET LL-2)	[Pattern]	PERENNIALS/GROUNDCOVERS (REFER TO PLANT SCHEDULE THIS PAGE)	[Pattern]	EXISTING TREE TO REMAIN AND BE PROTECTED
[Pattern]	LAWN SEED MIX (REFER TO SEED MIXES ON SHEET LL-2)	[Pattern]	WET SEED MIX (REFER TO SEED MIXES ON SHEET LL-2)	[Pattern]	APPROXIMATE LIMIT OF LAWN SEED MIX WITHIN PROPERTY INTERIOR

**FOR PERMITTING PURPOSES ONLY
 NOT RELEASED FOR CONSTRUCTION**



100 Constitution Plaza
 10th Floor
 Hartford, CT 06103
 (860) 249-2200
 (860) 249-2400 Fax



PROPOSED RETAIL DEVELOPMENT
 1100 BOSTON TURNPIKE
 BOLTON, CONNECTICUT

REVISIONS
 No. 1
 Date 03/05/2021
 Desc. REVISED PER TOWN COMMENTS

Designed L.M.W.
 Drawn L.M.W.
 Reviewed W.E.V.
 Scale 1"=20'
 Project No. 2002032
 Date 04/02/2021
 CAD File: LL200203201

Title
LANDSCAPE PLAN

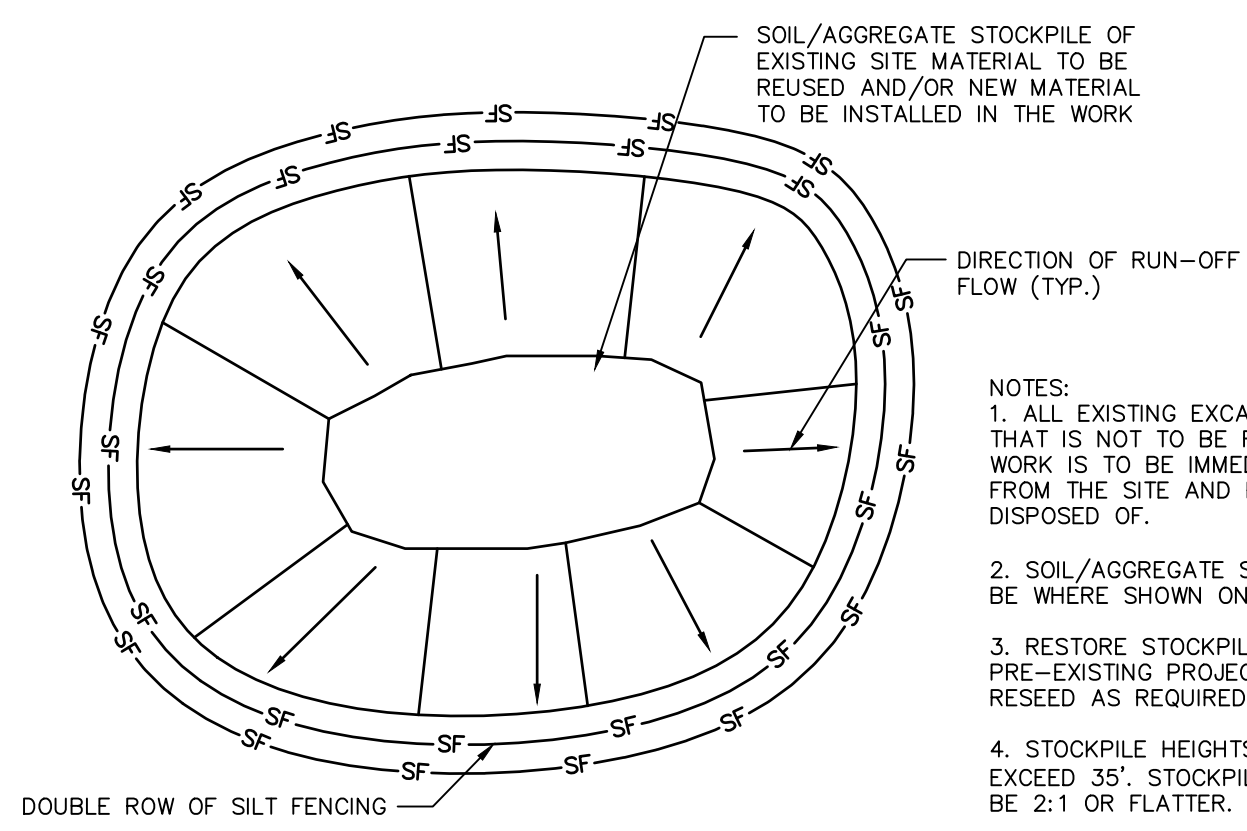
Sheet No.

LL-1

REVISIONS	Desc.	REVISED PER TOWN COMMENTS
No. 1	Date 03/05/2021	

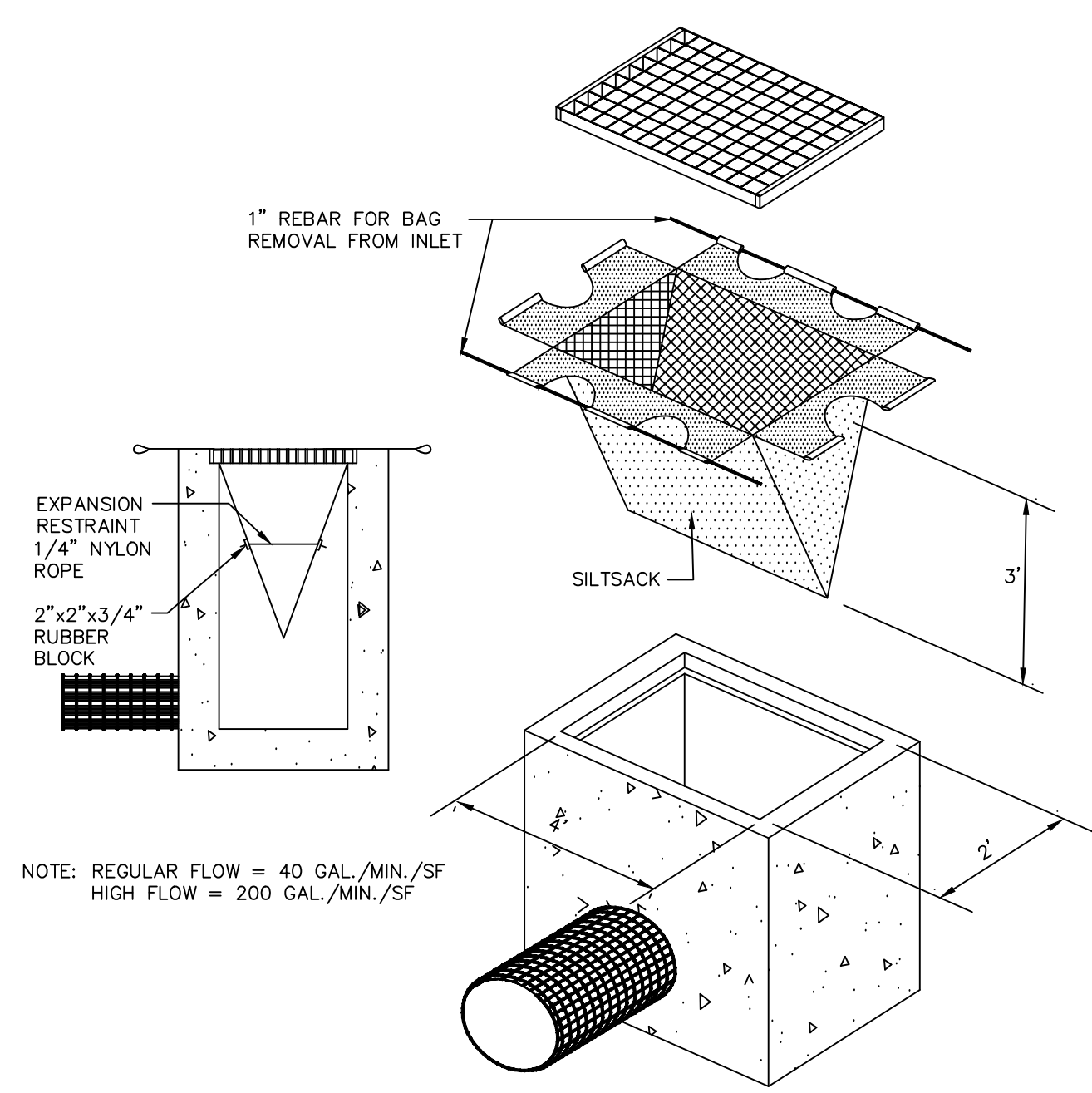
Designed	S.E.L.
Drawn	S.E.L.
Reviewed	K.M.M.
Scale	NONE
Project No.	2002032
Date	04/02/2021
CAD File	DN200203201

Title
DETAILS SHEET
Sheet No.

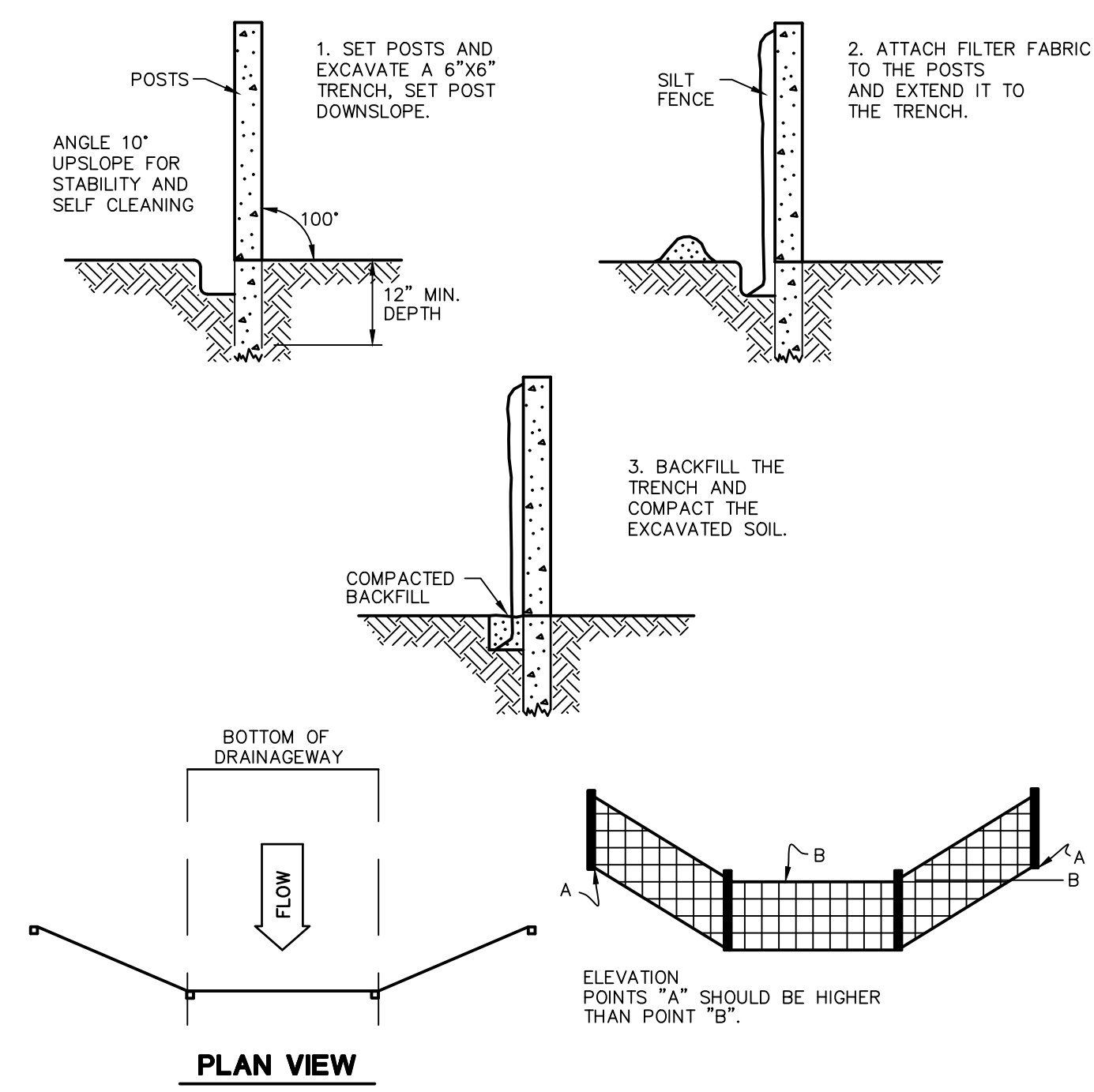


- NOTES:
1. ALL EXISTING EXCAVATED MATERIAL THAT IS NOT TO BE REUSED IN THE WORK IS TO BE IMMEDIATELY REMOVED FROM THE SITE AND PROPERLY DISPOSED OF.
 2. SOIL/AGGREGATE STOCKPILE SITES TO BE WHERE SHOWN ON THE DRAWINGS.
 3. RESTORE STOCKPILE SITES TO PRE-EXISTING PROJECT CONDITION AND RESEED AS REQUIRED.
 4. STOCKPILE HEIGHTS MUST NOT EXCEED 35'. STOCKPILE SLOPES MUST BE 2:1 OR FLATTER.

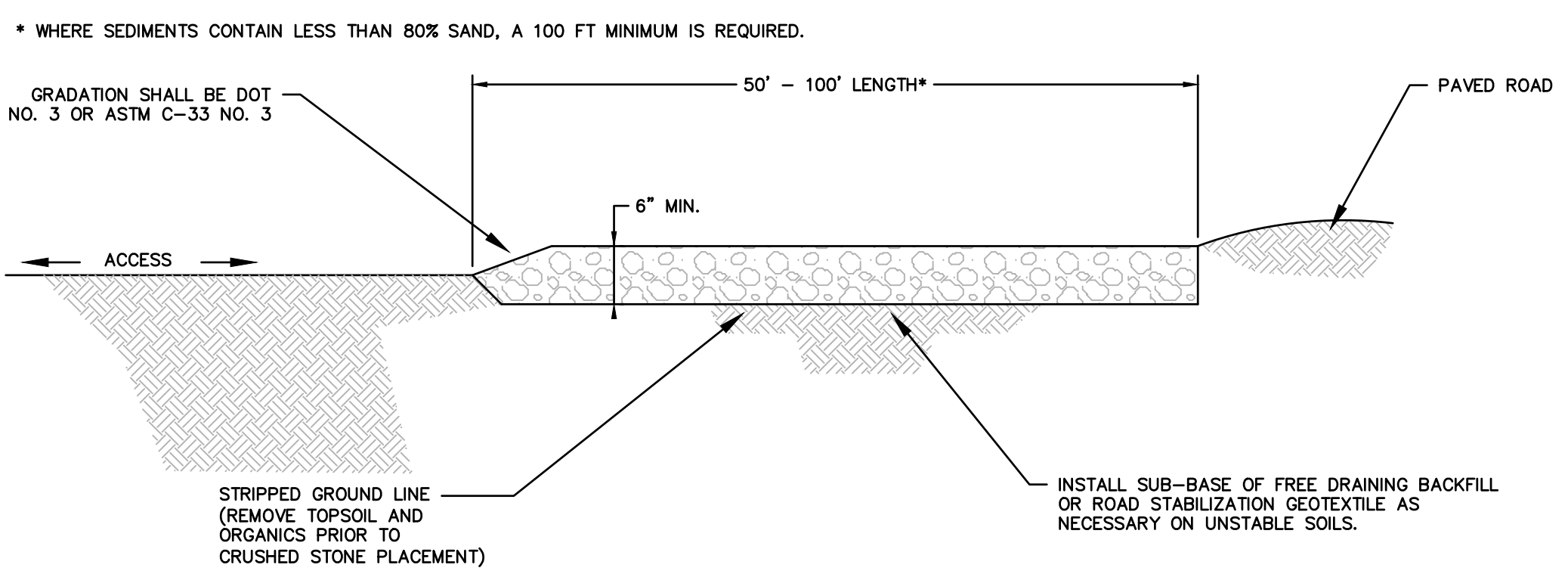
MATERIALS STOCKPILE DETAIL
N.T.S. BLEC-006



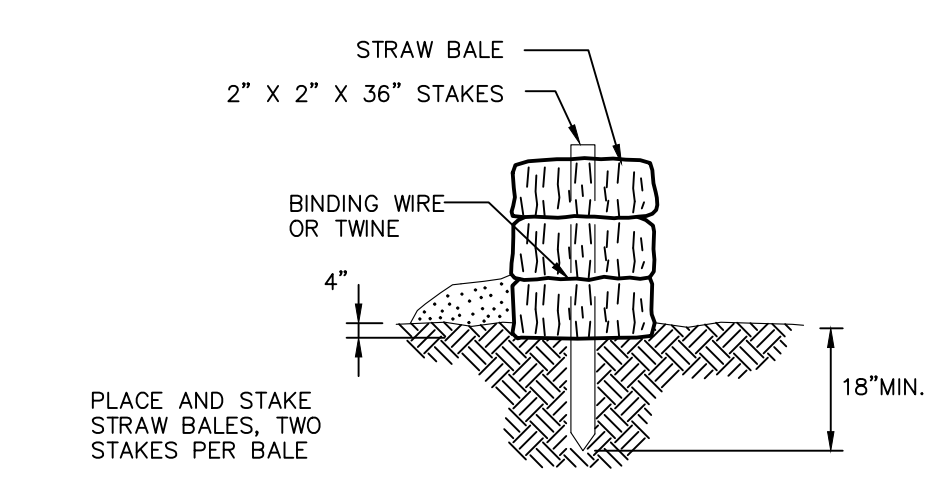
SILTSACK DETAIL
N.T.S. BLEC-005



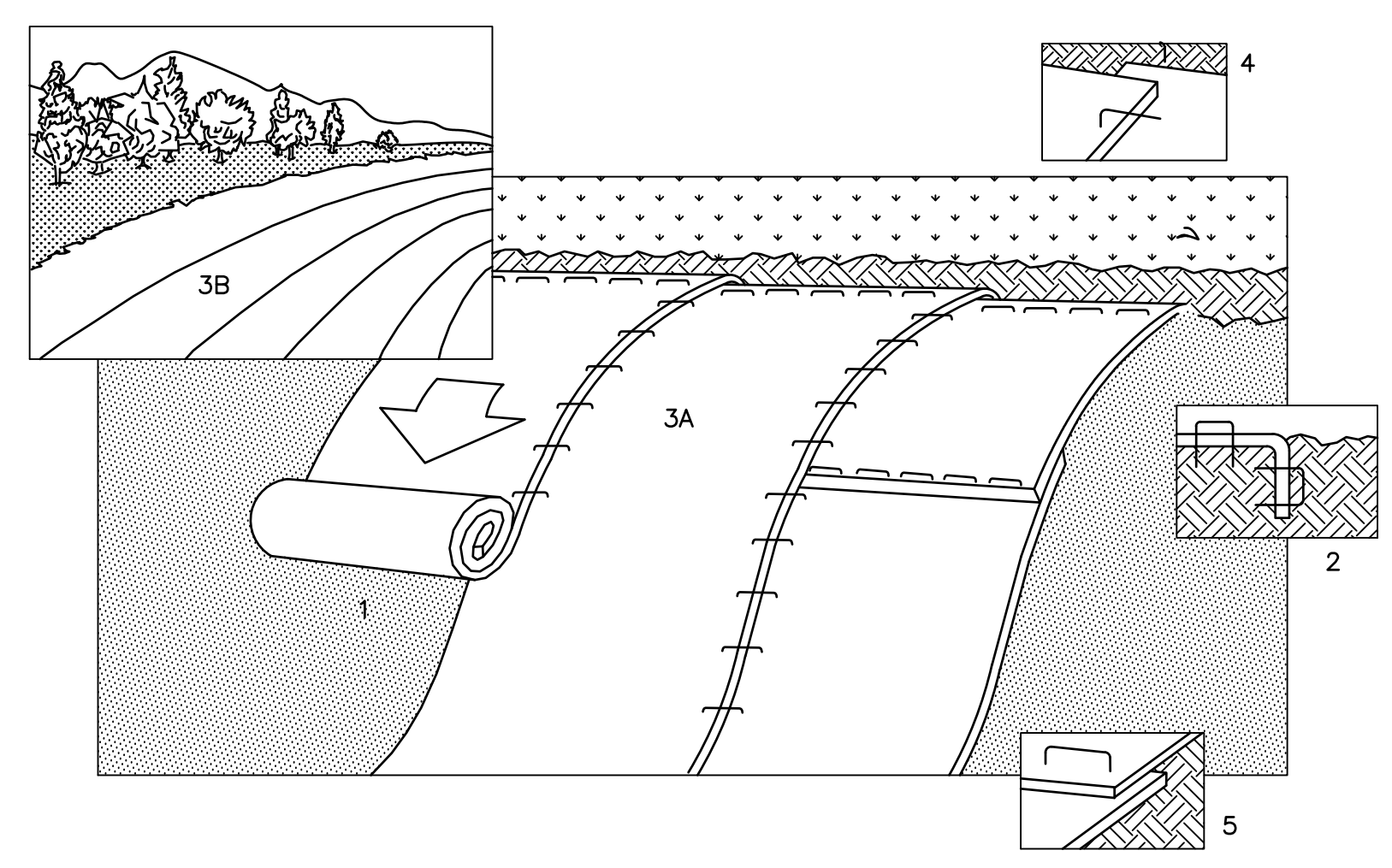
SILT FENCE BARRIER
N.T.S. CTEC-003



TYPICAL CONSTRUCTION ENTRANCE
N.T.S. CT DEEP CE-2

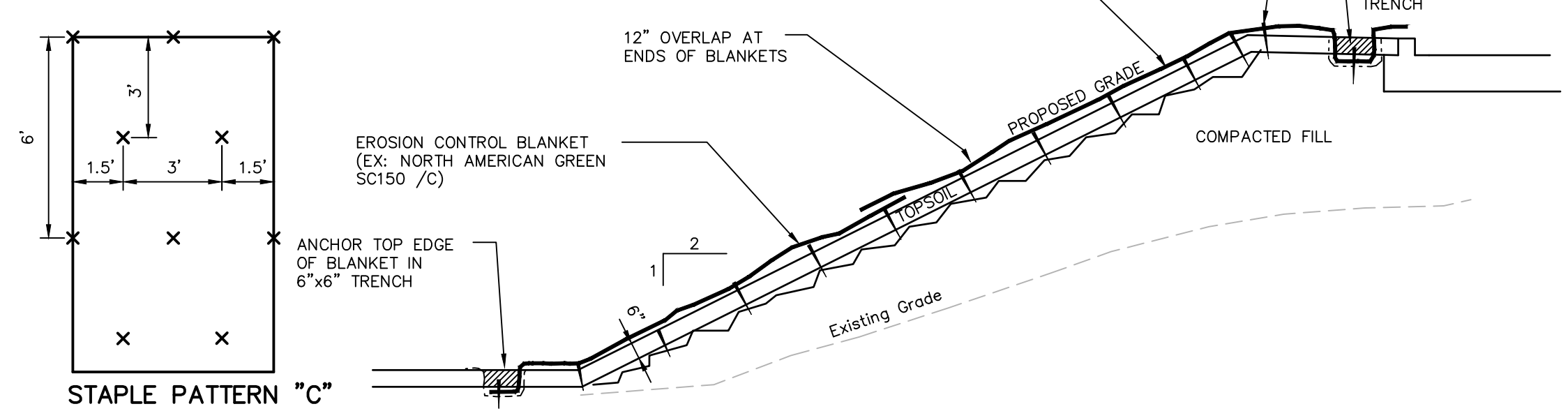


STRAW BALE DETAIL
N.T.S. BLEC-007

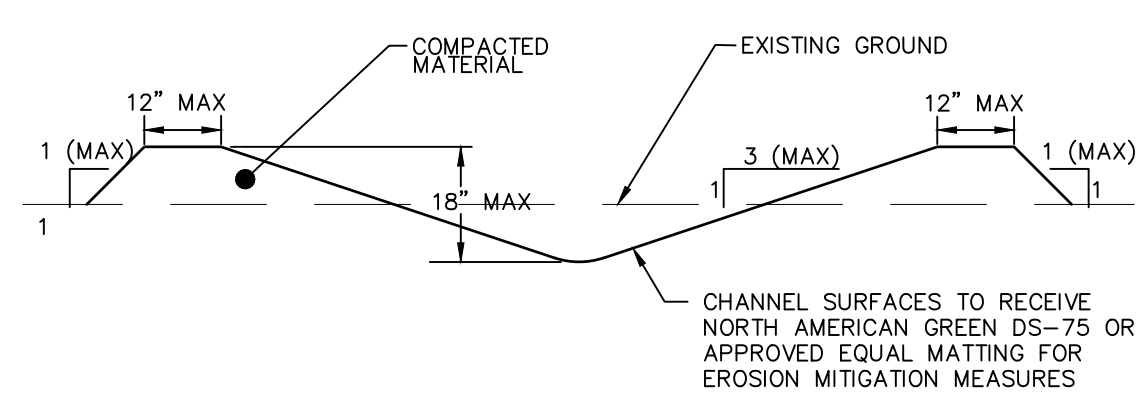


1. PREPARE SOIL BEFORE INSTALLING BLANKETS, INCLUDING APPLICATION OF LIME, FERTILIZER, AND SEED.
2. BEGIN AT THE TOP OF THE SLOPE BY ANCHORING THE BLANKET IN 6" DEEP X 6" WIDE TRENCH. BACKFILL AND COMPACT THE TRENCH AFTER STAPLING.
3. ROLL THE BLANKETS (A) DOWN OR (B.) HORIZONTALLY ACROSS THE SLOPE.
4. THE EDGES OF PARALLEL BLANKETS MUST BE STAPLED WITH APPROXIMATELY 2" OVERLAP.
5. WHEN BLANKETS MUST BE SPLICED DOWN THE SLOPE, PLACE BLANKETS END OVER END (SHINGLE STYLE) WITH APPROXIMATELY 4" OVERLAP. STAPLE THROUGH OVERLAPPED AREA, APPROXIMATELY 12" APART.

SLOPE STABILIZATION DETAIL
N.T.S. BLEC-010



BLANKET ON FILL SLOPE
N.T.S. BLEC-009



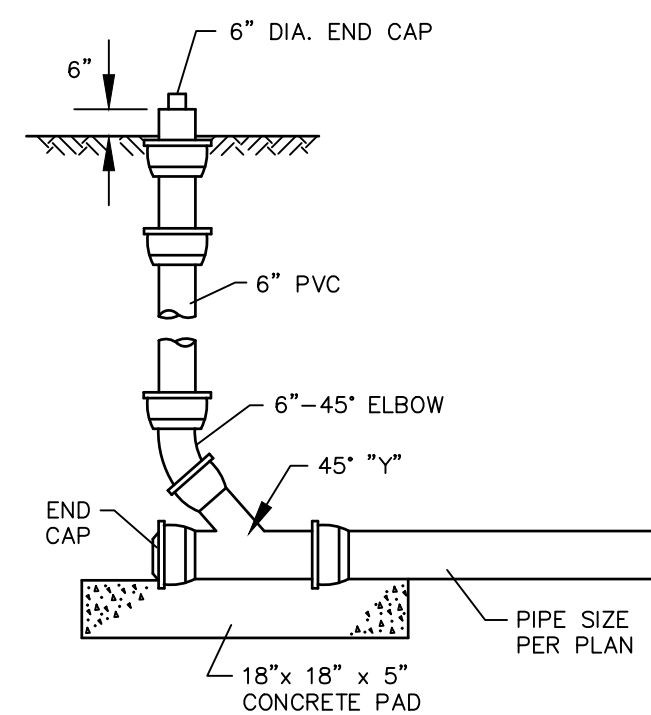
NON-ENGINEERED TEMPORARY DIVERSION DITCH DETAIL

BOLTON PLANNING AND ZONING COMMISSION

DATE APPROVED _____ DATE OF EXPIRATION _____

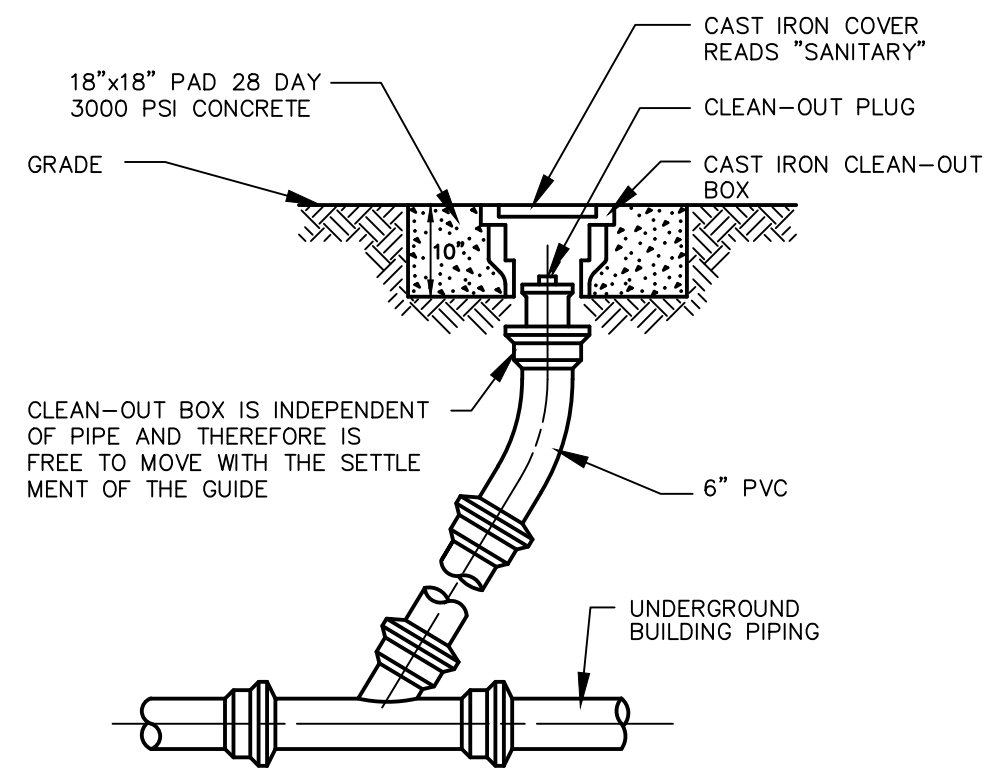
CHAIRMAN _____

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON _____



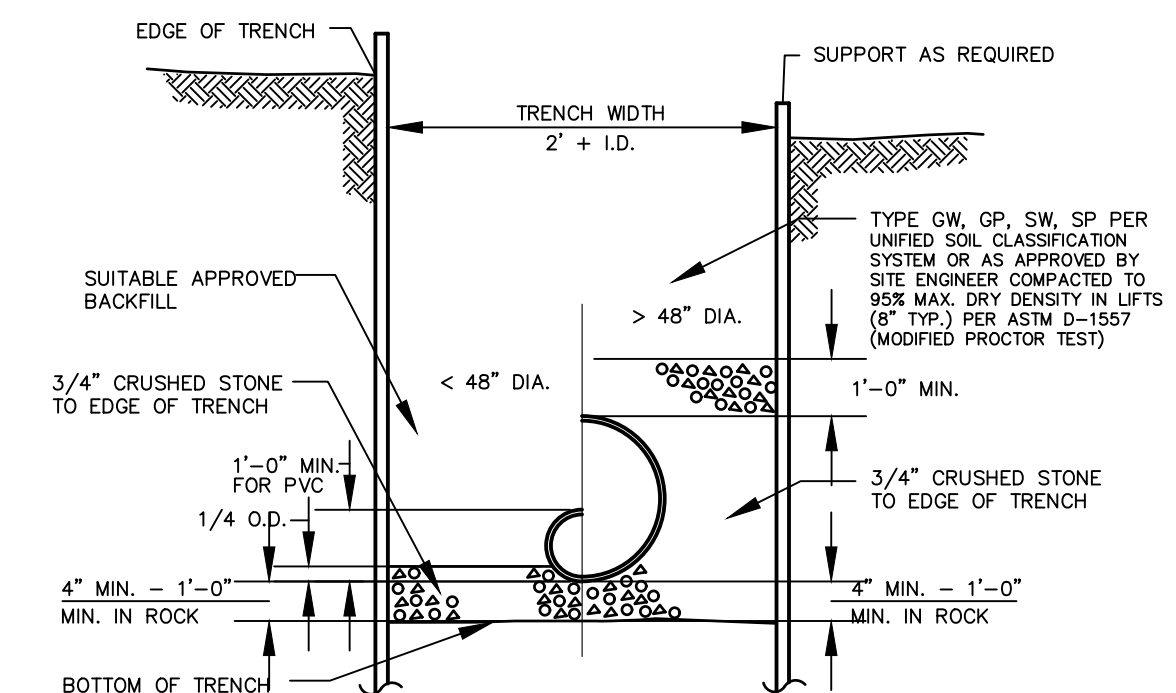
CLEANOUT IN LANDSCAPED AREA

N.T.S. BLS-007



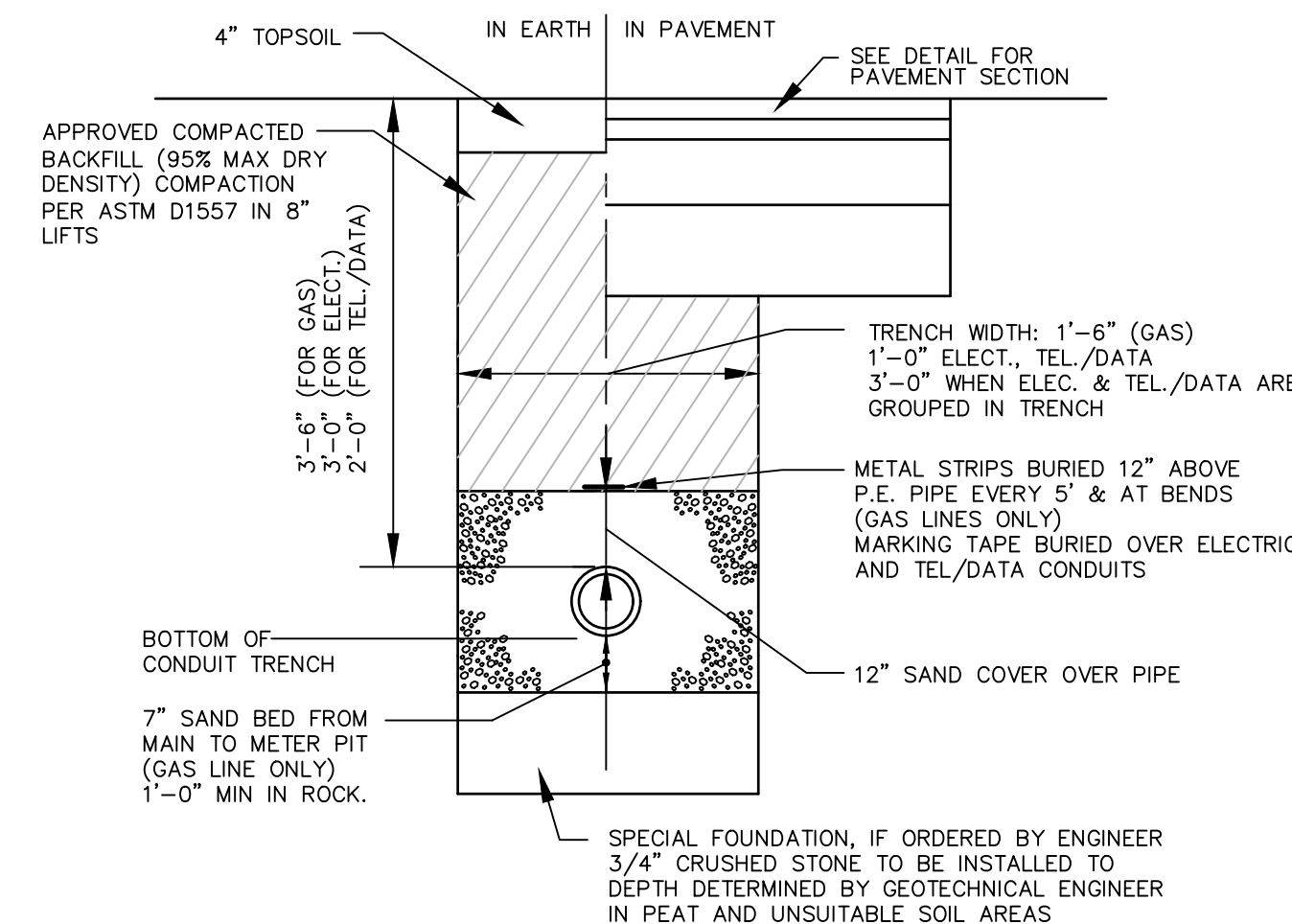
CLEANOUT IN PAVED AREA

N.T.S. BLS-008



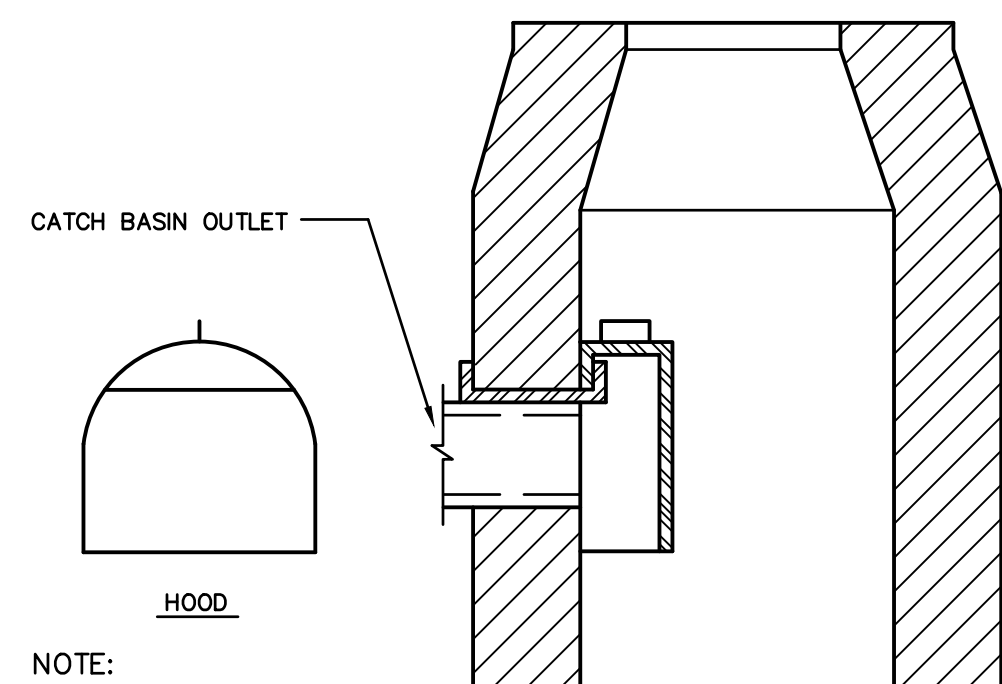
TYPICAL STORM SEWER TRENCH SECTION

N.T.S. BLD-004



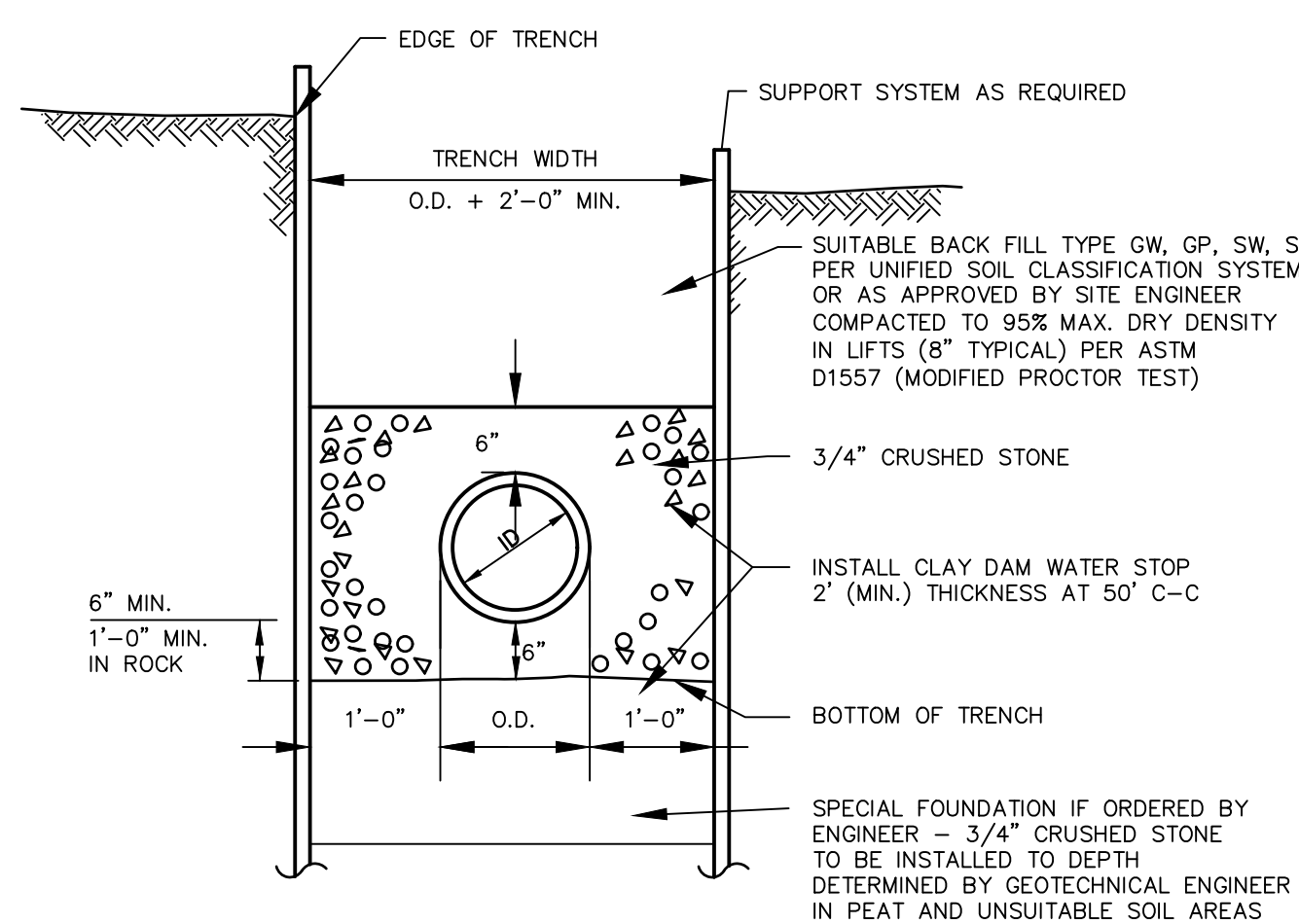
ELECTRICAL, TELEPHONE AND GAS TRENCH DETAIL

N.T.S. BLUD-001



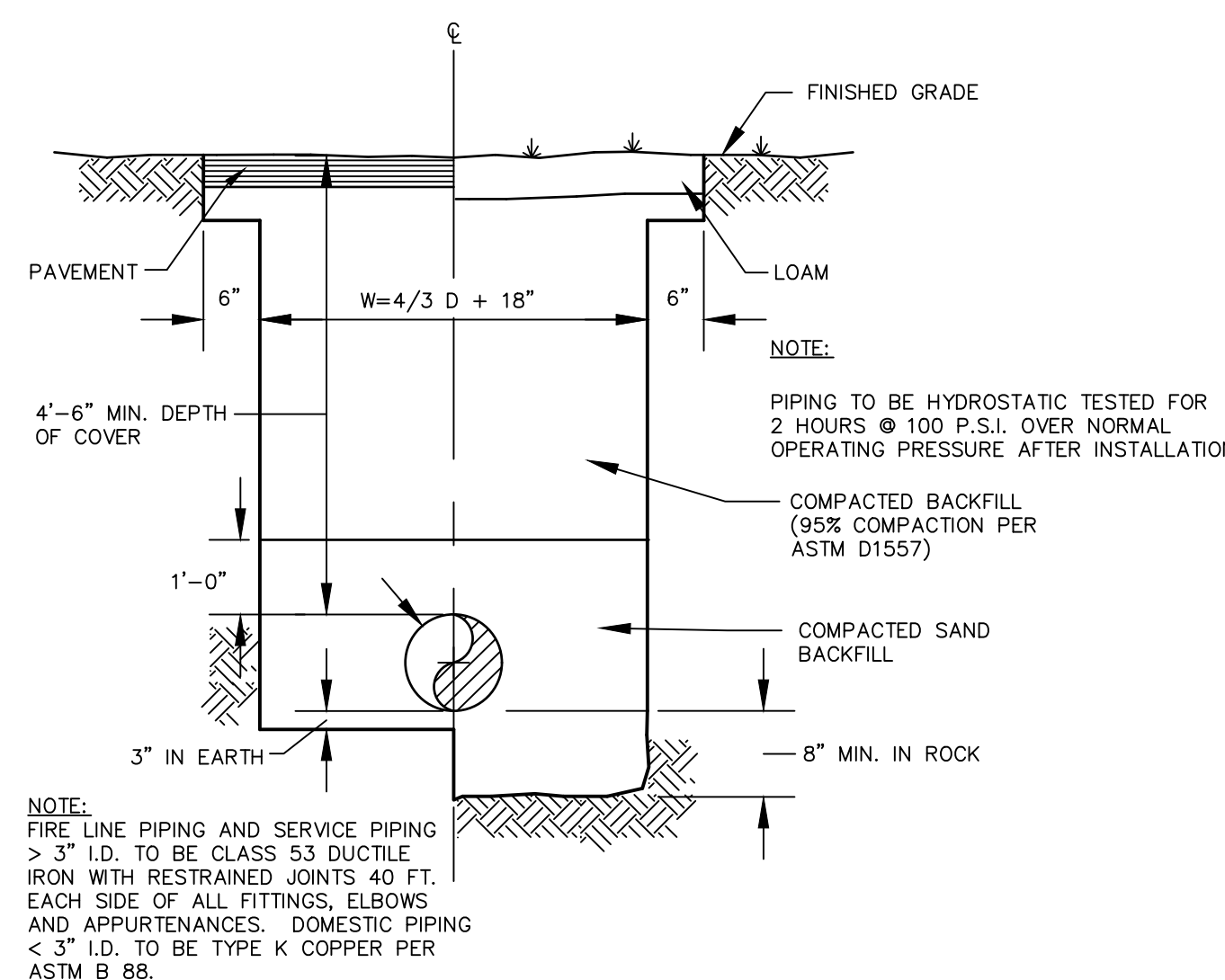
HOODED OUTLET

N.T.S.



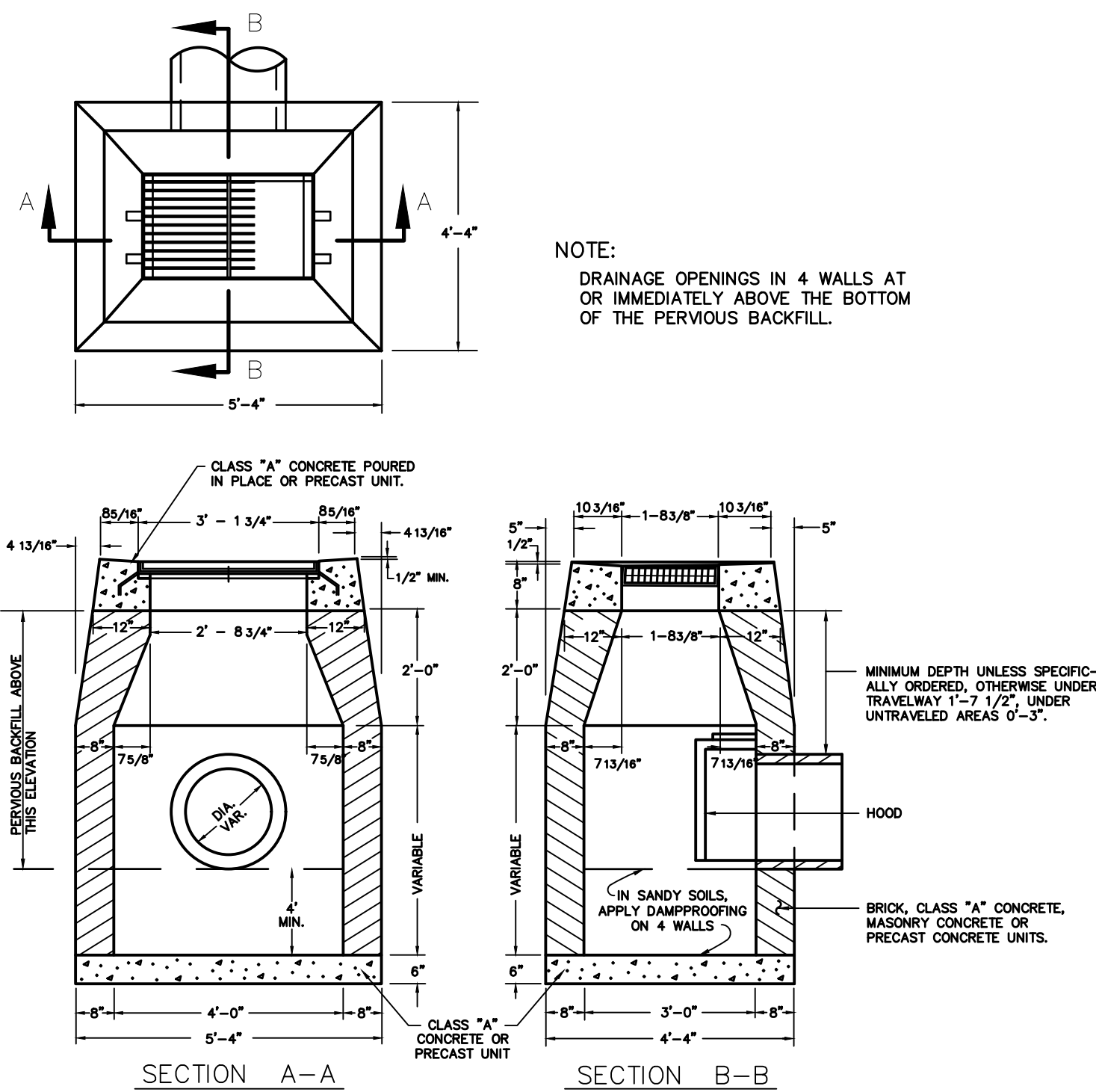
TYPICAL SANITARY SEWER TRENCH SECTION

N.T.S. BLS-010



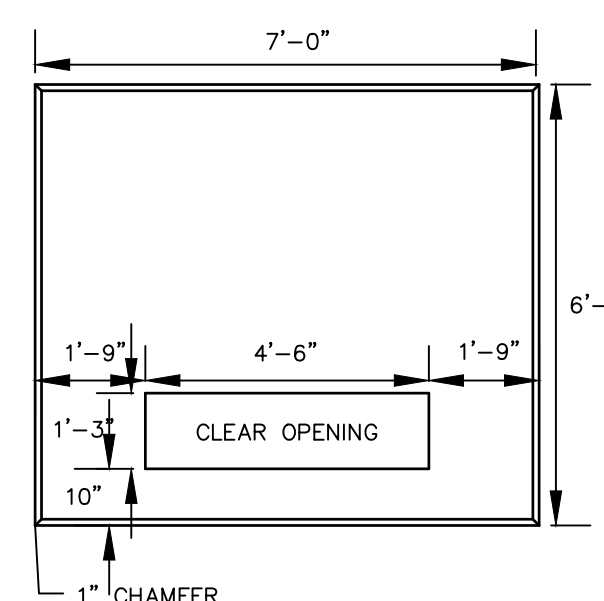
TYPICAL WATER SERVICE TRENCH DETAIL

N.T.S. BLWD-005



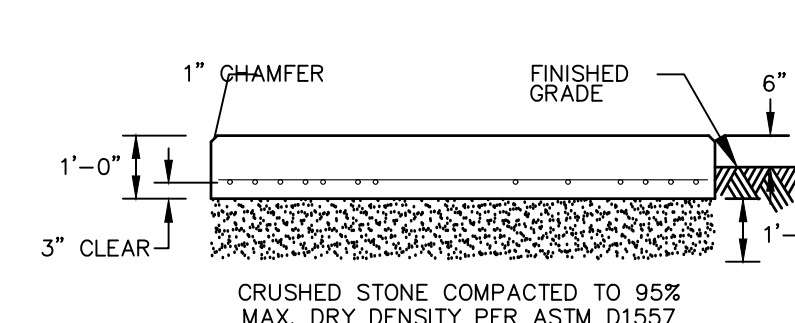
TYPE "C-L" CATCH BASIN WITH HOOD

N.T.S.



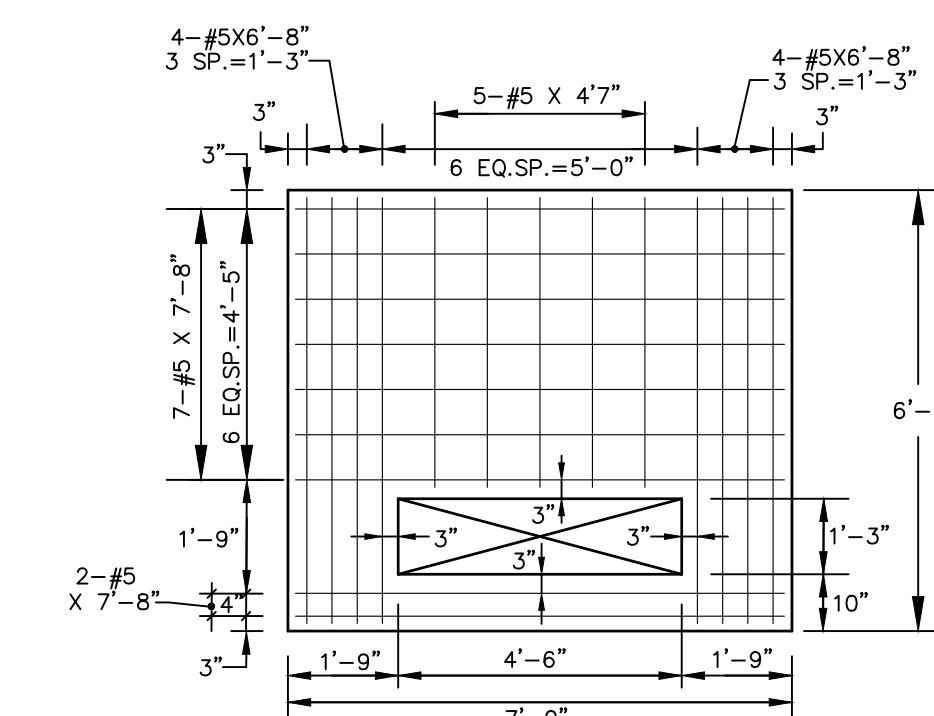
PLAN VIEW

N.T.S.



SECTION

CONFIRM SIZE WITH ELECTRIC COMPANY PRIOR TO CONSTRUCTION



PLAN OF REINFORCING

BLLE-001

BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT

DATE APPROVED _____ DATE OF EXPIRATION _____

CHAIRMAN

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON _____

**FOR PERMITTING PURPOSES ONLY
NOT RELEASED FOR CONSTRUCTION**

REVISIONS

No.	Date	Desc.
1.	05/05/2021	REVISED PER TOWN COMMENTS

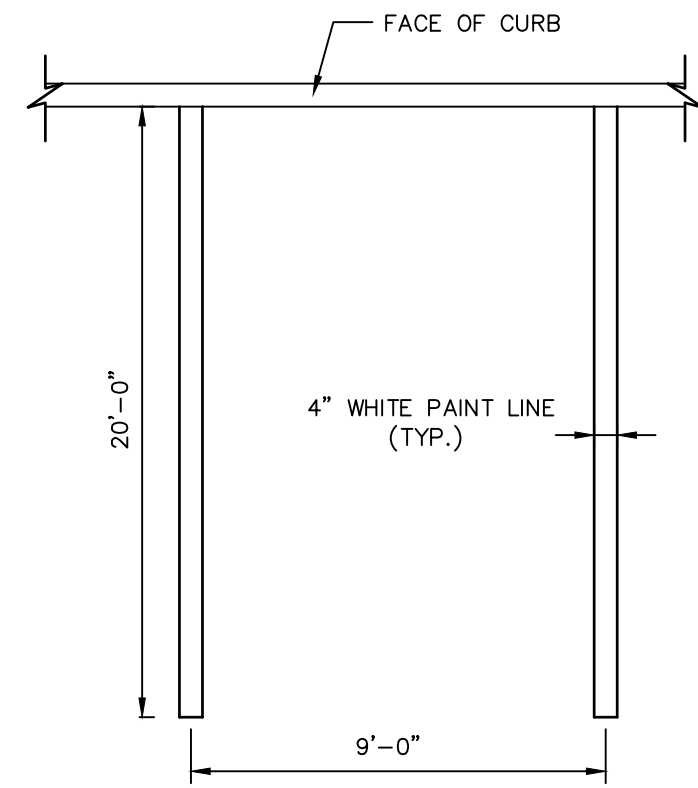
Designed	S.E.L.
Drawn	S.E.L.
Reviewed	K.M.M.
Scale	NONE
Project No.	2002032
Date	04/02/2021
CAD File:	DN200203201

Title

DETAILS SHEET

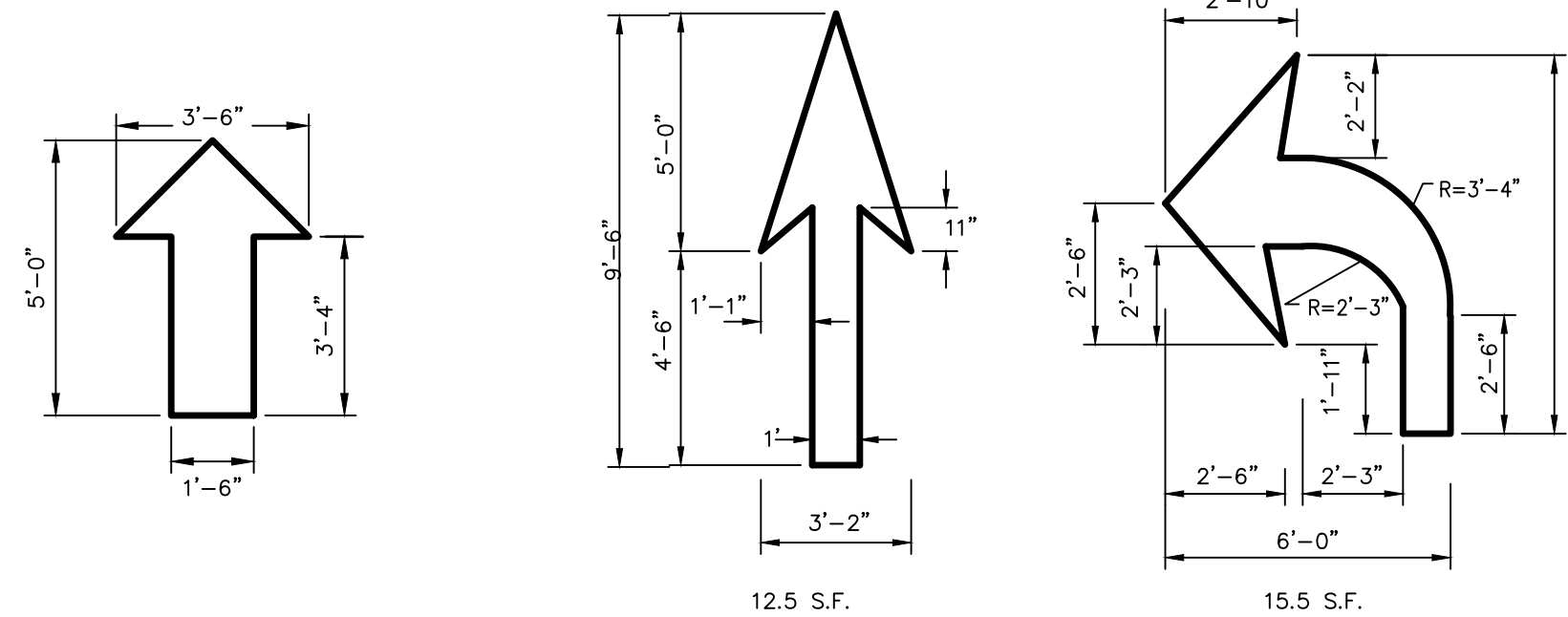
Sheet No.

DN-2



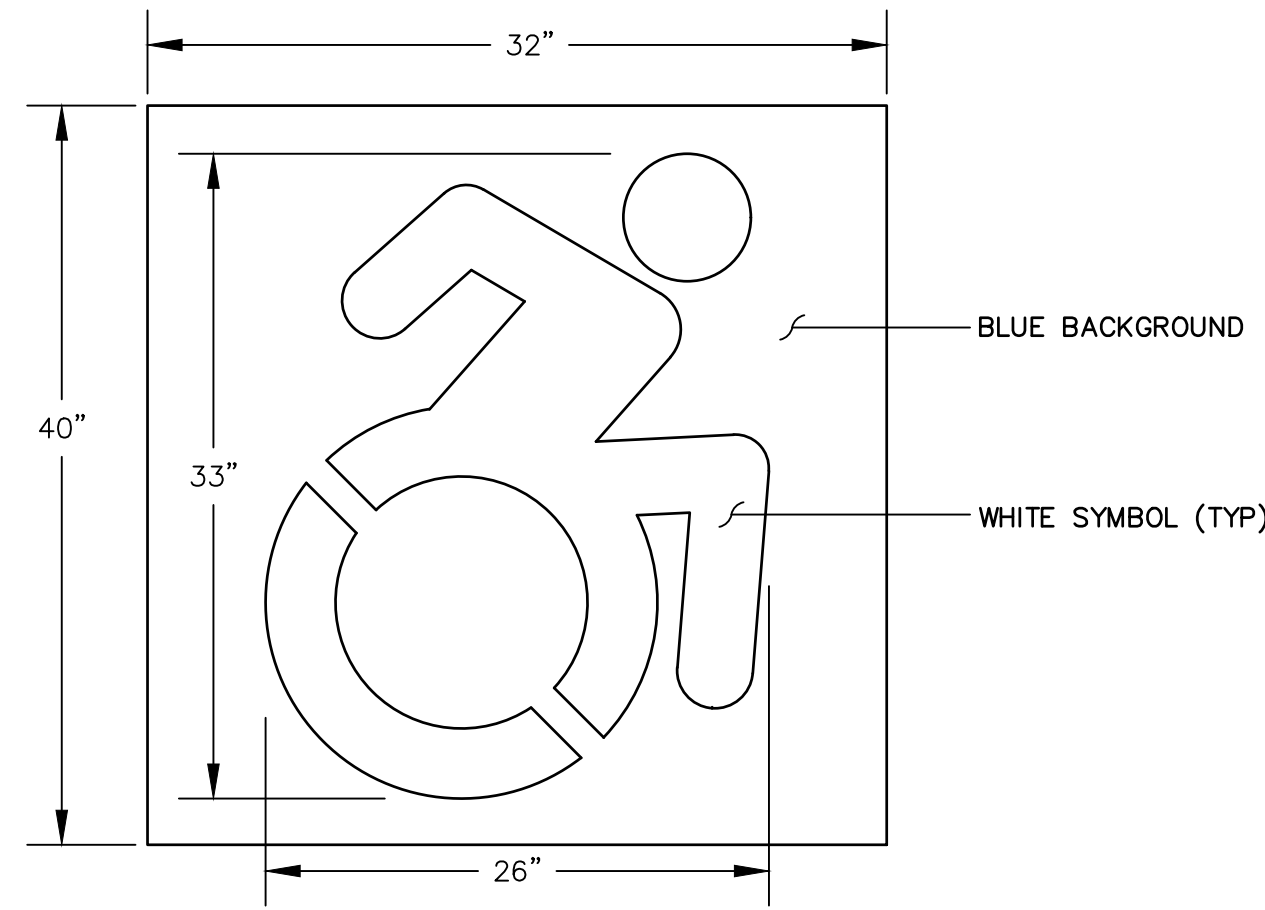
NOTE:
1. PROVIDE 2 COATS OF PAINT ON ALL SURFACES.
2. SEE PLAN FOR ACTUAL SPACE LOCATION AND DIMENSIONS.

TYPICAL PARKING SPACE DETAIL
N.T.S. BLPC-003



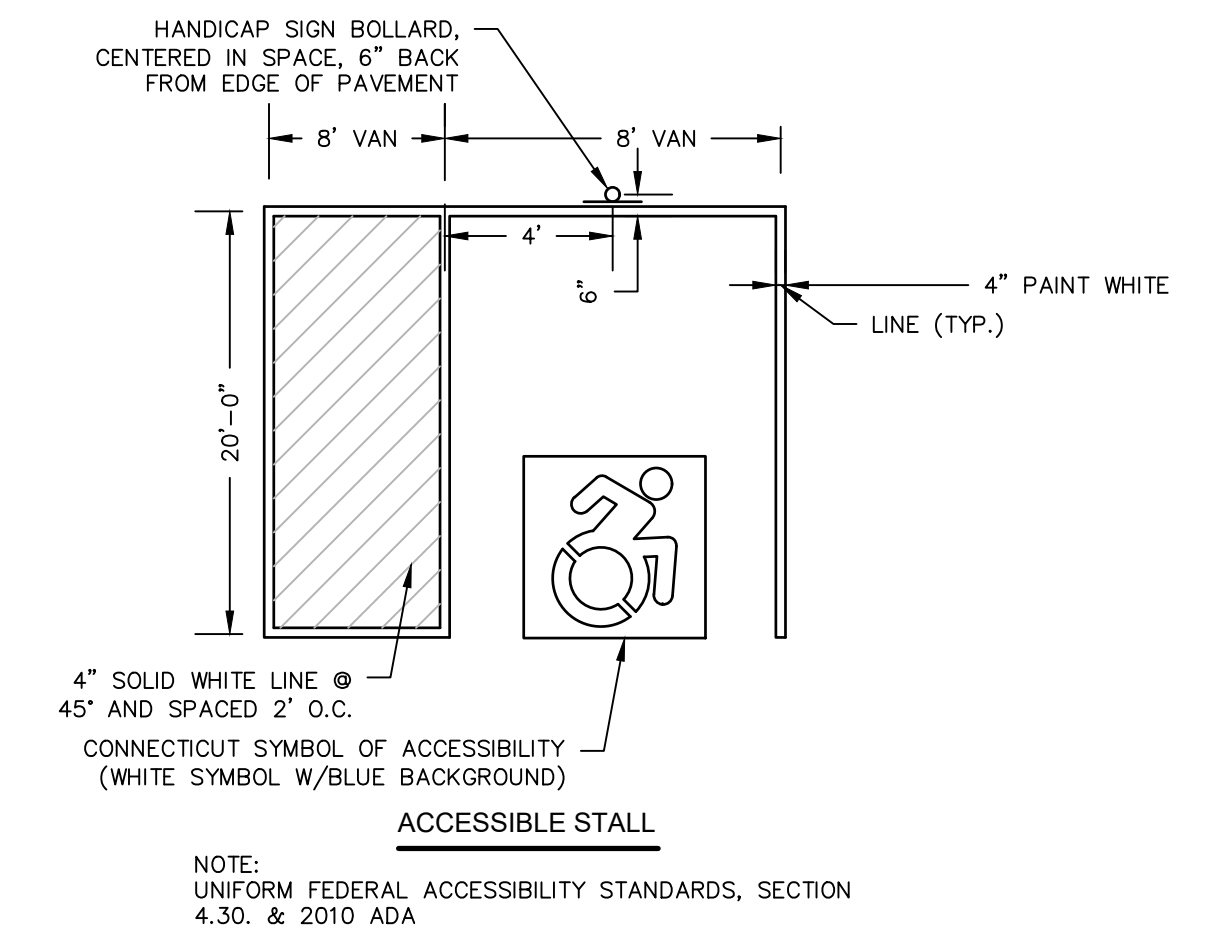
NOTES:
1. WHITE (ARROWS TO BE CENTERED IN TRAVEL LANE)

PAINTED TRAFFIC ARROW DETAILS
N.T.S. BLPC-006



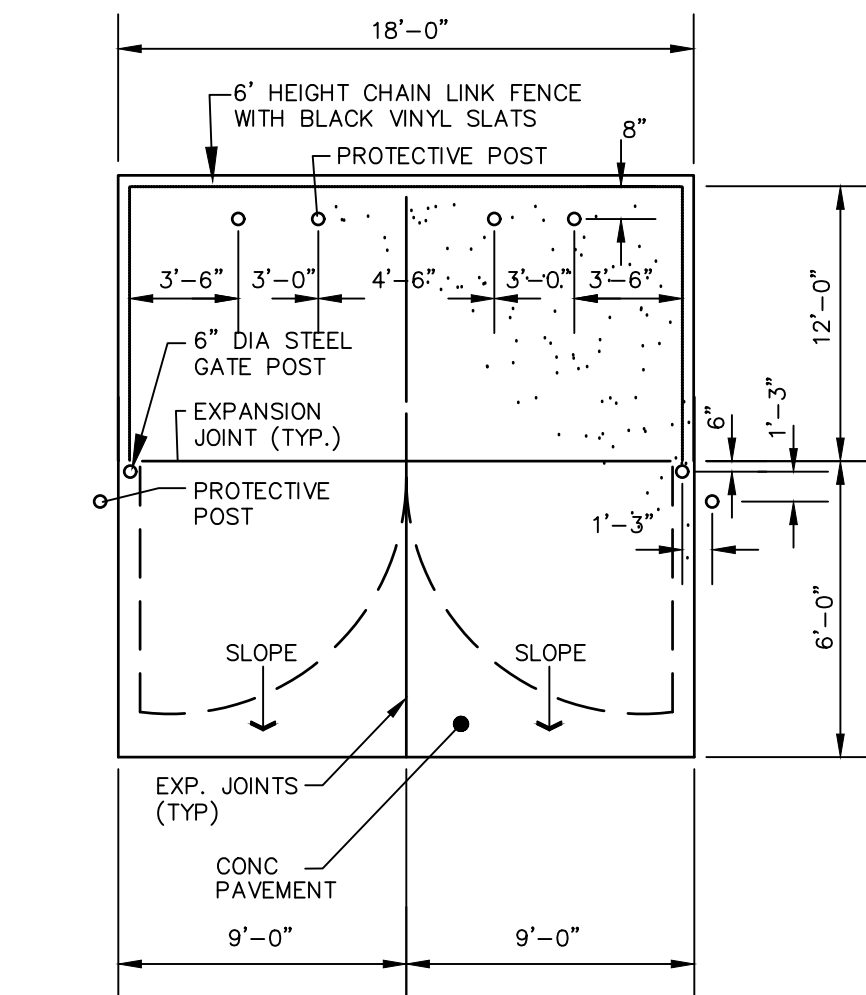
NOTE: HANDICAP SYMBOL TO ADHERE TO STATE BUILDING CODE, LATEST EDITION

CONNECTICUT SYMBOL OF ACCESSIBILITY
N.T.S.

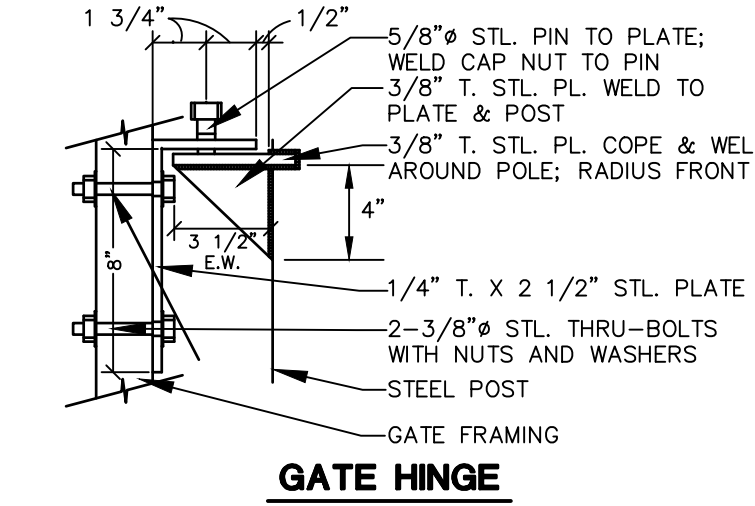


NOTE: UNIFORM FEDERAL ACCESSIBILITY STANDARDS, SECTION 4.30. & 2010 ADA

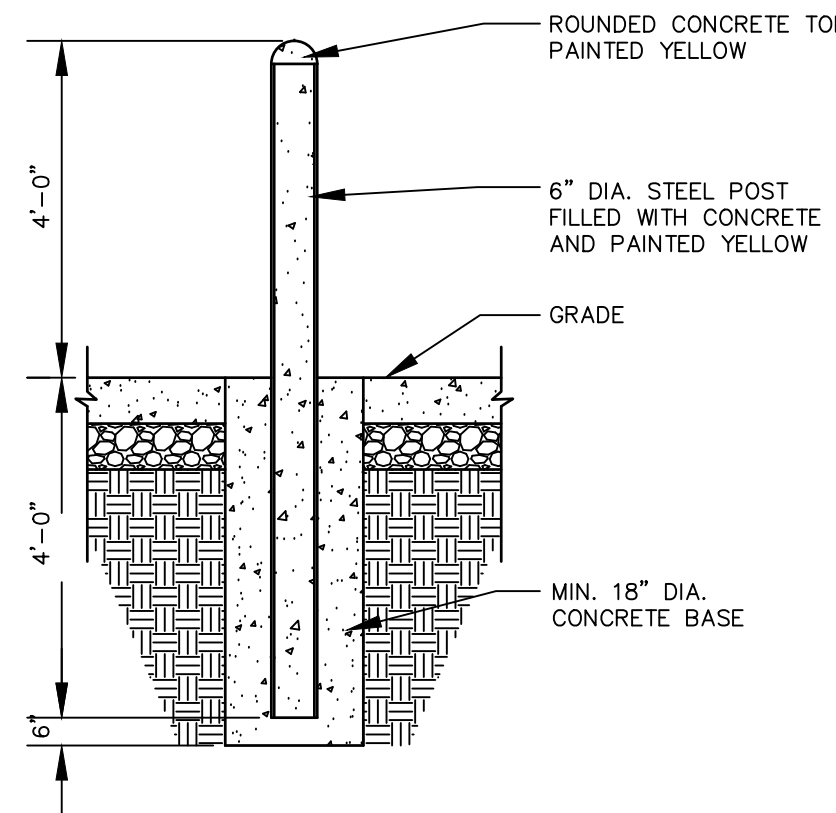
TYPICAL HANDICAP PARKING STALL LAYOUT
N.T.S.



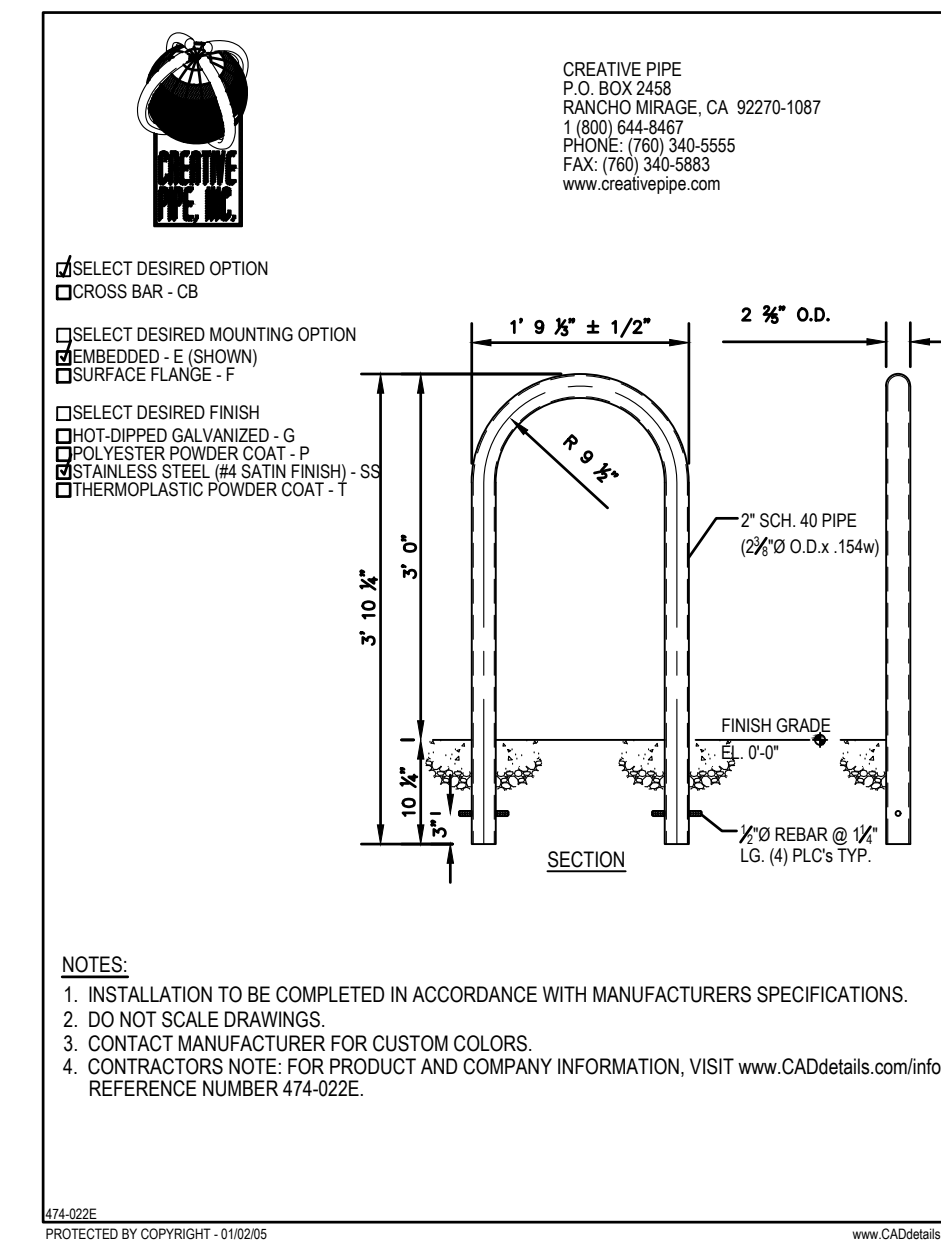
TRASH ENCLOSURE PLAN
N.T.S. BLSE-004



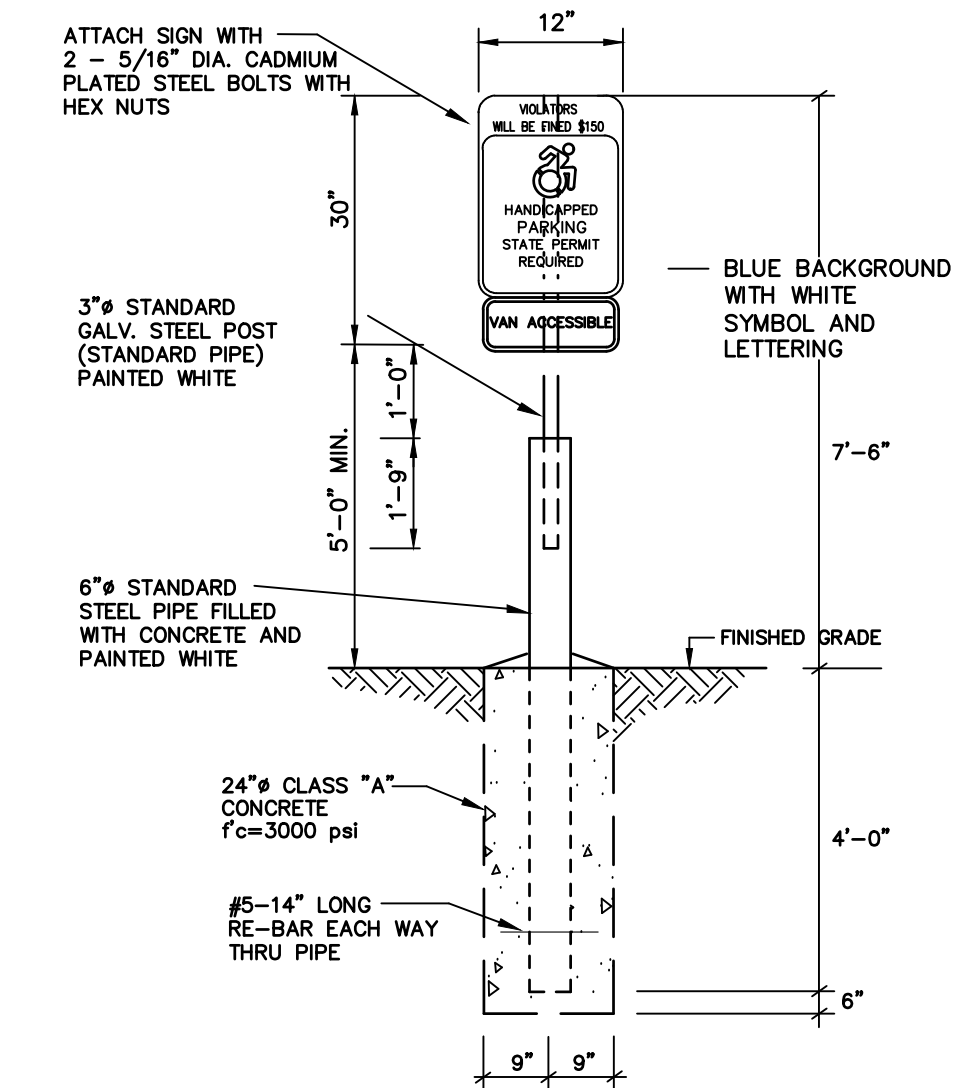
TRASH ENCLOSURE GATE (HALF SECTION)
N.T.S. BLSE-001



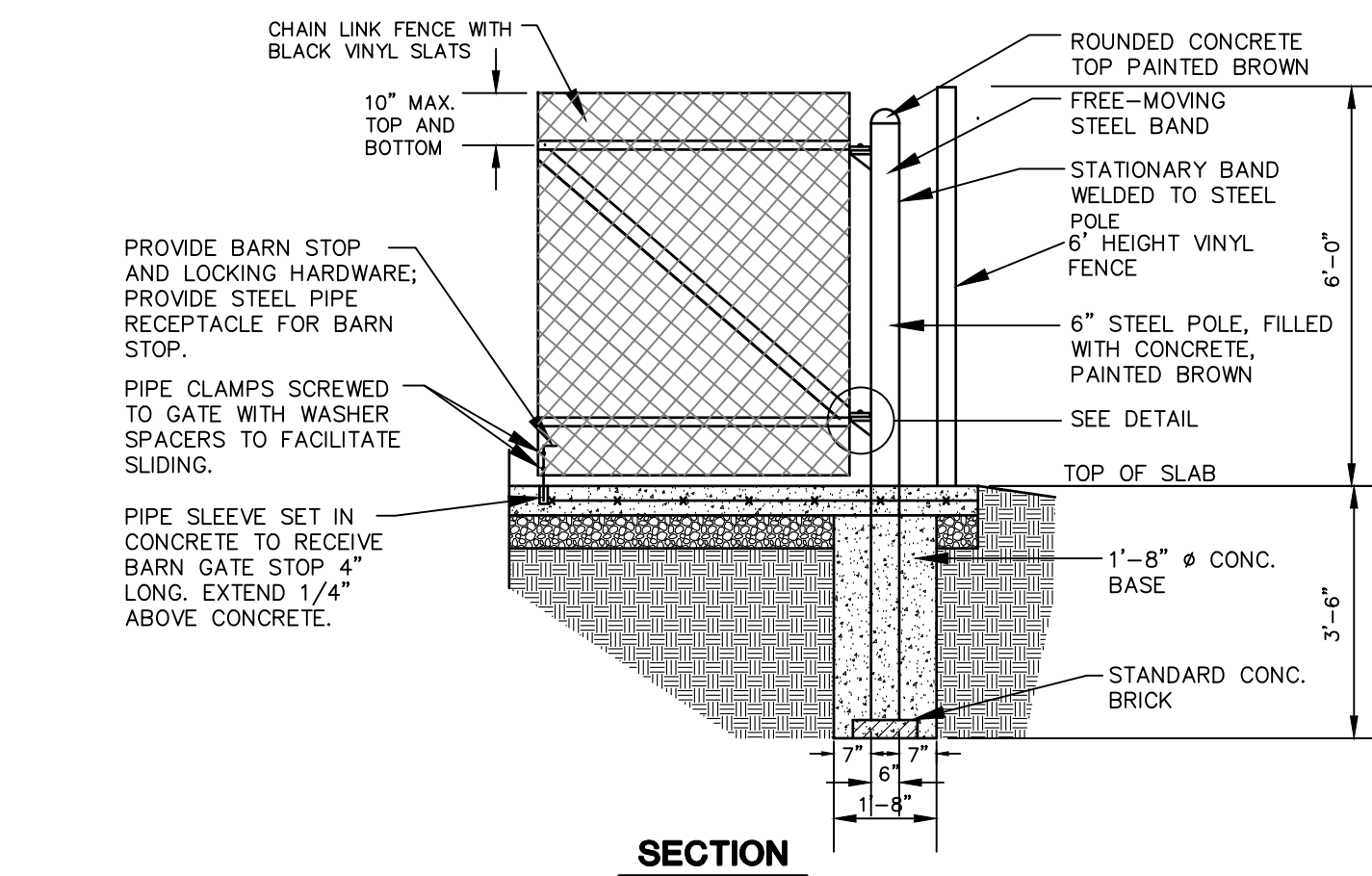
6" CONCRETE FILLED STEEL BOLLARD
N.T.S. BLSE-005



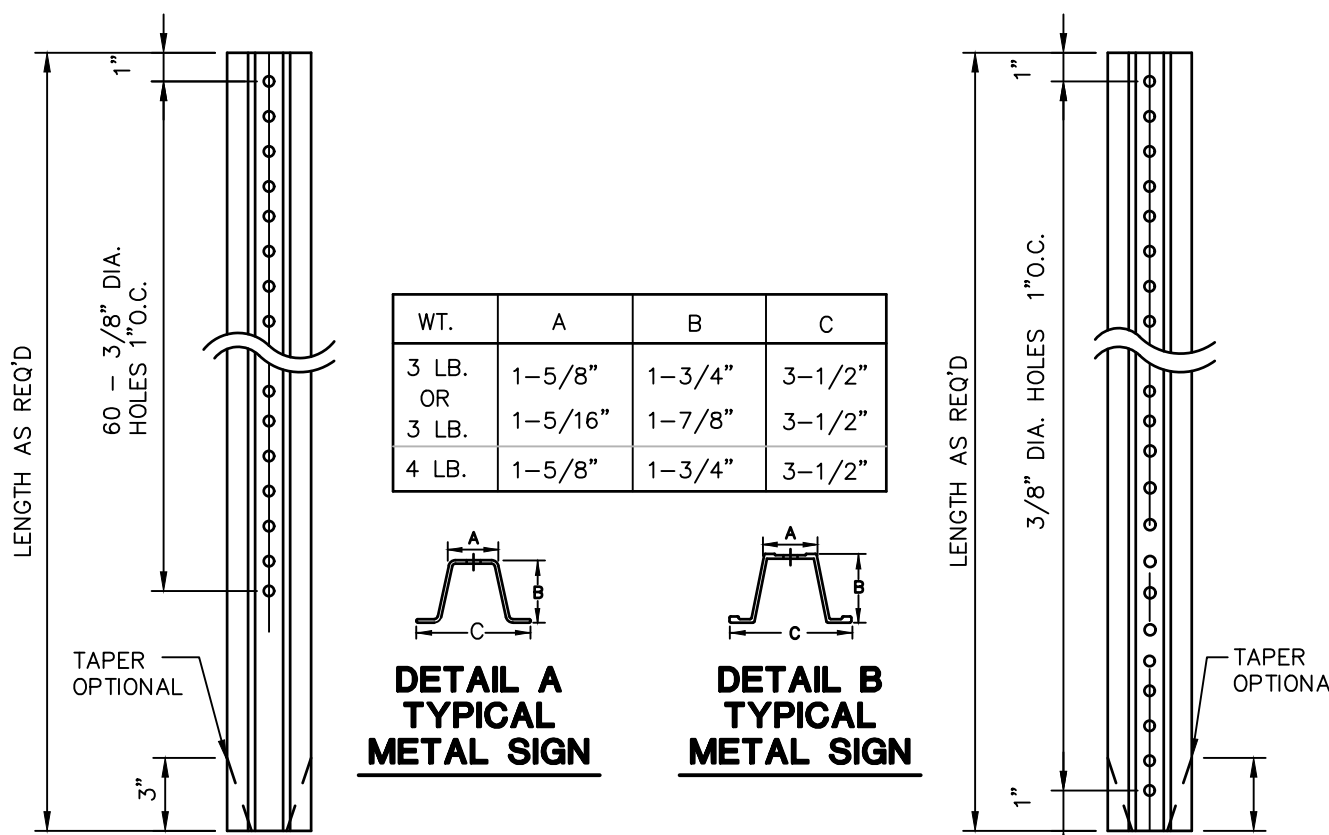
INVERTED 'U' BICYCLE RACK
N.T.S.



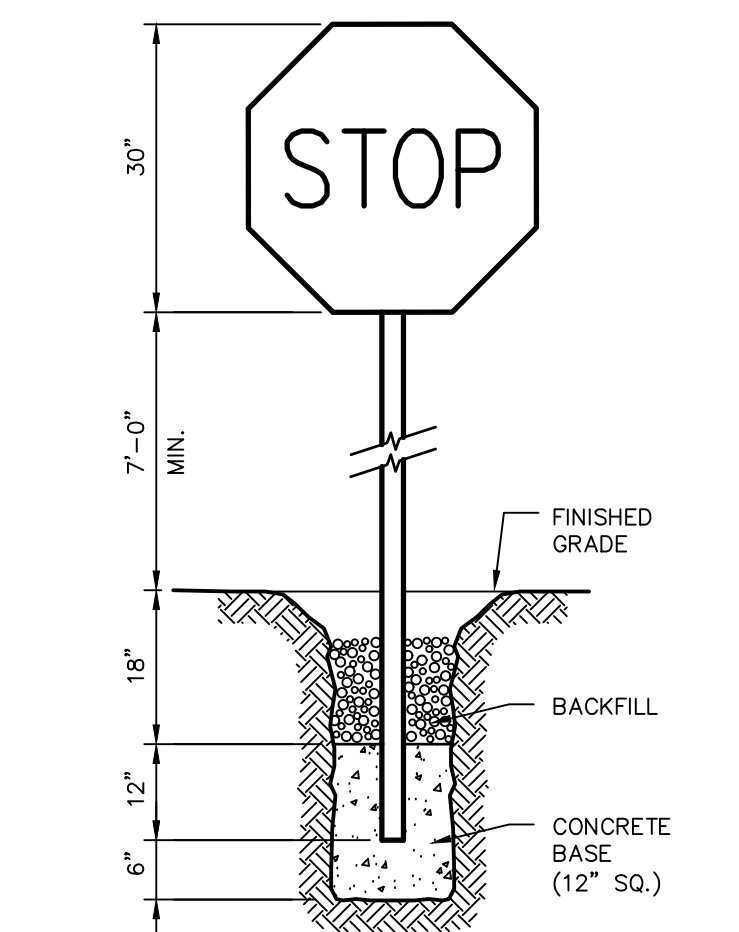
HANDICAP SIGN BOLLARD DETAIL
N.T.S.



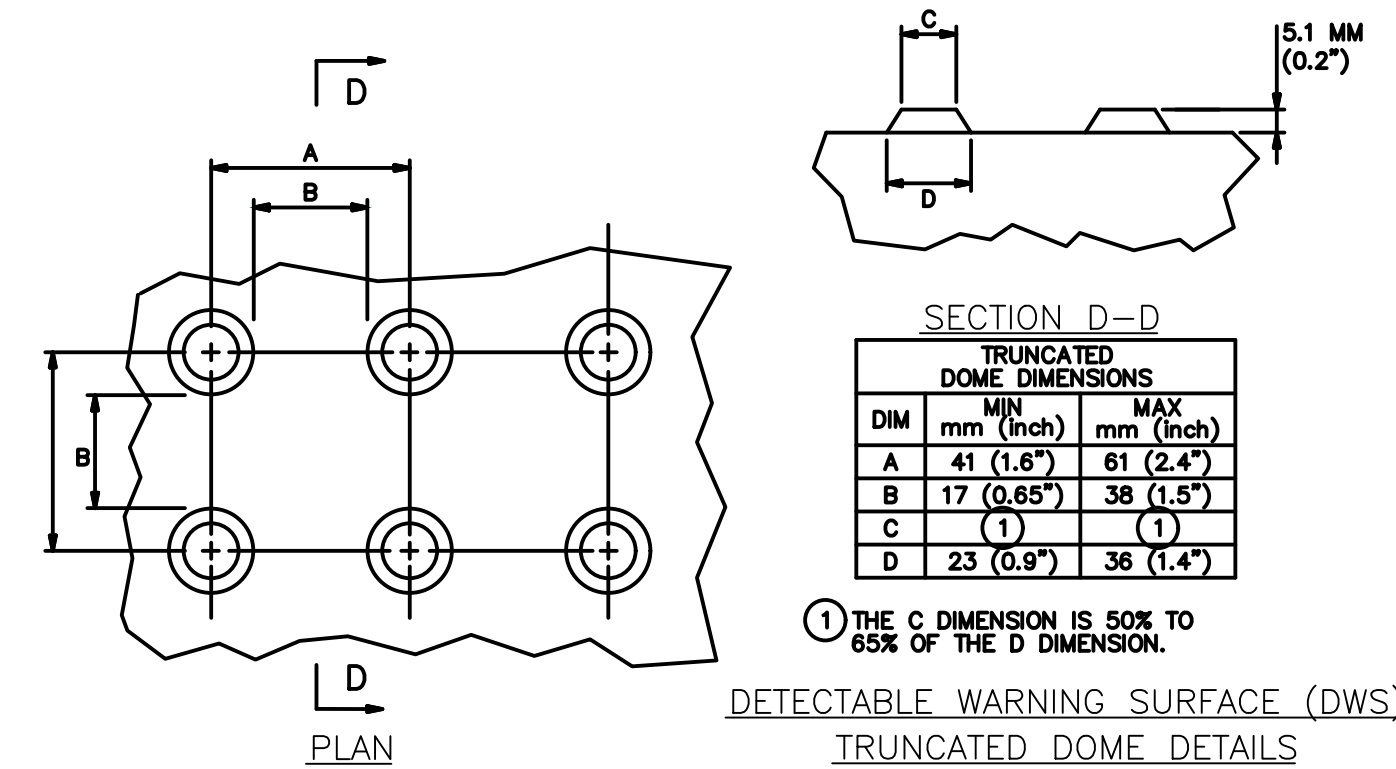
TRASH ENCLOSURE CHAIN LINK FENCE GATE
N.T.S. BLFD-001



TYPICAL METAL SIGN POSTS
N.T.S. BLSO-001

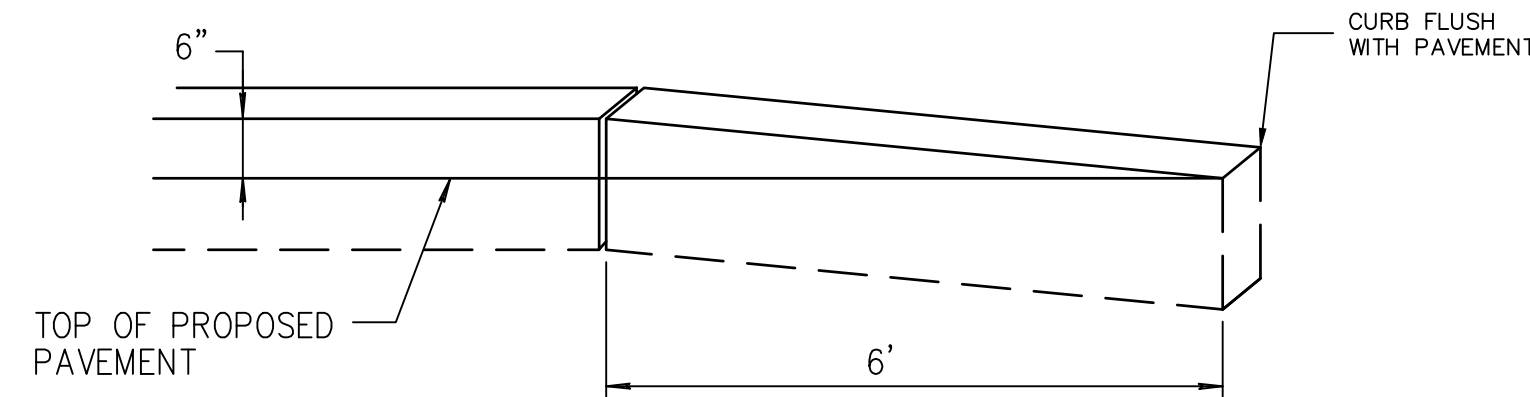


STOP SIGN
N.T.S. BLSO-002



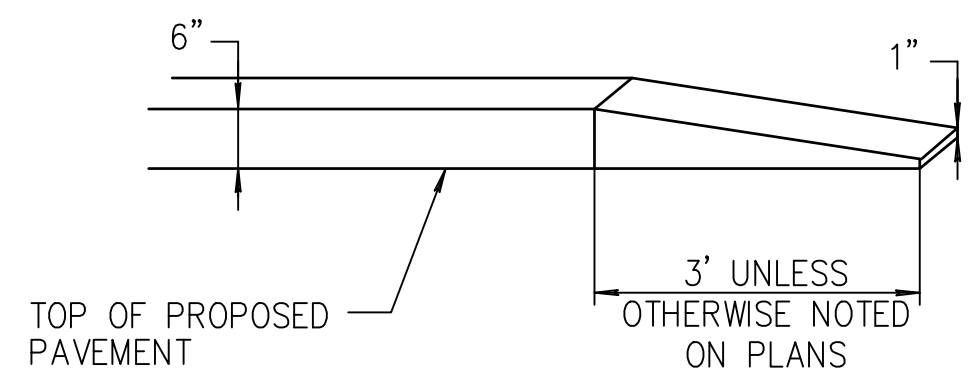
DETECTABLE TRUNCATED DOME DETECTABLE WARNING SURFACE (DWS) AND X-SECT.
N.T.S.

**FOR PERMITTING PURPOSES ONLY
NOT RELEASED FOR CONSTRUCTION**



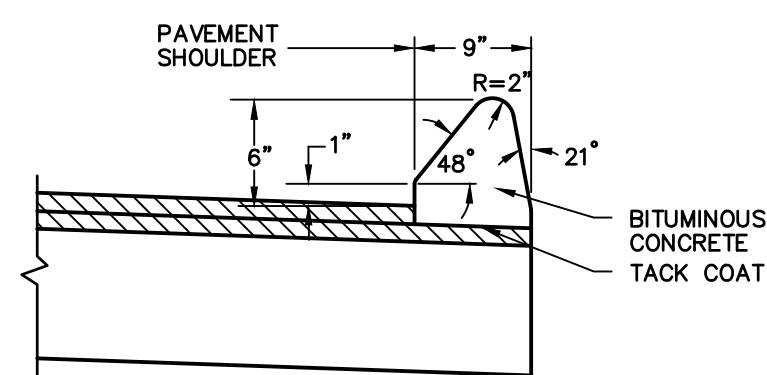
6' CONCRETE TRANSITION CURB

N.T.S.



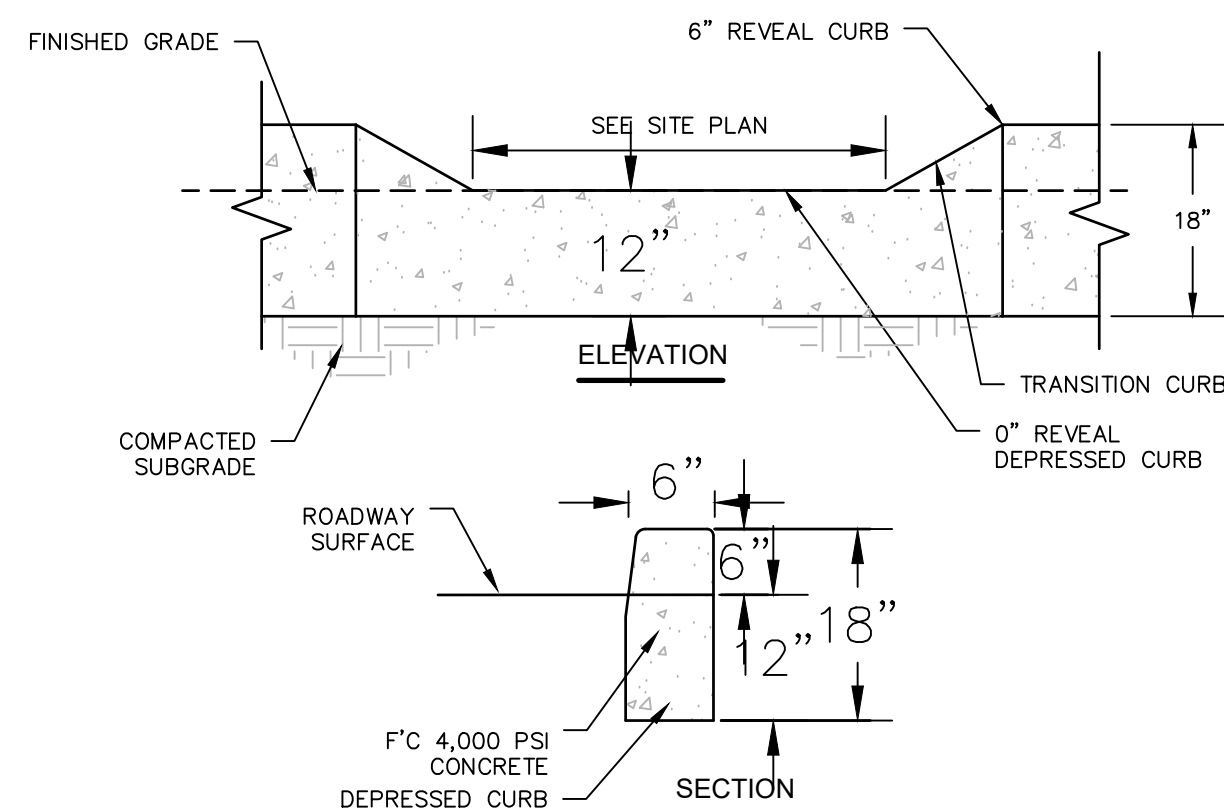
3' BITUMINOUS CONCRETE TRANSITION CURB

N.T.S.



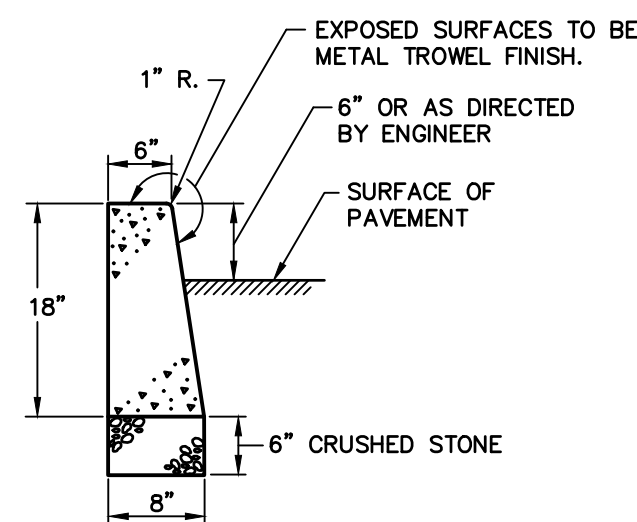
BITUMINOUS CONCRETE LIP CURBING

N.T.S.



6" REVEAL DEPRESSED/ CONCRETE CURB

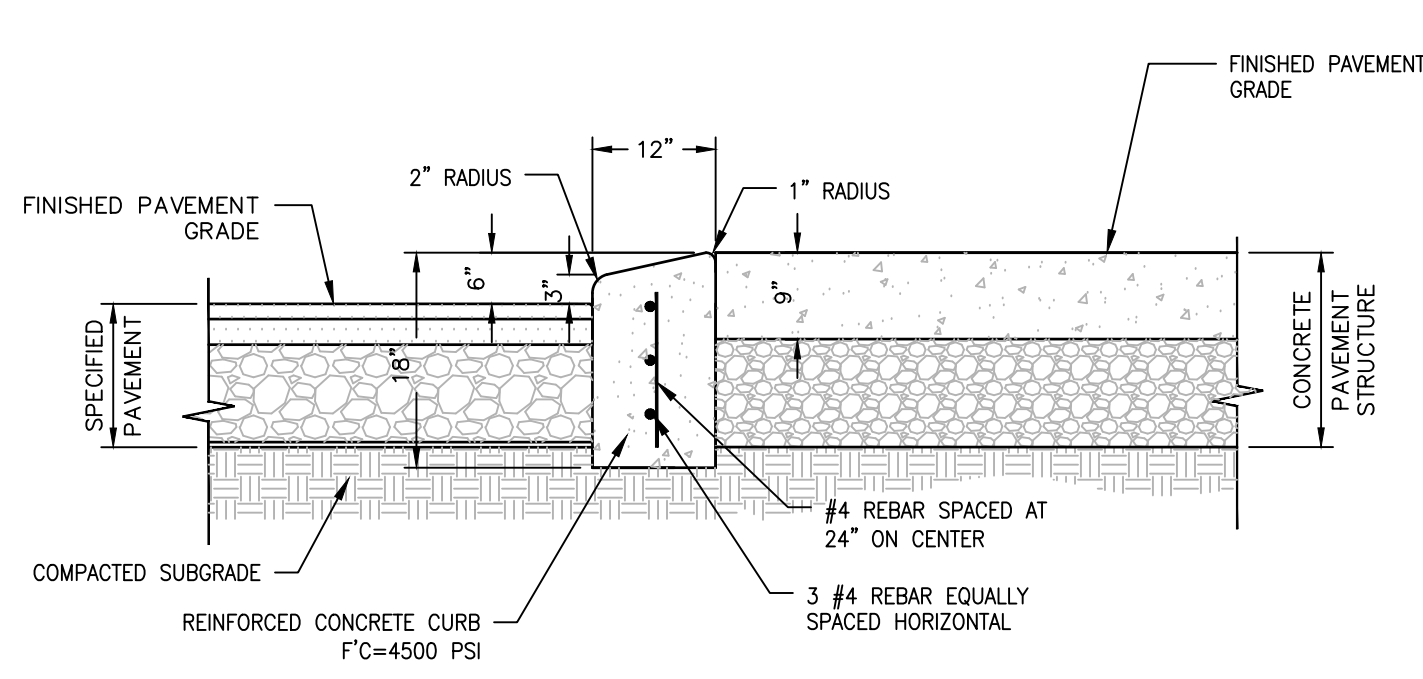
N.T.S.



CONCRETE CURB DETAIL

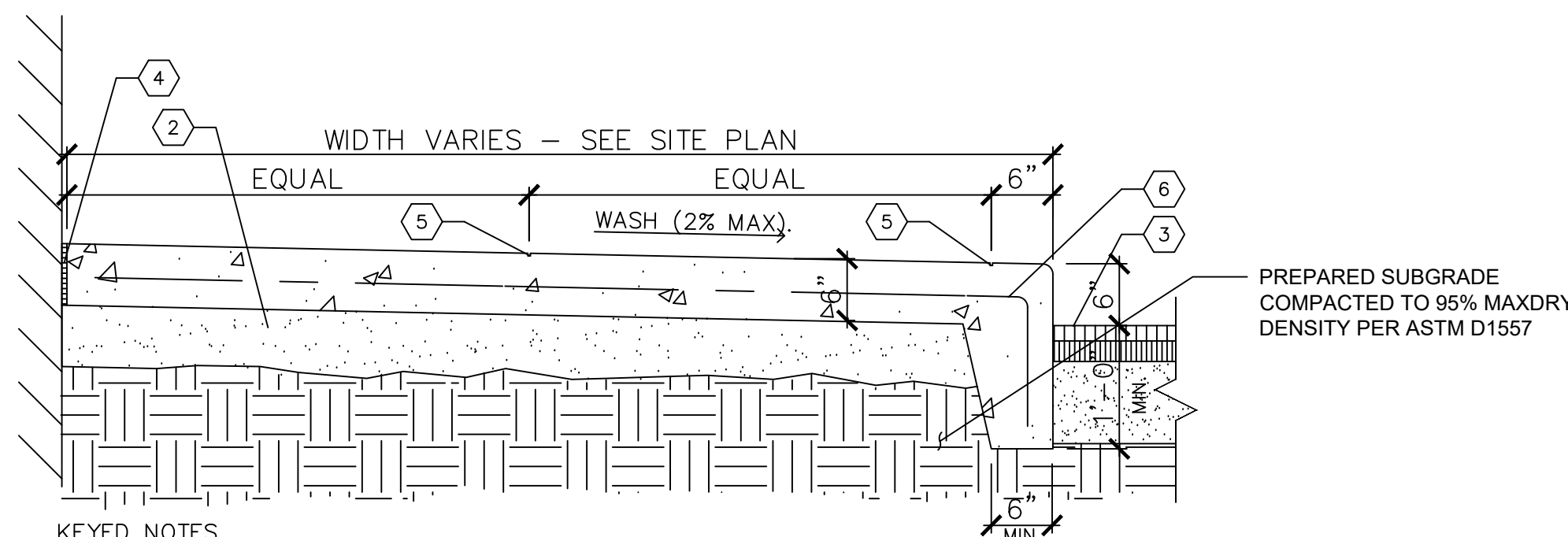
N.T.S.

ZPC-014



MOUNTABLE CONCRETE CURB

N.T.S.

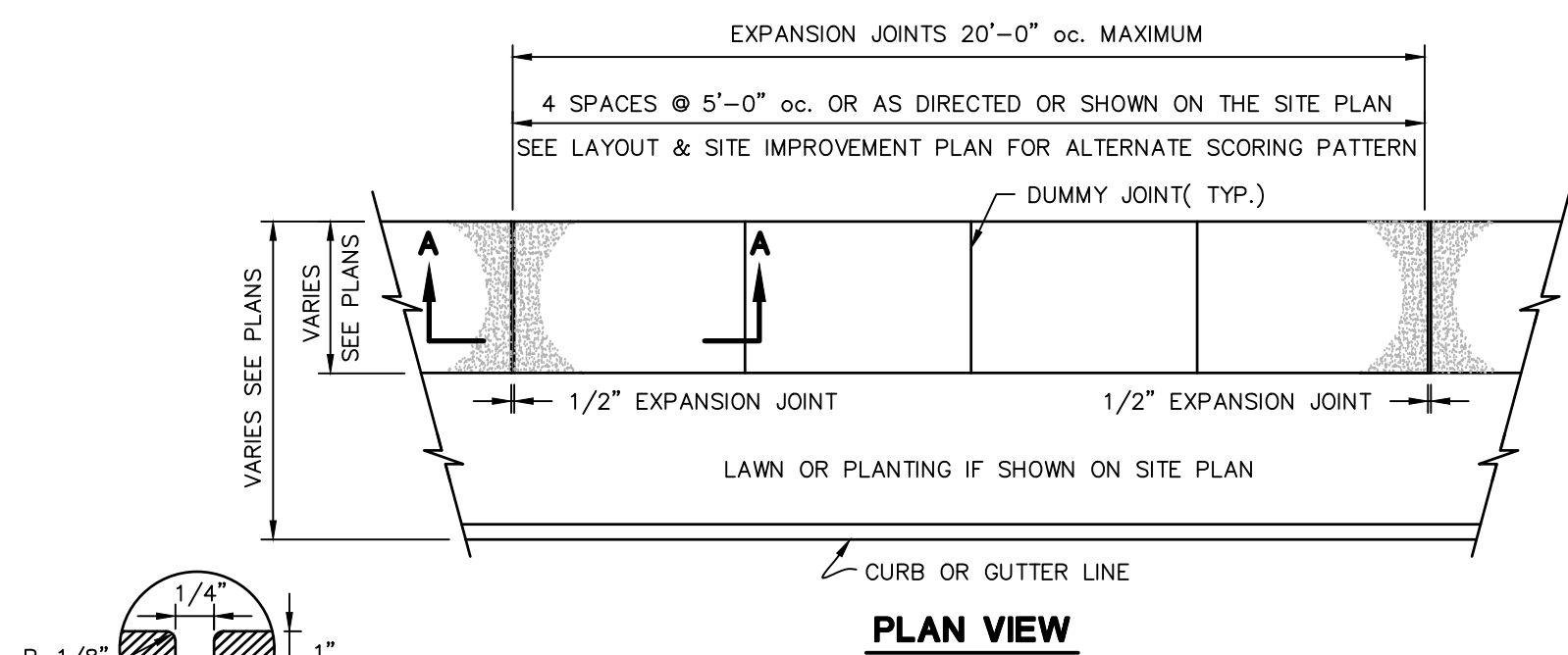


- KEYED NOTES**
- 1) N/A
 - 2) 6" PROCESSED AGGREGATE BASE COURSE, CTDOT M.05.01
 - 3) PAVEMENT.
 - 4) COMPRESSIBLE FILLER (3/4" MAXIMUM). CUT BACK AND PROVIDE SEALANT, TYPICAL, AT ALL JOINTS WITH FILLER.
 - 5) 1/4" TOOLED JOINT 20' O.C. MAXIMUM. 1/4" TOOL JOINT 5' O.C. OR AS DIRECTED. CONCRETE TO BE 4,000 P.S.I.
 - 6) 6" X 6" W2.1 X 2.1 W.W.F.

MONOLITHIC CONCRETE CURB AND SIDEWALK DETAIL

N.T.S.

WAG

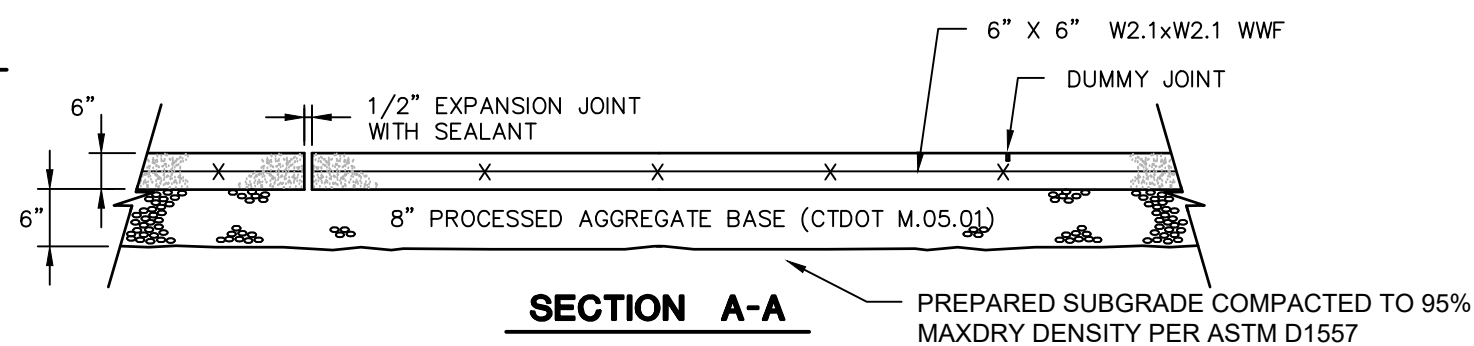


PLAN VIEW



DUMMY JOINT

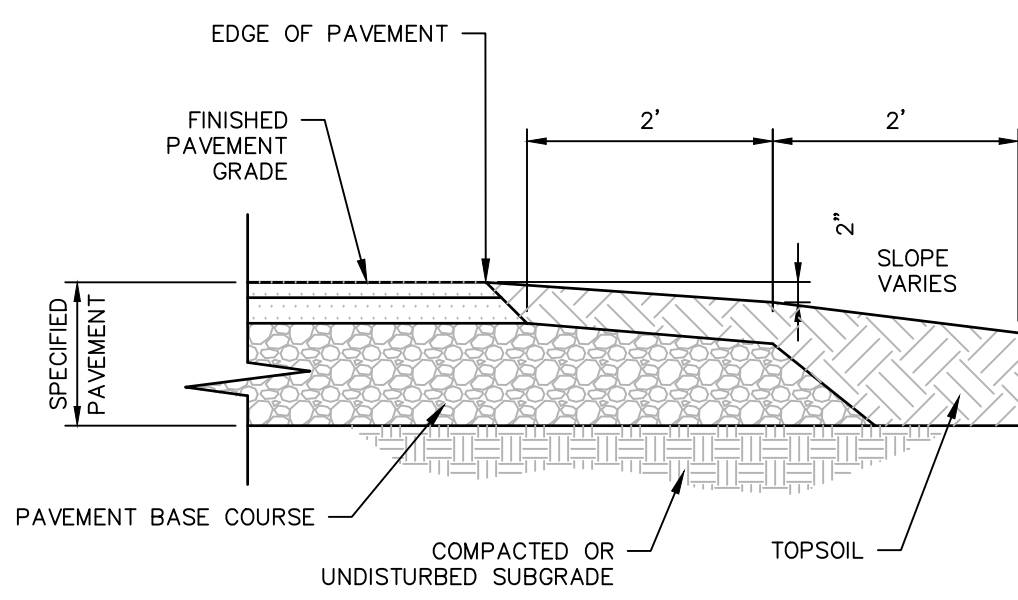
N.T.S.



CONCRETE SIDEWALK DETAIL

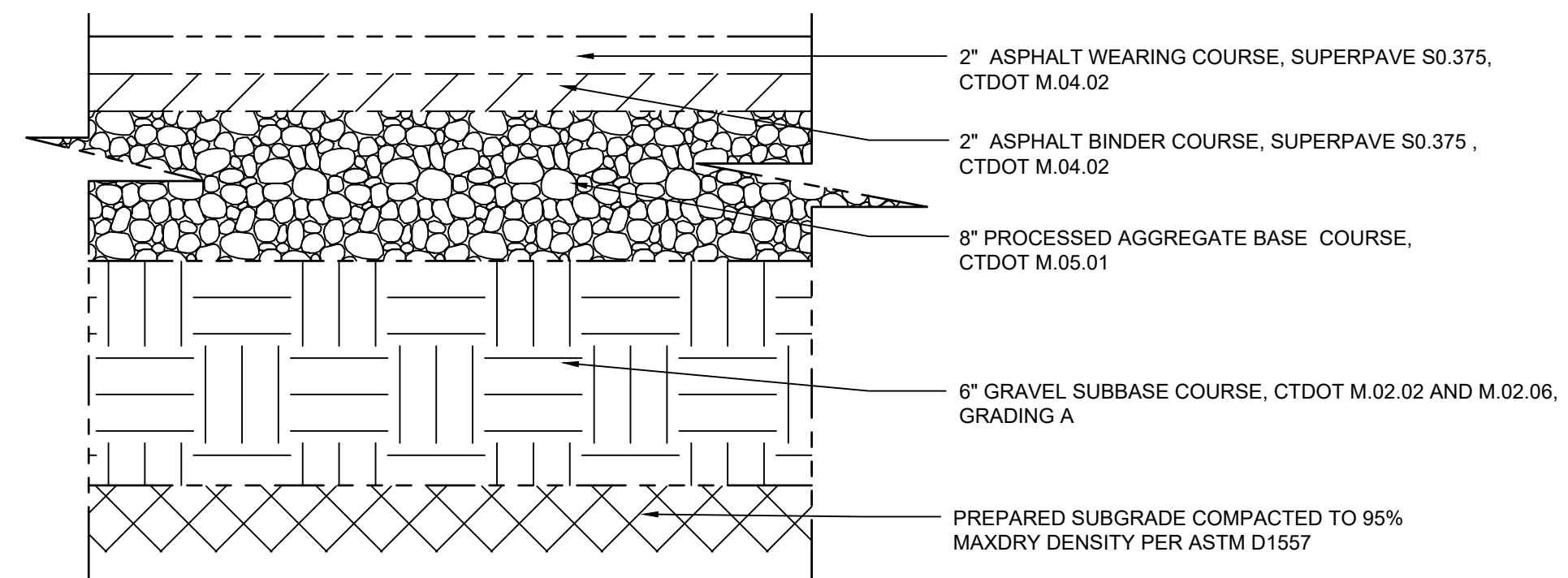
N.T.S.

BLSR-001



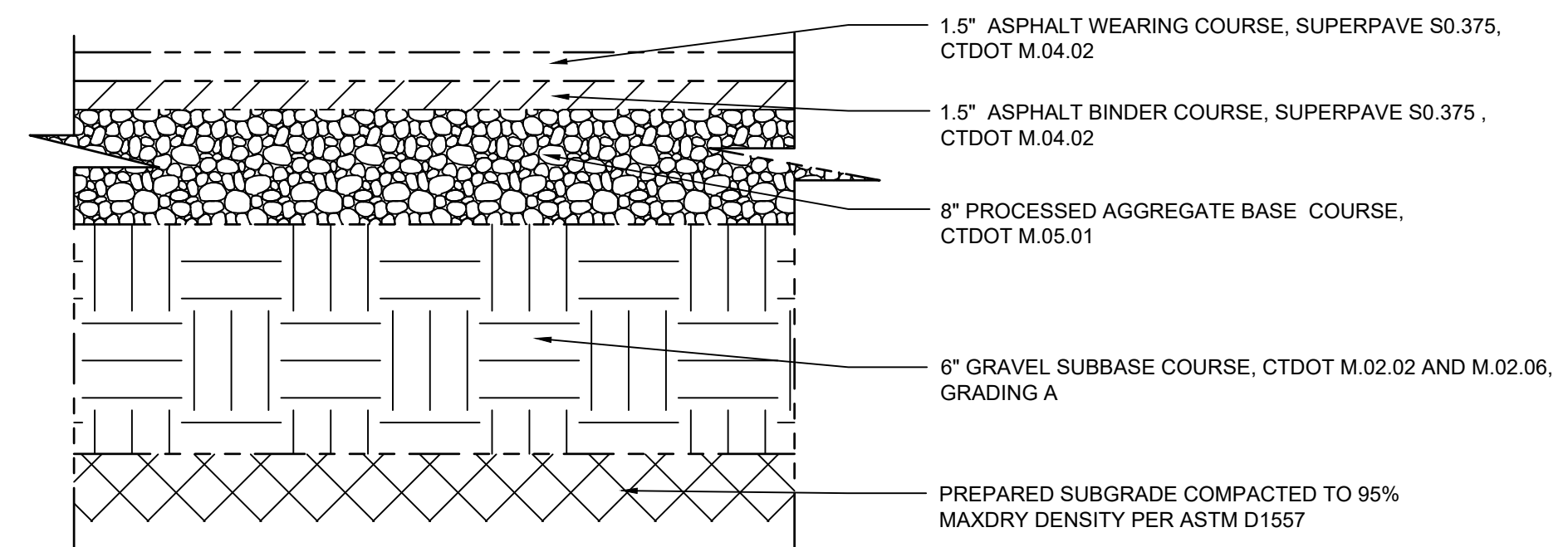
PAVEMENT END SECTION DETAIL

N.T.S.



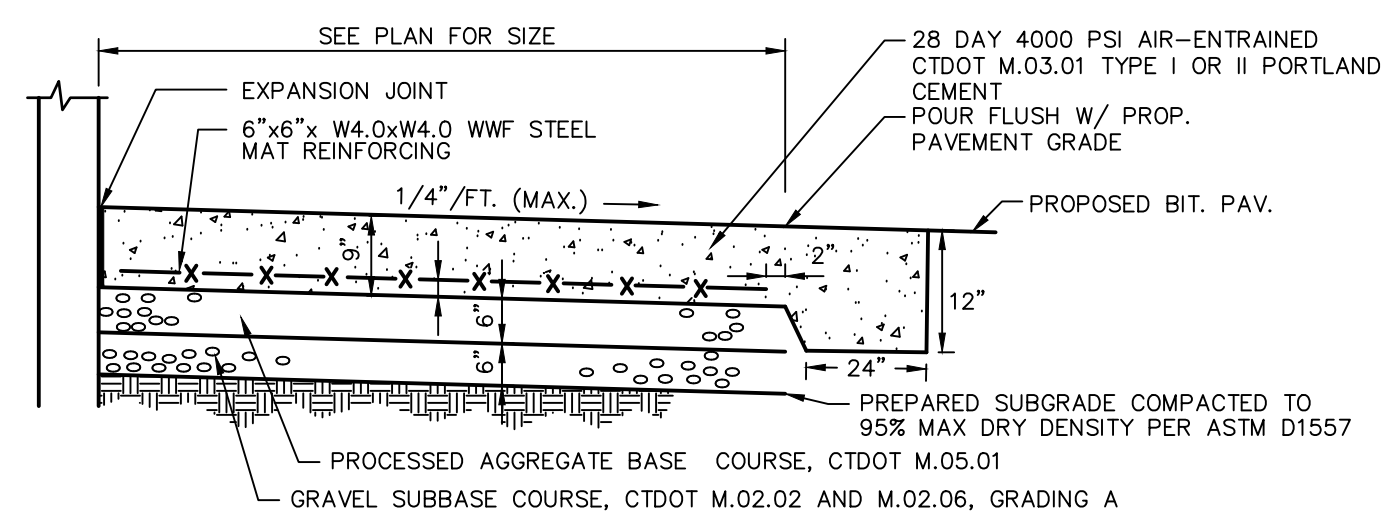
HEAVY DUTY BITUMINOUS CONCRETE PAVEMENT STRUCTURE DETAIL

N.T.S.



STANDARD DUTY BITUMINOUS CONCRETE PAVEMENT STRUCTURE DETAIL

N.T.S.

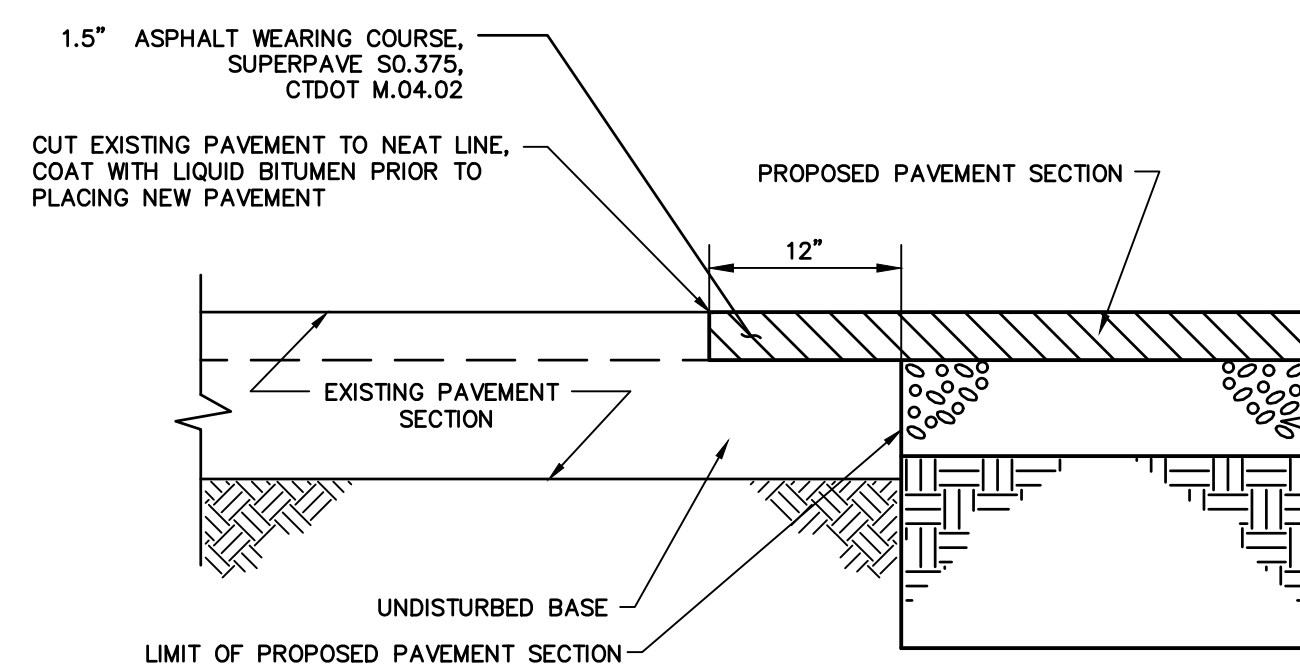


SECTION VIEW

CONCRETE TRASH ENCLOSURE DUMPSTER PAD AND CONCRETE PAVEMENT

N.T.S.

BLPC-002



EDGE OF PAVEMENT DETAIL

N.T.S.

**FOR PERMITTING PURPOSES ONLY
NOT RELEASED FOR CONSTRUCTION**

BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT

DATE APPROVED _____ DATE OF EXPIRATION _____

CHAIRMAN

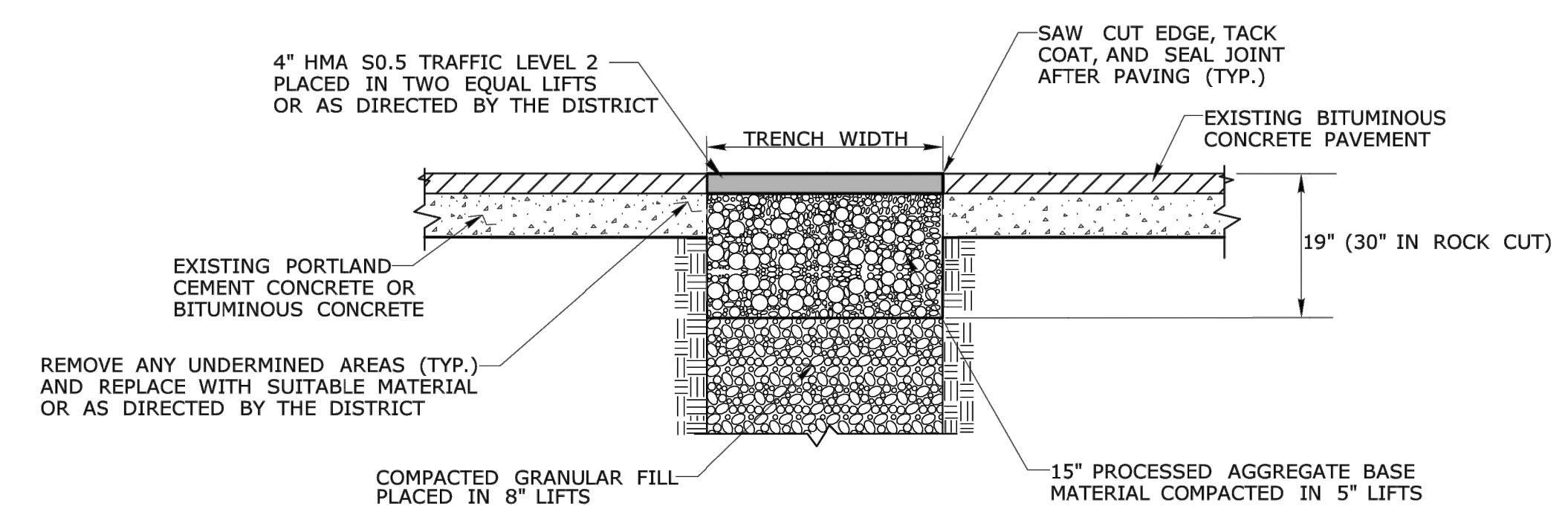
THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON _____

GENERAL NOTES:

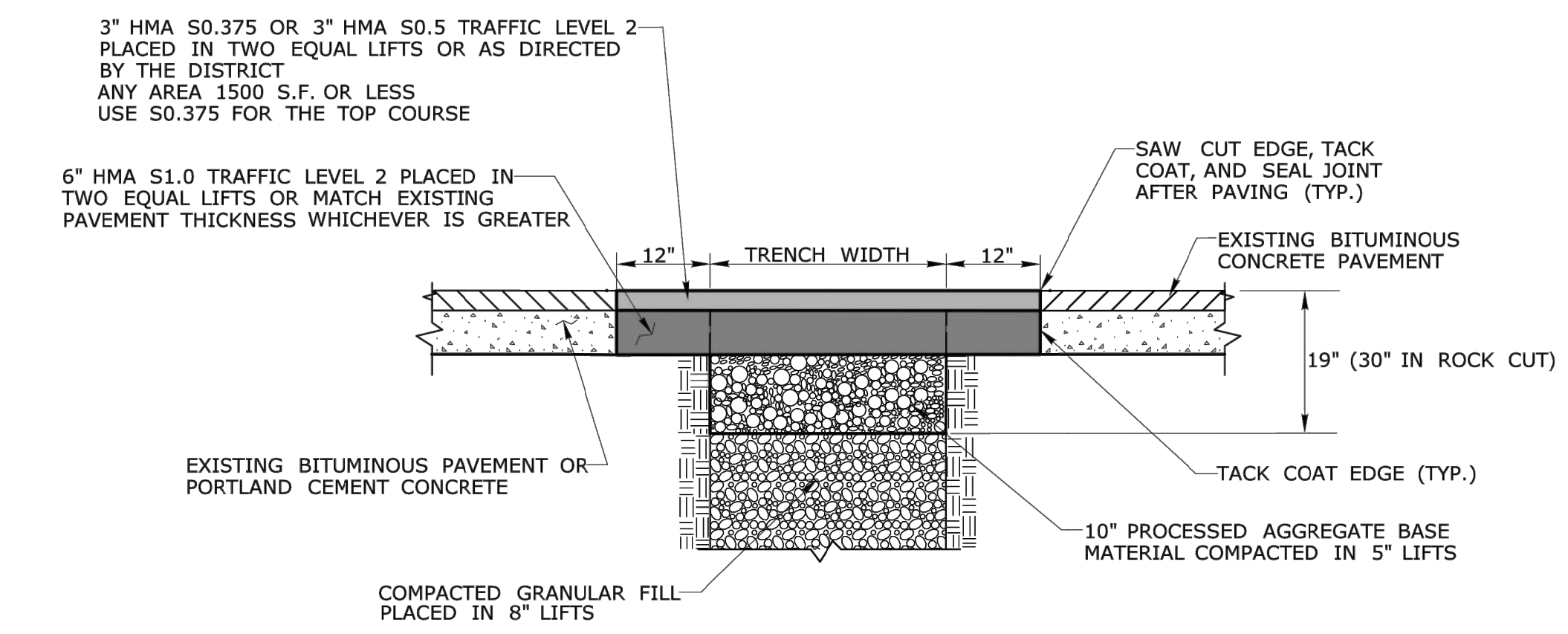
- LONGITUDINAL TRENCHING FOR JOINTED CONCRETE PAVEMENT:
 - IF THE LONGITUDINAL TRENCH FALLS BETWEEN THE SLAB CENTERLINE AND THE EDGE OF SLAB, REMOVE CONCRETE AND BITUMINOUS CONCRETE PAVEMENT FROM THE TRENCH EDGE TO THE EDGE OF ROAD. IF THE LONGITUDINAL TRENCH FALLS BETWEEN THE LONGITUDINAL JOINT AND THE SLAB CENTERLINE, REMOVE THE ENTIRE CONCRETE SLAB AND BITUMINOUS CONCRETE PAVEMENT TO THE EDGE OF ROAD. IN EITHER CASE REBUILD WITH THE FOLLOWING:
 - PLACE HMA S1.0 TRAFFIC LEVEL 2 IN TWO EQUAL 4" - 5" LIFTS TO MATCH EXISTING CONCRETE PAVEMENT THICKNESS
 - PLACE HMA S0.5 TRAFFIC LEVEL 2 IN 2" - 3" LIFTS TO MATCH EXISTING BITUMINOUS CONCRETE PAVEMENT THICKNESS, WITH THE FINAL LIFT BEING 2"
- TRANSVERSE TRENCHING FOR JOINTED CONCRETE PAVEMENT:

TABLE 1	
TOTAL SLAB LENGTH (L)	MIN. LENGTH REMAINING
40' OR LONGER	1/4 L
15' - 40'	10'
15' OR SHORTER	REBUILD TO NEAREST JOINT

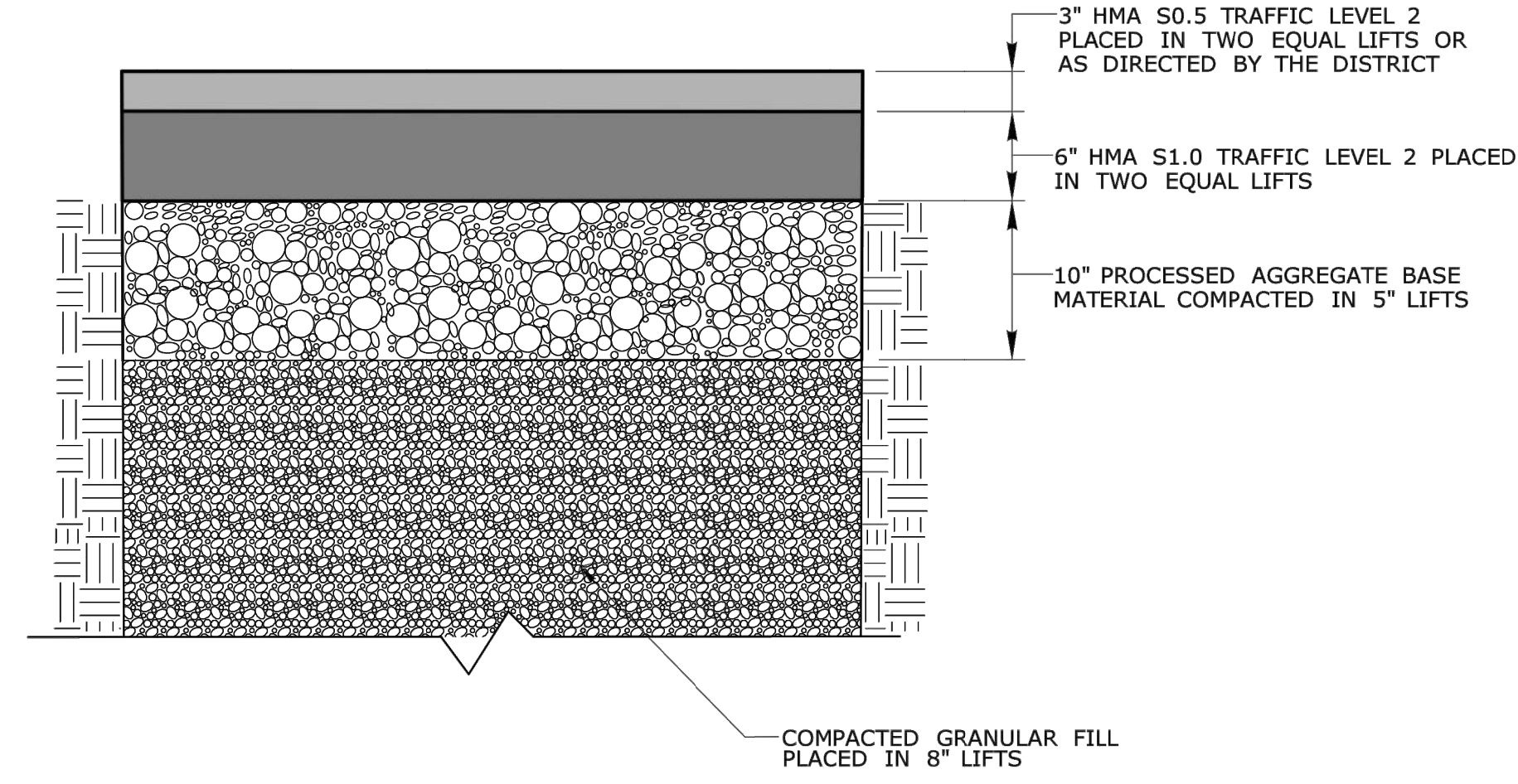
 - FOR TRANSVERSE TRENCHES, THE MINIMUM SLAB LENGTH AS SHOWN IN TABLE 1 SHALL BE LEFT IN PLACE TO THE NEAREST TRANSVERSE JOINT. IF THIS CRITERIA CANNOT BE MET, THE EXISTING SLAB AREA FROM THE TRENCH EDGE TO THE NEAREST TRANSVERSE JOINT SHALL BE REMOVED AND REBUILT AS FOLLOWS:
 - PLACE HMA S1.0 TRAFFIC LEVEL 2 IN TWO EQUAL 4" - 5" LIFTS TO MATCH EXISTING CONCRETE PAVEMENT THICKNESS
 - PLACE HMA S0.5 TRAFFIC LEVEL 2 IN 2" - 3" LIFTS TO MATCH EXISTING BITUMINOUS CONCRETE PAVEMENT THICKNESS, WITH THE FINAL LIFT BEING 2"



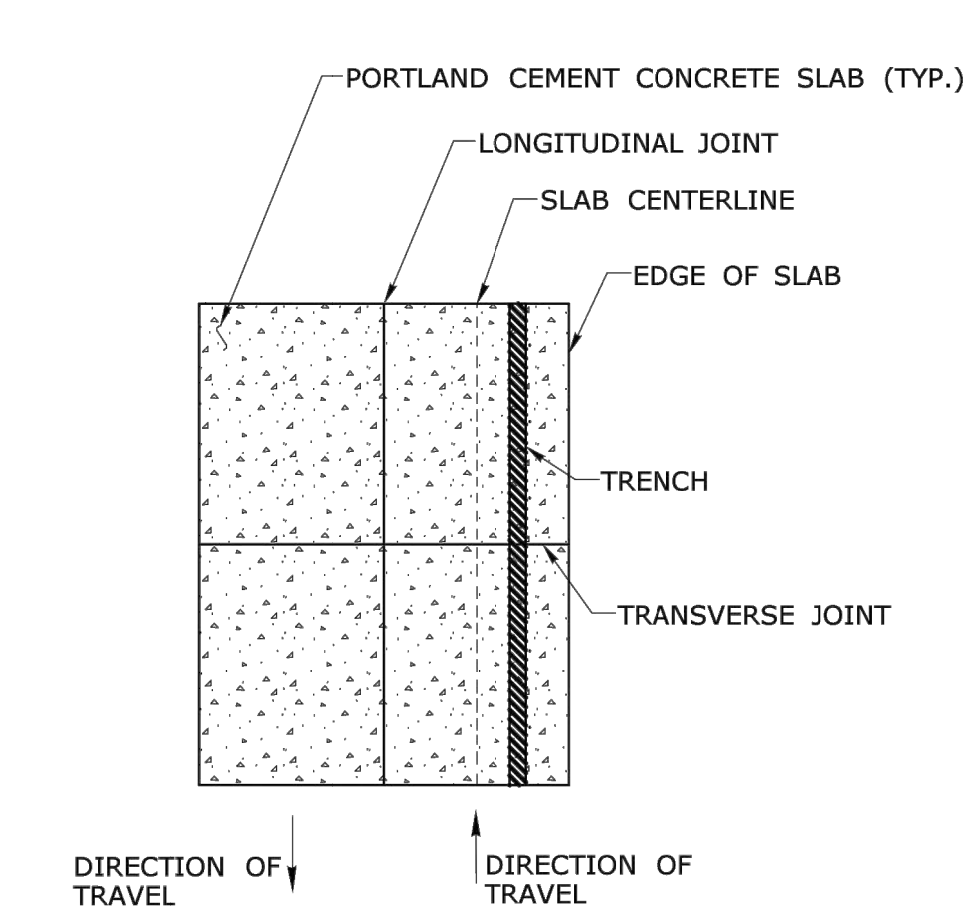
TEMPORARY PAVEMENT REPAIR FOR TRENCH THROUGH OVERLAID PORTLAND CEMENT CONCRETE OR BITUMINOUS CONCRETE PAVEMENT



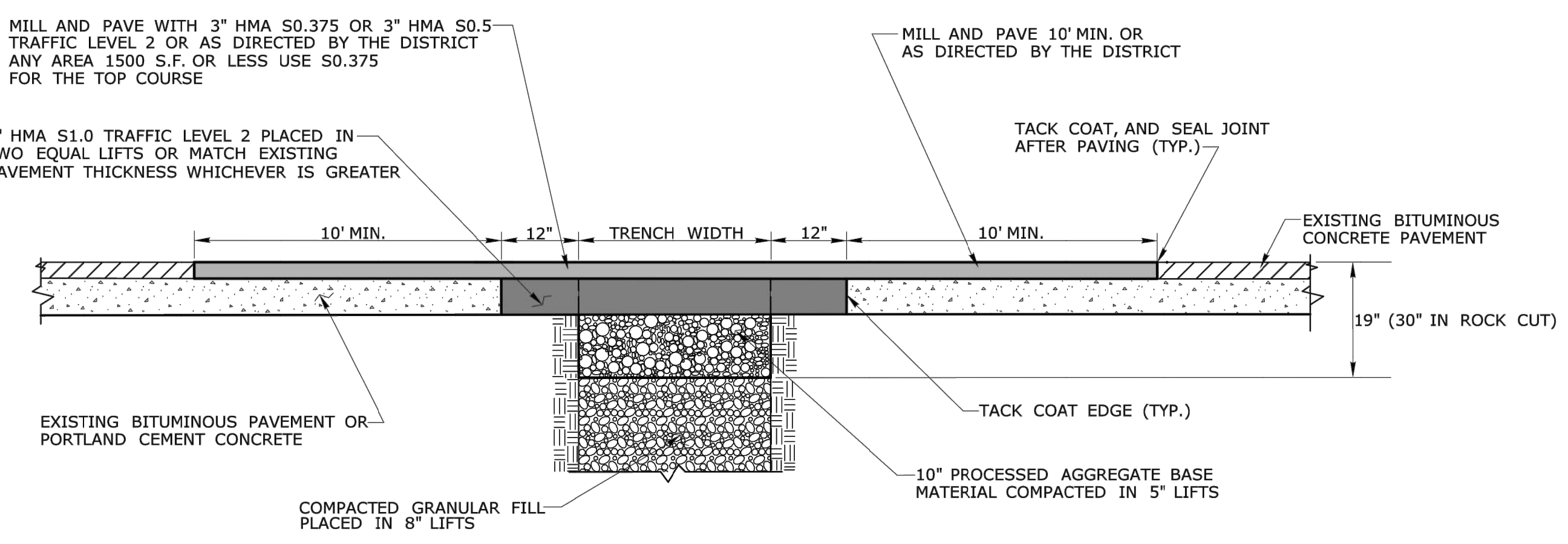
PERMANENT PAVEMENT REPAIR WITHOUT MILLING - THROUGH PORTLAND CEMENT CONCRETE OR BITUMINOUS CONCRETE PAVEMENT



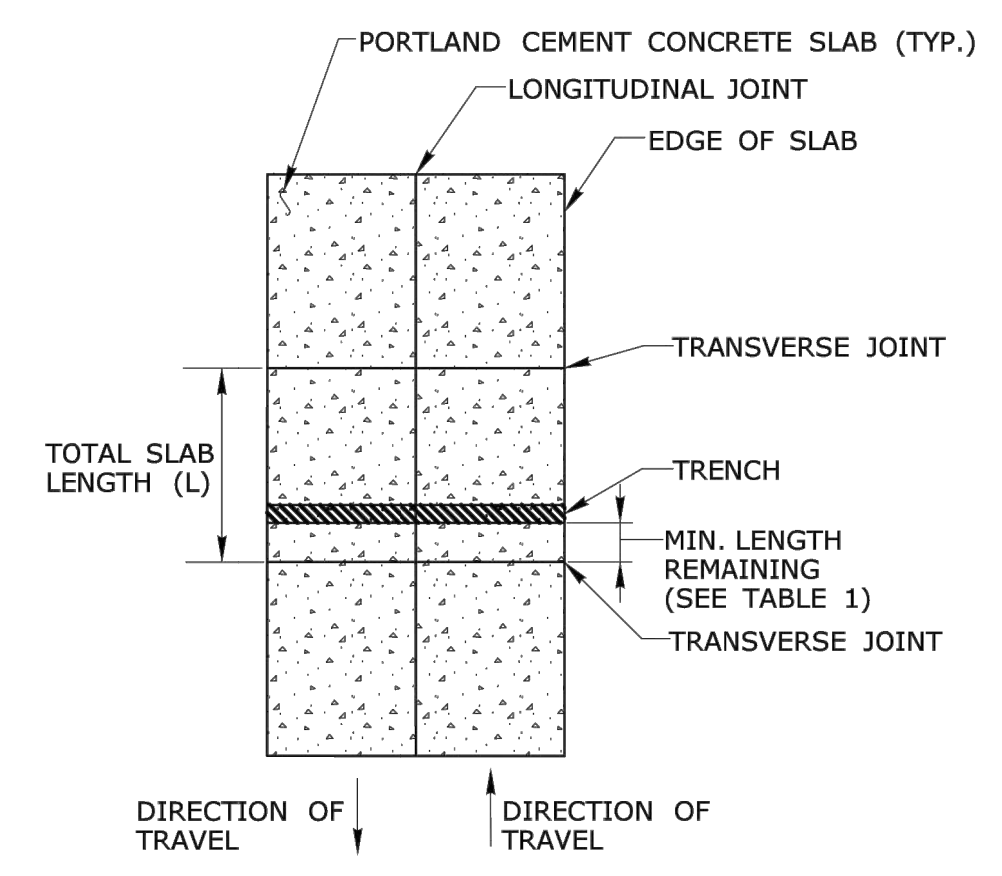
ROADWAY PROFILE



LONGITUDINAL TRENCHING FOR JOINTED CONCRETE PAVEMENT (SEE NOTE 1)



PERMANENT PAVEMENT REPAIR WITH MILLING



TRANSVERSE TRENCHING FOR JOINTED CONCRETE PAVEMENT (SEE NOTE 2)

DRAFTER: MS CHECKED BY: EL NO SCALE	HIGHWAY OPERATIONS	OFFICE OF MAINTENANCE OPERATIONS SPECIAL SERVICES AND PLANNING	 STATE OF CONNECTICUT DEPARTMENT OF TRANSPORTATION	DRAWING TITLE: ENCROACHMENT PERMIT - PAVEMENT REPAIR
---	---------------------------	---	---	--

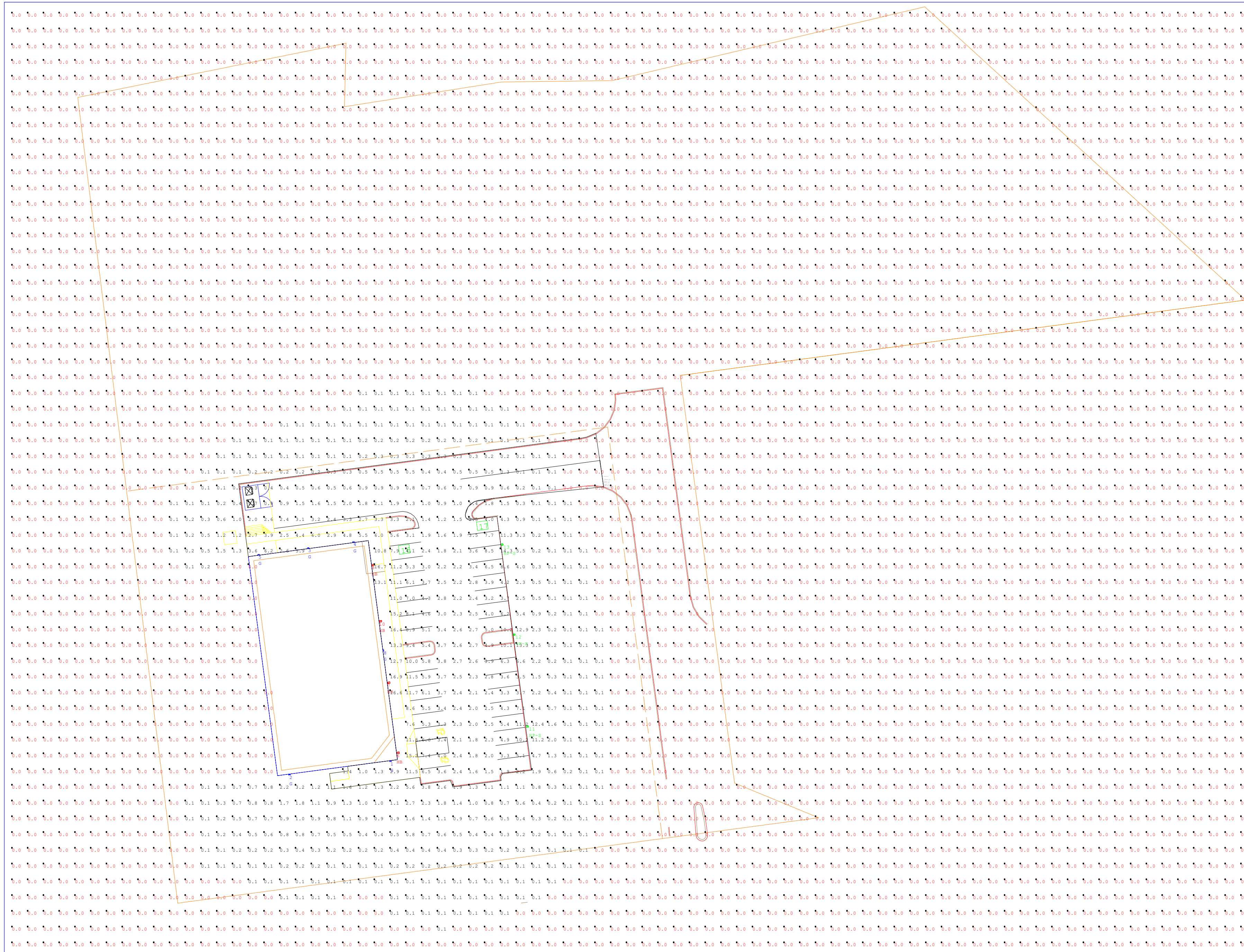
BOLTON PLANNING AND ZONING COMMISSION, BOLTON, CT

DATE APPROVED _____ DATE OF EXPIRATION _____

CHAIRMAN

THE STATUTORY FIVE-YEAR PERIOD FOR COMPLETION OF ALL PHYSICAL IMPROVEMENTS EXPIRES ON _____

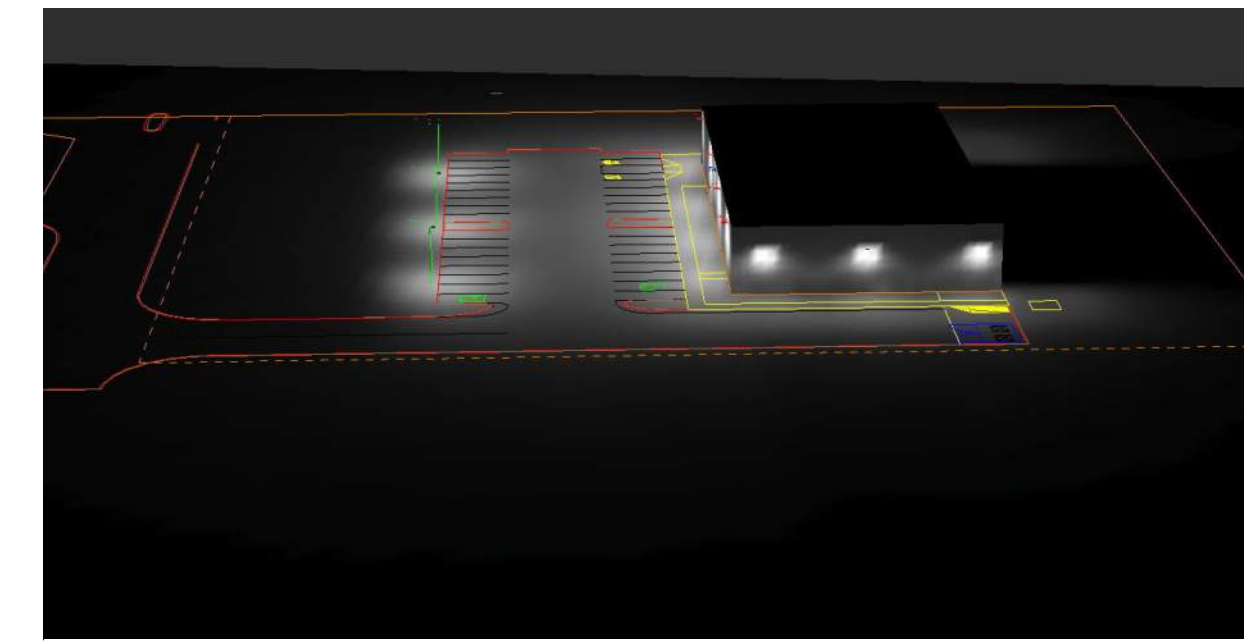
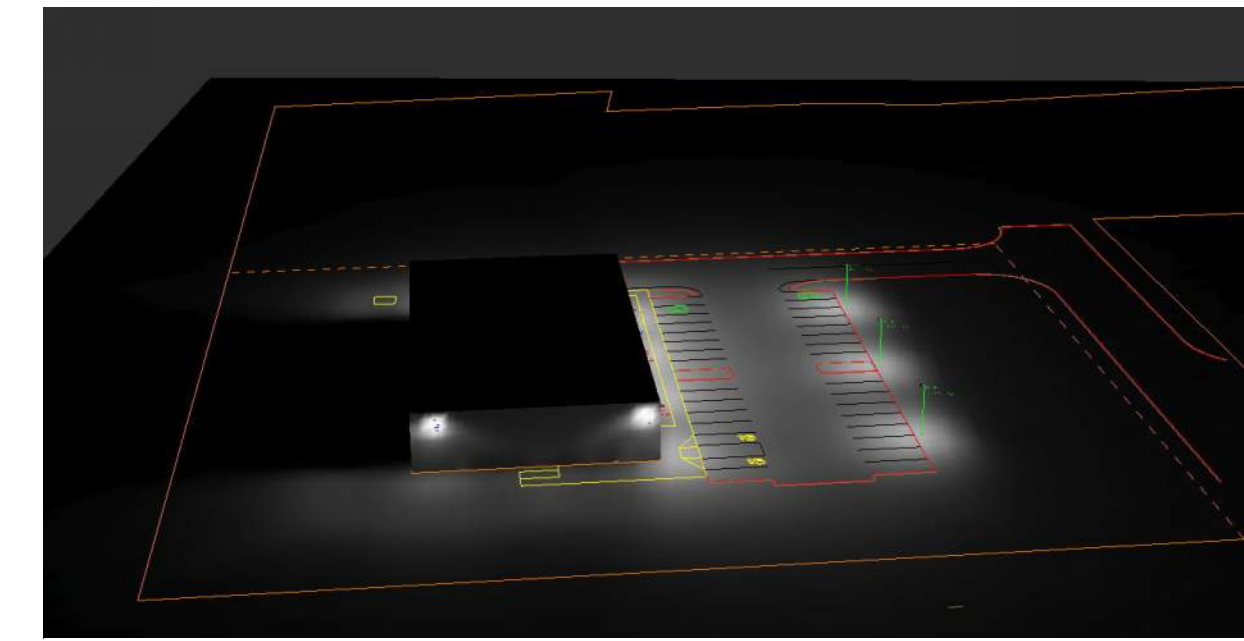
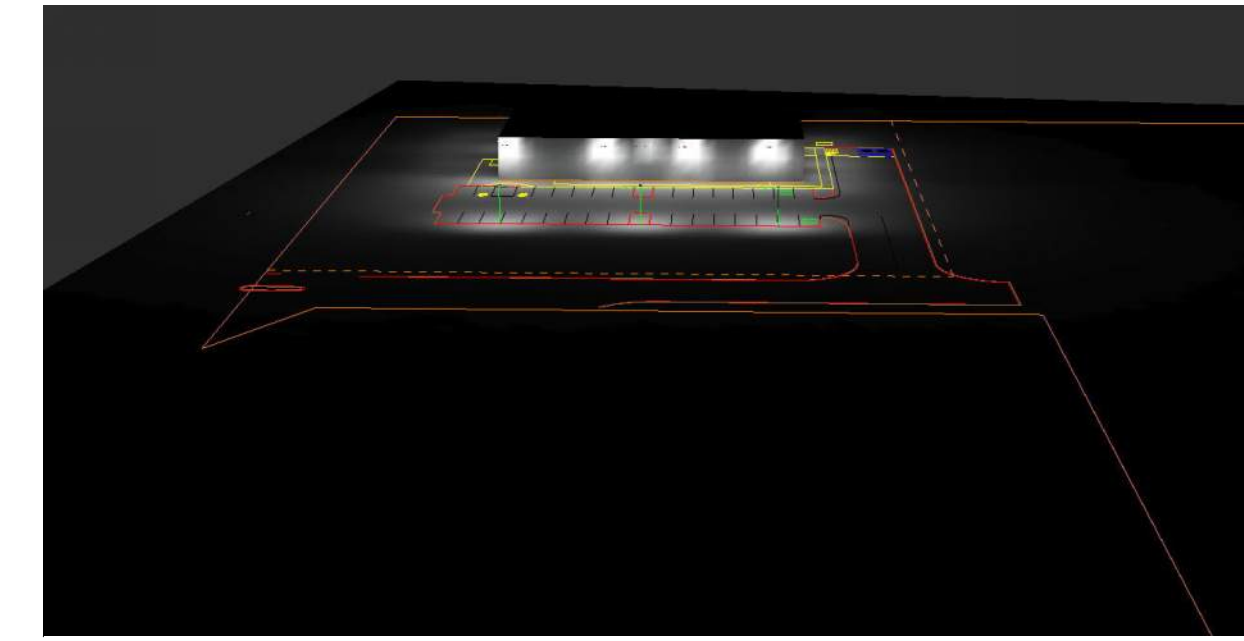
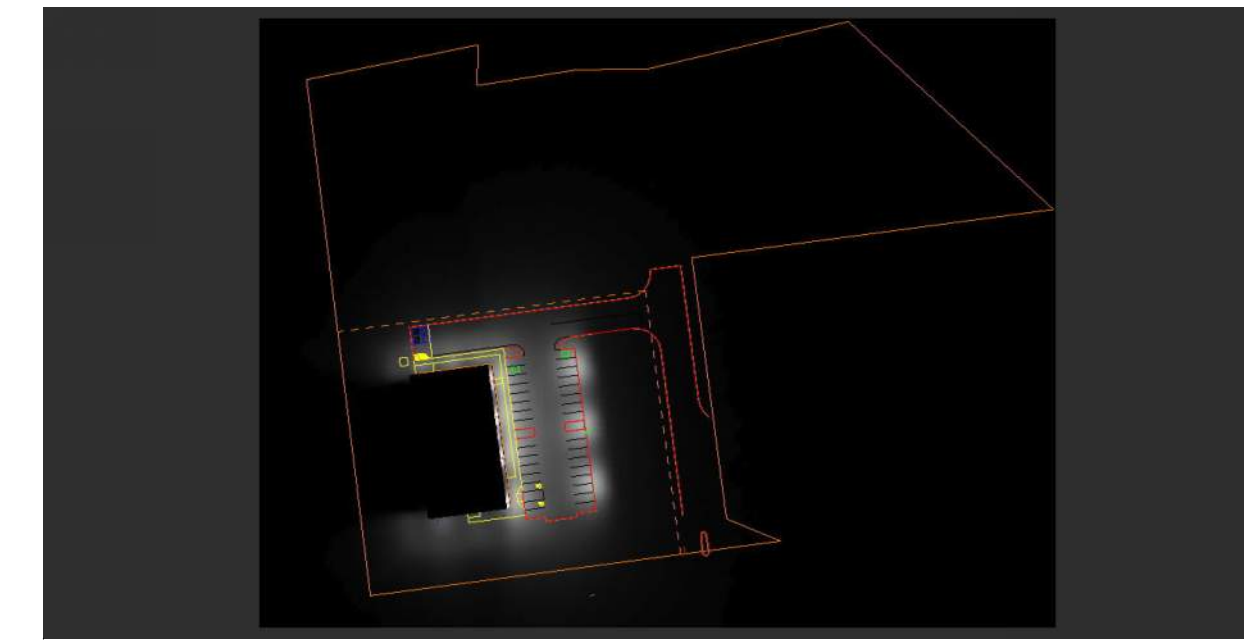
**FOR PERMITTING PURPOSES ONLY
NOT RELEASED FOR CONSTRUCTION**



LumNo	Label	Z
1	G	16.5
2	G	16.5
3	G	12
4	G	12
5	G	12
6	HB	16.5
7	HB	16.5
8	G	16.5
9	HB	16.5
10	HB	16.5
11	HP-S	17
12	HP-S	17
13	HP-S	17

Symbol	Qty	Label	Arrangement	Total Lamp Lumens	LLF	Description
[Green Box]	3	HP-S	SINGLE	13632	0.950	LEDS-1210-S - Single Pole Mt 150w, Type 4, 5K, Shielded
[Red Box]	4	HB	SINGLE	19188	0.950	LEDS-AL120 - Wall Mt, 150w, Type 4, 5K
[Blue Box]	6	G	SINGLE	4740	0.950	LEDBG42W001B-5000K - Wall Pack, 42W, Full Cutoff, 5K

Label	CalcType	Units	Avg	Max	Min	Avg/Min	Max/Min
Site	Illuminance	Fc	0.24	16.9	0.0	N.A.	N.A.
Parking Lot	Illuminance	Fc	3.75	16.9	0.0	N.A.	N.A.



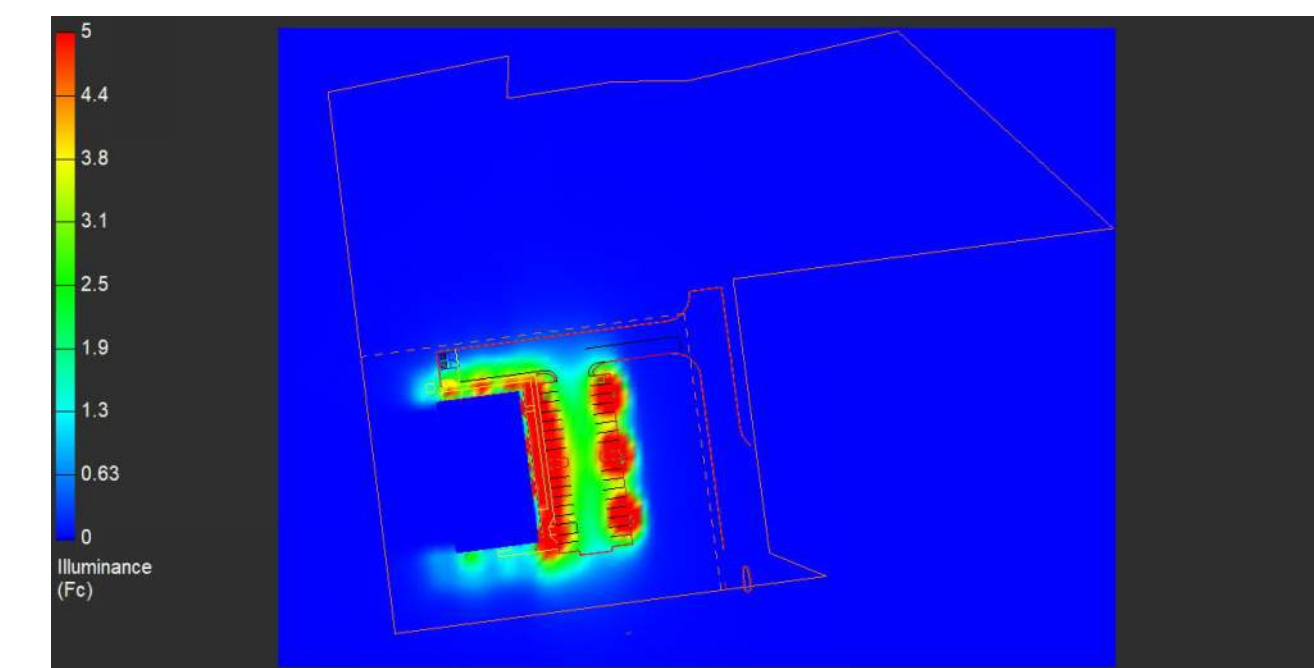
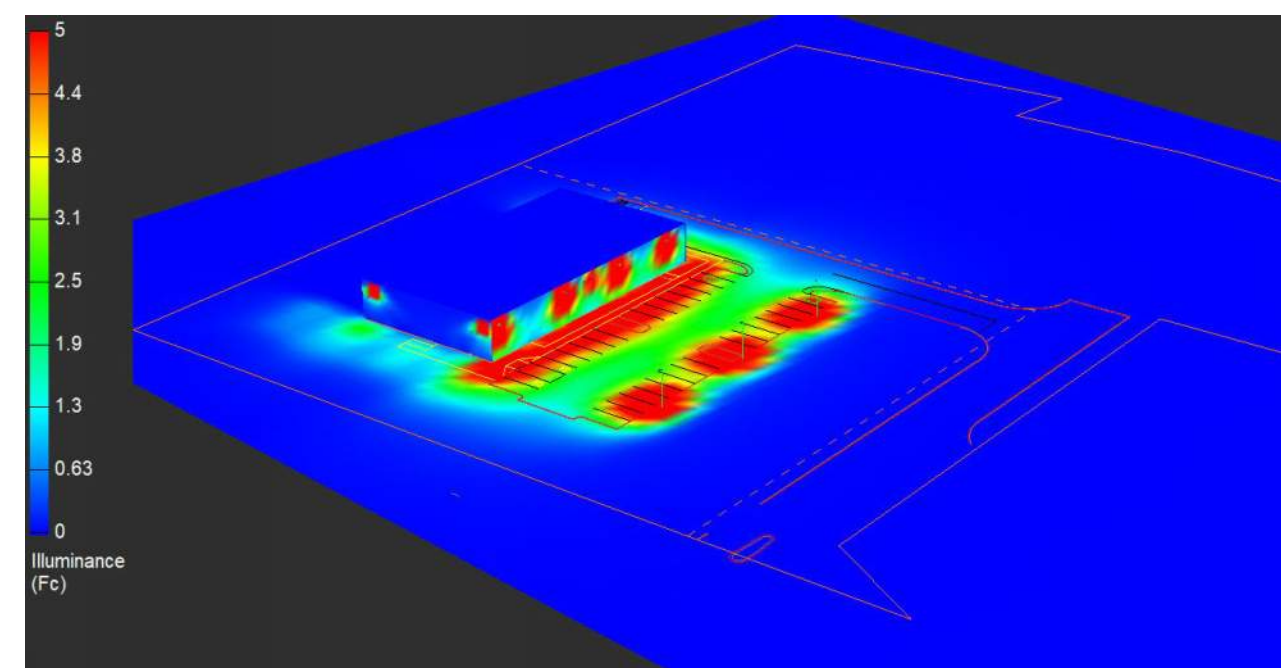
#	Date	Comments
Revisions		

Drawn By: BMF, LC
 Checked By:
 Date: 3/29/2021
 Scale:

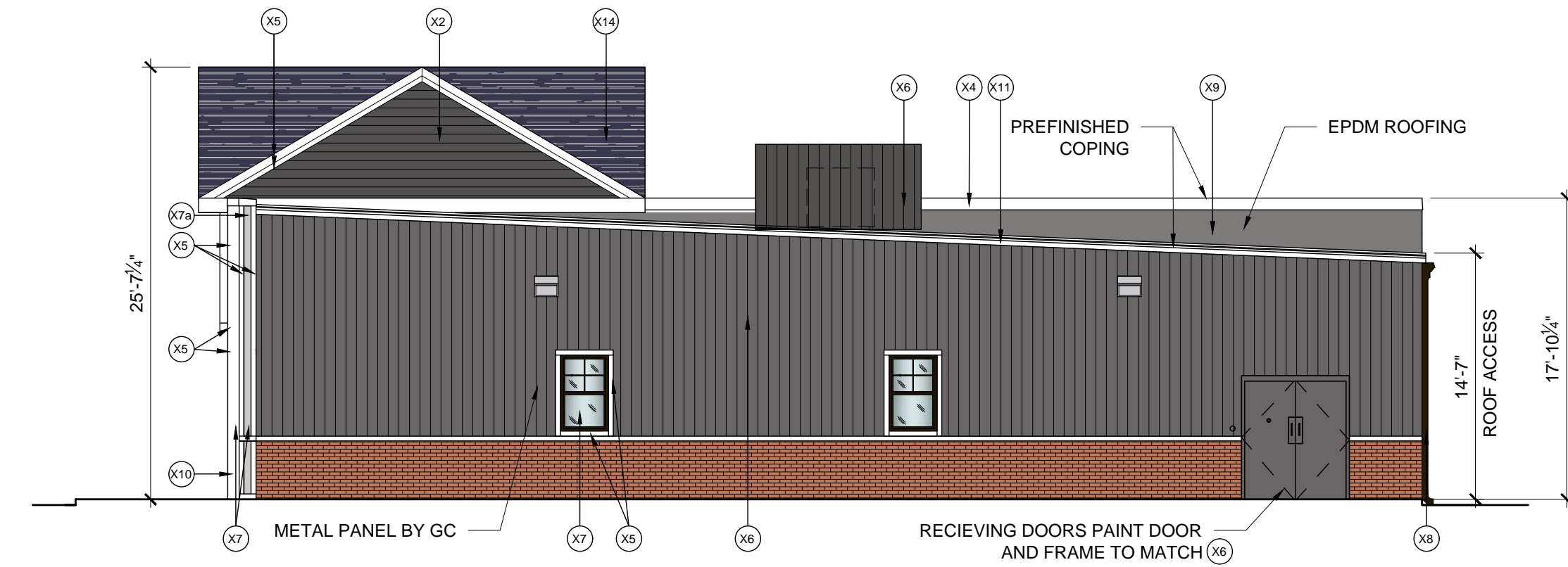
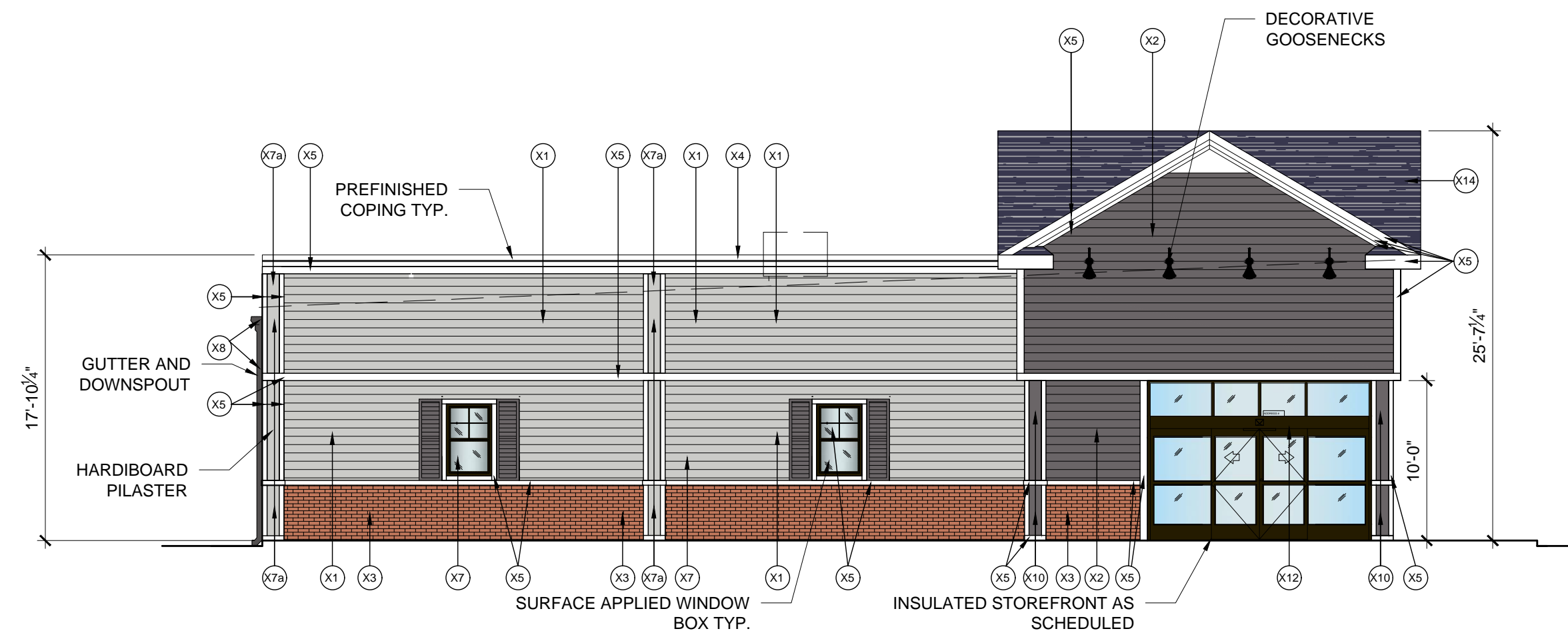
Notes:

Plan Notes:
 Calculations at Ground Level (10' x 10' Grid Spacing). Refer to luminaire location summary for mounting heights of each fixture. Pole mounted fixtures include a 2ft concrete base. Mounting heights indicated on luminaire location summary is a total A.F.G. height.

General Notes:
 Due to changing lighting ordinances it is the contractors responsibility to submit the site photometrics & luminaire specs to the local inspector before ordering to ensure this plan complies with local lighting ordinances. This lighting design is based on information supplied by others. Changes in electrical supply, area geometry & objects within the lighted area may produce illumination values different from the predicted results shown on this layout. This layout is based on .IES files that were lab tested or computer generated, actual results may vary.

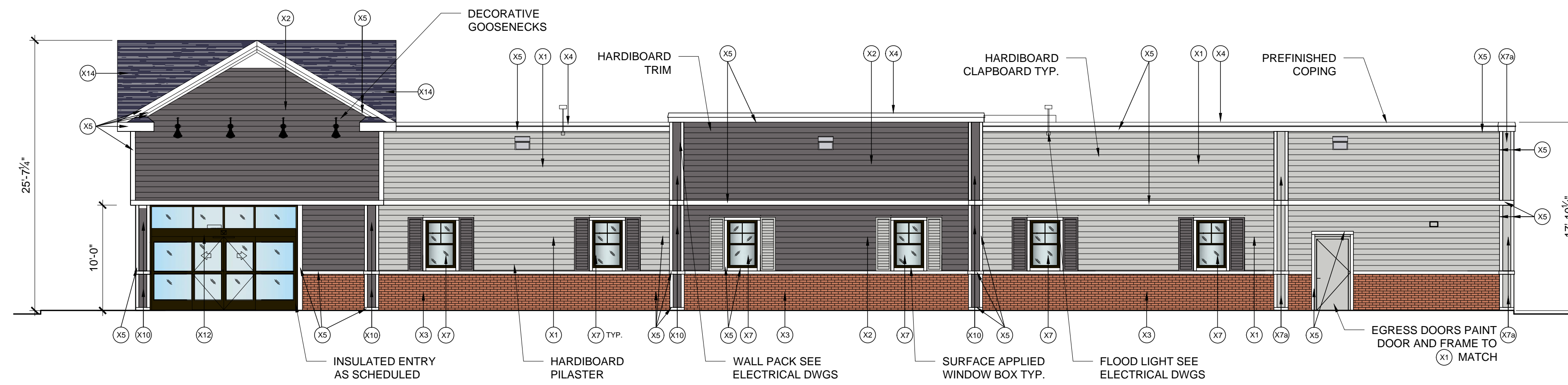


Bolton CT 23232



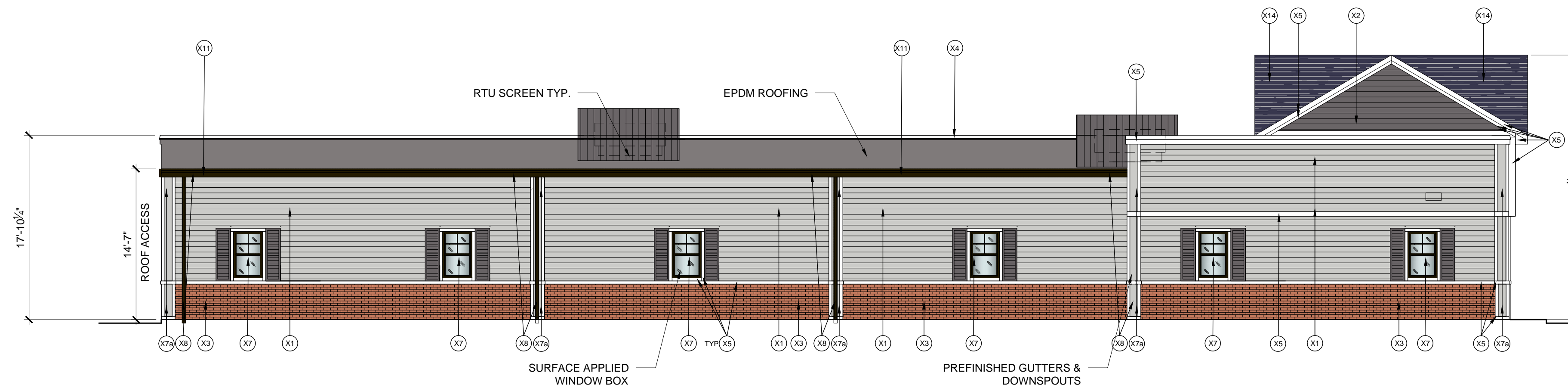
1 PROPOSED SIDE ELEVATION (BOSTON TURNPIKE RT 44)
SCALE: 3/16"-1'-0"

2 PROPOSED SIDE ELEVATION
SCALE: 3/16"-1'-0"

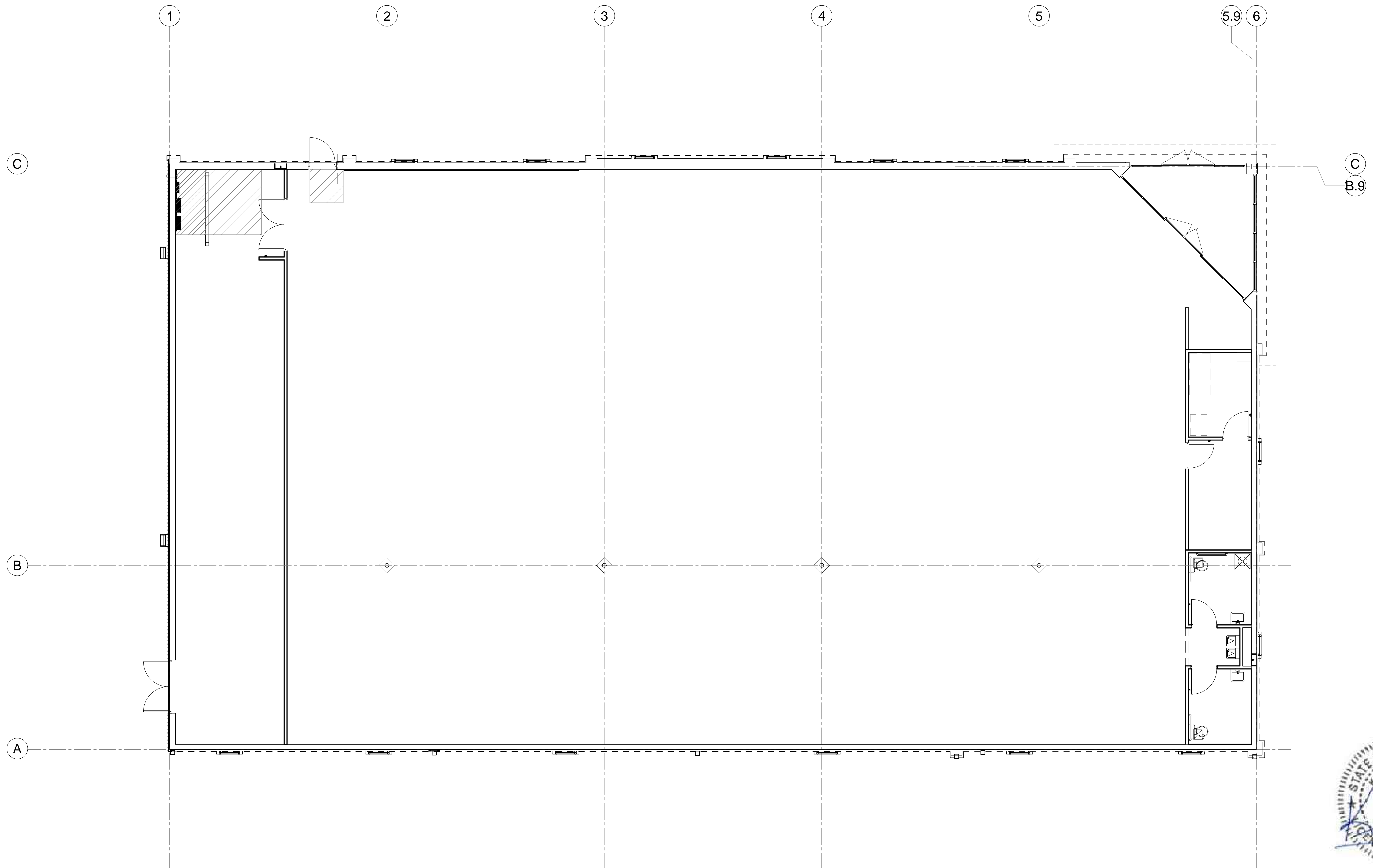


3 PROPOSED FRONT ELEVATION
SCALE: 3/16"-1'-0"

EXTERIOR FINISH SCHEDULE			
TAG	MATERIAL/ MFG.	COLOR/ NO.	NOTES
X1	HARDIEE-PLANK LAP SIDING	COLOR: PEARL GRAY	PRE-FINISHED; 6" EXPOSURE
X2	HARDIEE-PLANK LAP SIDING	COLOR: NIGHT GRAY	PRE-FINISHED 6" EXPOSURE
X3	VEE BRICK	COLOR: TAVERN FLASH	
X4	METAL COPING	COLOR: WHITE	PRE-FINISHED
X5	HARDIE-BOARD TRIM	COLOR: WHITE TO MATCH (X4)	COPING BY GC PRE-FINISHED
X6	METAL WALL PANEL	COLOR: CHARCOAL GRAY O.A.E	PRE-FINISHED
X7	SURFACE APPLIED STOREFRONT	COLOR: DARK BRONZE	LIGHT GRAY SPANDREL
X7a	HARDIE-BOARD TRIM	COLOR: TO MATCH (X1)	PAINTED
X8	GUTTER & DOWNSPOUT	COLOR: DARK BRONZE	PRE-FINISHED
X9	EPDM ROOF	COLOR: DARK GRAY	PRE-FINISHED
X10	HARDIE-BOARD TRIM	COLOR: TO MATCH (X2)	PAINTED
X11	METAL COPING	COLOR: CHARCOAL GRAY	PRE-FINISHED
X12	INSULATED SLIDING ENTRY DOORS	COLOR: DARK BRONZE	PRE-FINISHED
X13	METAL DOOR & FRAME	COLOR: TO MATCH (X2)	PAINTED
X14	ARCH ASPHALT SHINGLES	COLOR: PEWTER GREY	GAF TIMBERLINE



4 PROPOSED REAR ELEVATION
SCALE: 3/16"-1'-0"



1 PROPOSED FLOOR PLAN
SCALE: 3/16"=1'-0"

Conceptual Floor Plan

Bolton, CT Retail Building Conceptual Scheme
1100 Boston Turnpike Bolton, CT

Prepared For Garrett Homes

SCALE: NOTED
31, March 2021

1 OF 1
DRAWN BY: DSG
PROJECT NO: 221003

BKA ARCHITECTS

Boston + Brockton
142 Crescent Street
Brockton, MA 02302
508.583.5603
bkaarchitects.com

Stormwater Management Narrative and Hydrologic Calculations Proposed Retail Development – 1100 Boston Turnpike – Bolton, CT May 5, 2021

This narrative has been prepared in support of a Permit Application by Garrett Homes, LLC to the Town of Bolton for the proposed retail development at 1100 Boston Turnpike. The property is approximately 1.85 acres in size and is currently an undeveloped parcel. The property is located on the northern side of Boston Turnpike and is roughly bordered by residential properties to the west and south and a dentist office on the previously subdivided parcel to the east. The site is bordered by undeveloped woodland and Bolton Lake to the north. The subject parcel described in this report is proposed to be subdivided from “Parcel 2” to the north.

Existing Site Conditions

The project parcel is currently undeveloped, consisting entirely of lawn area. There are no formal stormwater management systems currently located on site. Stormwater from the subject property sheet flows untreated to the adjacent properties.

The site soil identified by the United States Department of Agriculture (USDA) Natural Resources Conservation Services (NRCS) is Woodbridge fine sandy loam, 3 to 8 percent slopes, Woodbridge fine sandy loam, 0 to 8 percent slopes, very stony, and Canton and Charlton fine sandy loams, 3 to 8 percent slopes. Per the USDA, the NRCS Hydrologic Soil Group rating for within the project area is C/D, C/D, and B respectively. For the Soil Group ratings of C/D, a Soil Group rating of C was assumed in order to be conservative in the change of curve number from grass to impervious.

Developed Site Conditions

The proposed site improvements will include a 10,640 square foot retail building, paved parking areas, landscaped areas, pedestrian sidewalks, site utilities and lighting, and a stormwater management system.

The proposed stormwater management system will utilize a surface bioretention basin for stormwater quality treatment and peak flow mitigation of stormwater runoff generated by impervious surfaces eventually draining to the neighboring property to the east.

Stormwater Management – Existing Drainage Patterns

The existing site drainage area that was analyzed totals 4.92 acres and is approximately 8% impervious.

Stormwater from the subject property sheet flows untreated to the adjacent properties. There is a ridge line that roughly bifurcates the site into two main drainage areas. The northern portion of the project parcel and neighboring properties sheet flow to the wetland to the northeast of the site (Design Point 1). The northern portion of the site consists of primarily of grassed surface cover

with some wooded and impervious surface cover. The southeastern portion of the project parcel sheet flows to the existing catch basins within the shared driveway to the east that drains to the stormwater management system located within the previously subdivided parcel to the east (Design Point 2). The southeastern portion of the site consists mainly of grassed area with some impervious area from the shared driveway.

Stormwater Management – Proposed Drainage Patterns

The proposed site drainage area totals 4.92 acres and is approximately 28% impervious.

The same Design Points used in the existing conditions analysis have been retained for the proposed analysis. The site stormwater system will provide stormwater retention and quality improvements through the installation of a Bioretention Basin with a grass filter strip and a formalized street sweeping program for the impervious surfaces. These measures will treat the stormwater quality flow through structural means to provide water quality treatment in conformance with the State of Connecticut Water Quality Manual. The proposed stormwater management system has been designed to treat the runoff generated by the proposed development for a minimum 80% TSS removal as required in the CT Stormwater Quality Manual, retain and infiltrate the Water Quality Volume, and provide groundwater recharge.

As noted from town staff during a pre-application meeting, due to the site being in the lower reach of the watershed peak flow mitigation has not been deemed necessary to the wetland located northeast of the site (Design Point 1). Peak flow to the existing offsite drainage system on the parcel to the east (Design Point 2) will be matched in the 2, 10, and 100-year storms to ensure the proposed development will not negatively impact the existing neighboring drainage system, as seen in the peak flow rate comparison table below.

Drainage Area	Peak Flow Rate in Cubic Feet per Second (cfs)		
	2-yr	10-yr	100-yr
Design Point 1 Wetland to Northeast			
Existing	2.6	6.3	12.9
Proposed	2.0	4.7	9.3
Percent Change	-23.1%	-25.4%	-27.9%
Design Point 2 Ex. CBs in Driveway			
Existing	0.9	1.8	3.3
Proposed	0.9	1.8	3.2
Percent Change	0.0%	0.0%	-3.0%

Conclusion

The post-development peak discharge rates for the total developed site have been decreased or matched for all storm events. All post development stormwater will be discharged offsite to mimic existing drainage patterns. The proposed Bioretention Basin been designed to attenuate peak flows to Design Point 2 at the offsite drainage system, while providing water quality improvements. Though it was not necessary to match peak flows to Design Point 1, the flow has been mitigated by reducing the size of the contributing drainage area. The area removed from the drainage area to Design Point 1 now contributes to the proposed Bioretention Basin drainage area, which ultimately discharges to Design Point 2.

This letter has been prepared to compliment the submitted project plans and full Stormwater Management Report, as well as to represent the technical basis for the designs presented herein.

APPENDIX A

DRAINAGE MAPS





ED-1 – Existing Hydrology Mapping

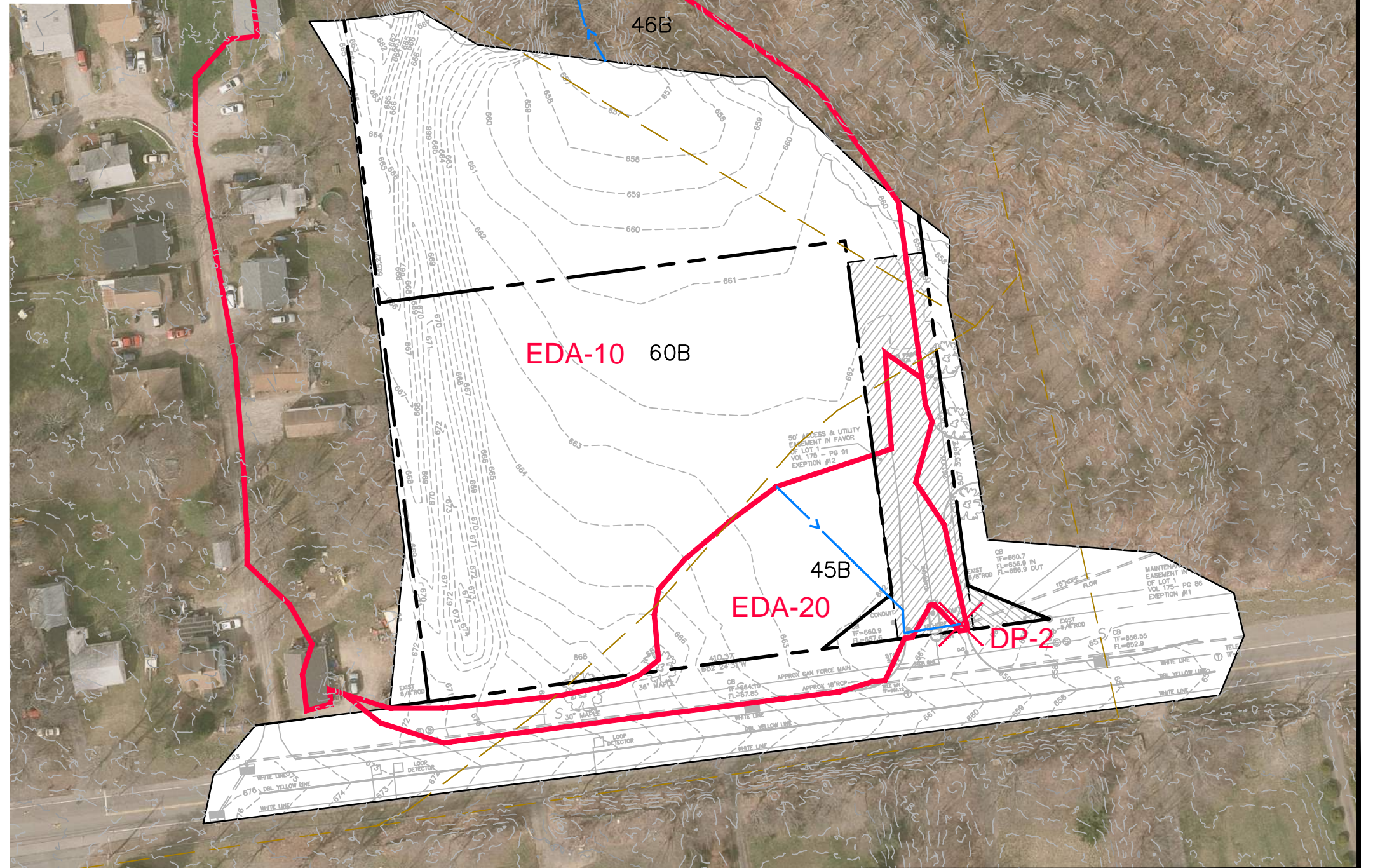
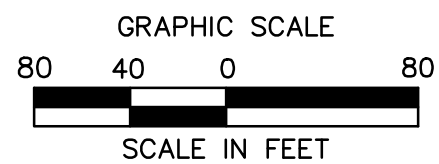
PD-1 – Proposed Hydrology Mapping

EXISTING HYDROLOGY INFORMATION

DRAINAGE AREA	TOTAL AREA (S.F.)	IMPERVIOUS AREA (S.F.)	PERVIOUS AREA (S.F.)	PERCENT IMPERVIOUS (%)	CN	TIME OF CONCENTRATIONS (MIN.)
EDA-10	185,210	12,135	173,075	6.6%	72	14.5
EDA-20	29,230	4,605	24,625	15.8%	81	25.3

HYDROLOGY LEGEND

	PROPERTY LINE
	DRAINAGE AREA BOUNDARY
	TIME OF CONCENTRATION FLOW PATH
	SOIL TYPE BOUNDARY
306	SOIL TYPE DESIGNATION



EXISTING DRAINAGE MAPPING

PROPOSED RETAIL DEVELOPMENT
1100 BOSTON TURNPIKE
BOLTON, CONNECTICUT

Designed	S.E.L.
Drawn	S.E.L.
Reviewed	J.A.B.
Scale	1"=80'
Project No.	2002032
Date	04/02/2021
CAD File	ED200203201

ED-1








ARCHITECTURE
ENGINEERING
ENVIRONMENTAL
LAND SURVEYING

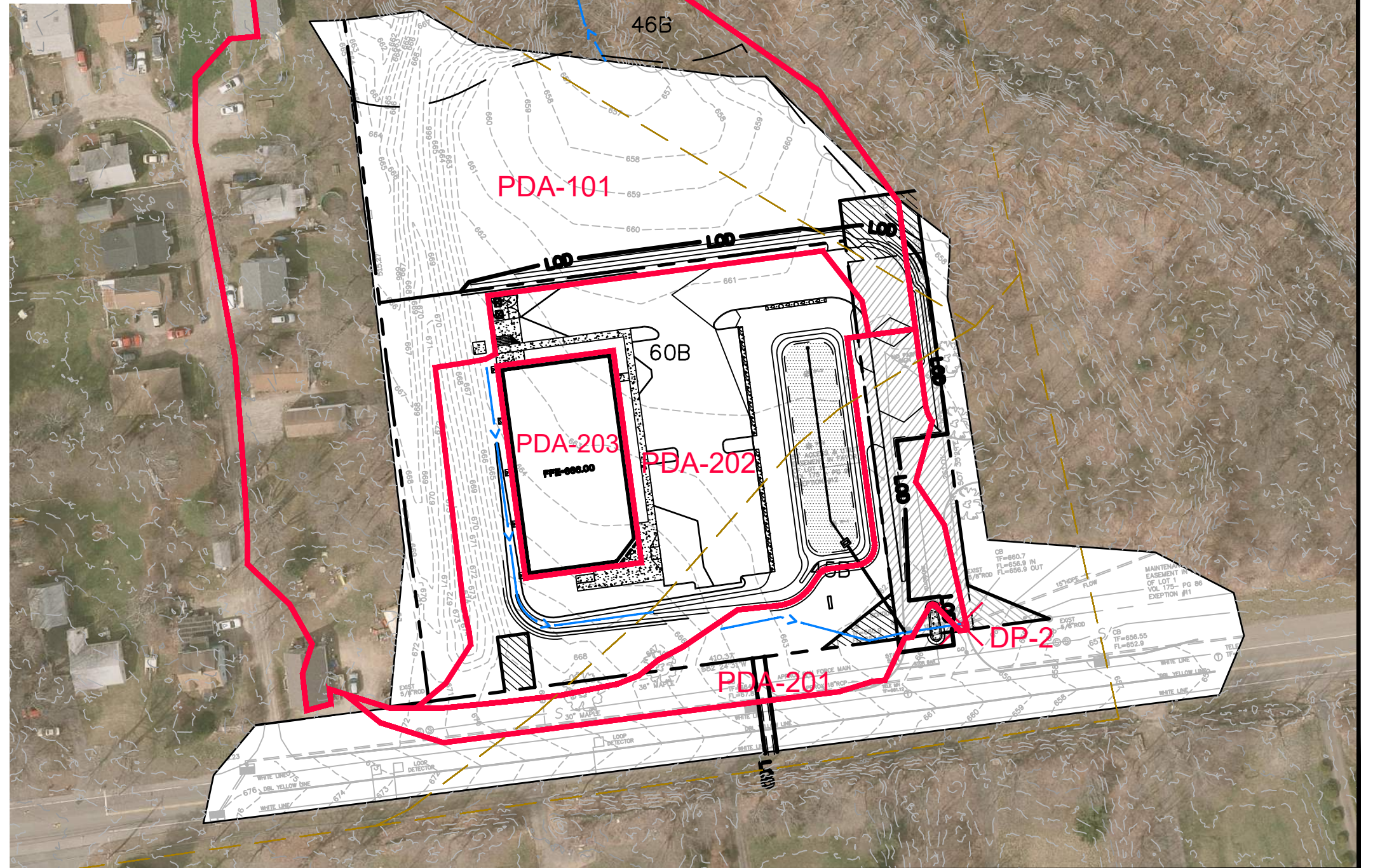
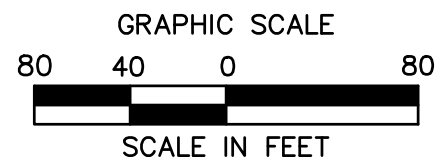
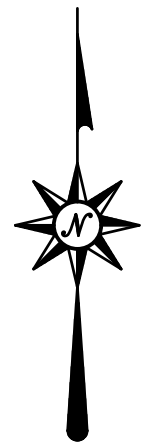
100 Constitution Plaza, 10th Floor
Hartford, CT 06103
(860) 249-2200
(860) 249-2400 Fax

PROPOSED HYDROLOGY INFORMATION

DRAINAGE AREA	TOTAL AREA (S.F.)	IMPERVIOUS AREA (S.F.)	PERVIOUS AREA (S.F.)	PERCENT IMPERVIOUS (%)	CN	TIME OF CONCENTRATIONS (MIN.)
PDA-101	127,015	15,295	111,720	12.0%	74	25.0
PDA-201	22,250	5,970	16,280	26.8%	83	9.8
PDA-202	54,405	27,695	26,710	50.9%	85	16.3
PDA-203	10,770	10,770	0	100.0%	98	5.0

HYDROLOGY LEGEND

-  PROPERTY LINE
-  DRAINAGE AREA BOUNDARY
-  TIME OF CONCENTRATION FLOW PATH
-  SOIL TYPE BOUNDARY
-  SOIL TYPE DESIGNATION



PROPOSED DRAINAGE MAPPING

PROPOSED RETAIL DEVELOPMENT
1100 BOSTON TURNPIKE
BOLTON, CONNECTICUT

Designed	S.E.L.
Drawn	S.E.L.
Reviewed	J.A.B.
Scale	1"=80'
Project No.	2002032
Date	05/04/2021
CAD File	PD200203201

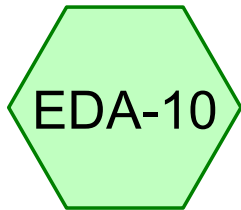
PD-1



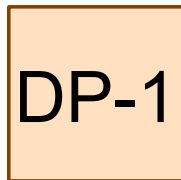
ARCHITECTURE
ENGINEERING
ENVIRONMENTAL
LAND SURVEYING

100 Constitution Plaza, 10th Floor
Hartford, CT 06103
(860) 249-2200
(860) 249-2400 Fax

APPENDIX B
PRE-DEVELOPMENT HYDROLOGY



Area to Wetland to the Northeast



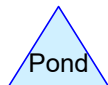
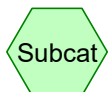
Wetland to Northeast



Area to Ex. CBs in Driveway



Ex. CBs in Driveway



Routing Diagram for C-DAT-2002032-EXISTING HYDROLOGY
Prepared by {enter your company name here}, Printed 5/4/2021
HydroCAD® 10.00-25 s/n 01334 © 2019 HydroCAD Software Solutions LLC

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-10: Area to Wetland to Runoff Area=185,210 sf 6.55% Impervious Runoff Depth>0.99"
Flow Length=136' Slope=0.0145 '/' Tc=25.0 min CN=72 Runoff=2.57 cfs 0.350 af

Subcatchment EDA-20: Area to Ex. CBs in Runoff Area=29,230 sf 15.75% Impervious Runoff Depth>1.55"
Flow Length=169' Tc=14.3 min CN=81 Runoff=0.92 cfs 0.087 af

Reach DP-1: Wetland to Northeast Inflow=2.57 cfs 0.350 af
Outflow=2.57 cfs 0.350 af

Reach DP-2: Ex. CBs in Driveway Inflow=0.92 cfs 0.087 af
Outflow=0.92 cfs 0.087 af

Total Runoff Area = 4.923 ac Runoff Volume = 0.437 af Average Runoff Depth = 1.06"
92.19% Pervious = 4.539 ac 7.81% Impervious = 0.384 ac

Summary for Subcatchment EDA-10: Area to Wetland to the Northeast

Runoff = 2.57 cfs @ 12.31 hrs, Volume= 0.350 af, Depth> 0.99"

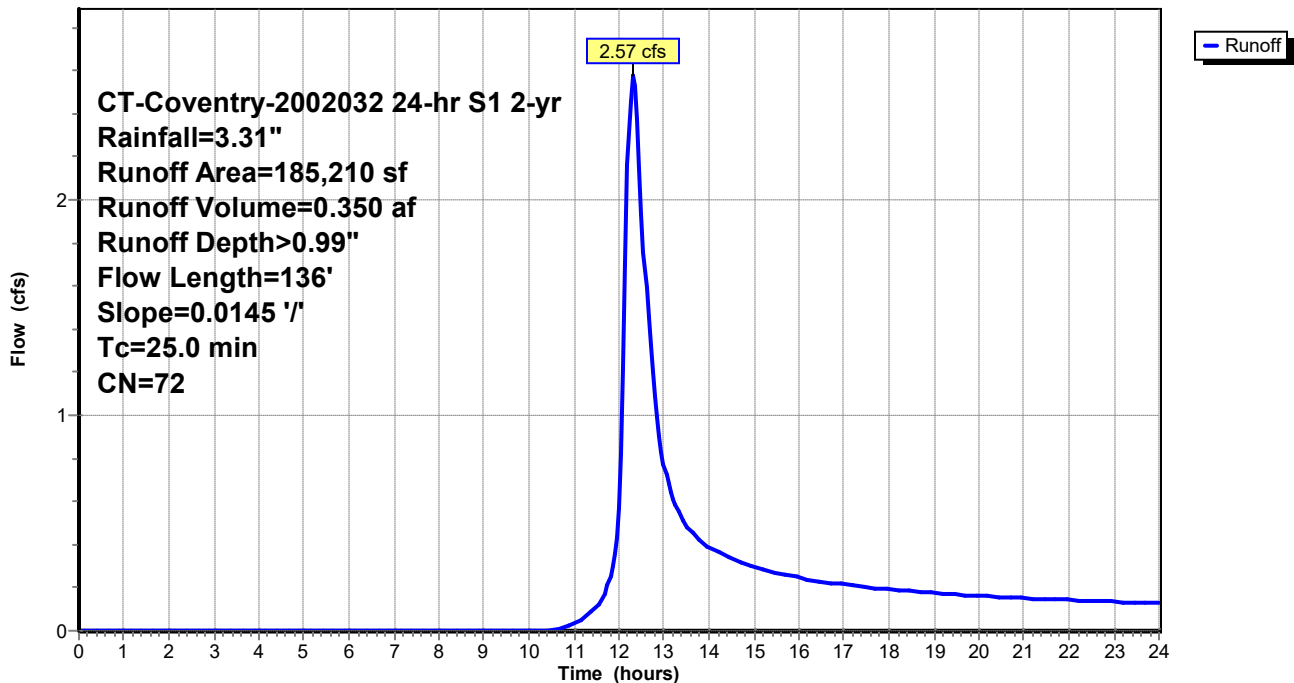
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"

Area (sf)	CN	Description
134,225	69	50-75% Grass cover, Fair, HSG B
15,340	79	50-75% Grass cover, Fair, HSG C
2,445	60	Woods, Fair, HSG B
21,065	73	Woods, Fair, HSG C
12,135	98	Paved parking, HSG B
0	98	Paved parking, HSG C
185,210	72	Weighted Average
173,075		93.45% Pervious Area
12,135		6.55% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.0	100	0.0145	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.31"
1.0	36	0.0145	0.60		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
25.0	136	Total			

Subcatchment EDA-10: Area to Wetland to the Northeast

Hydrograph



Summary for Subcatchment EDA-20: Area to Ex. CBs in Driveway

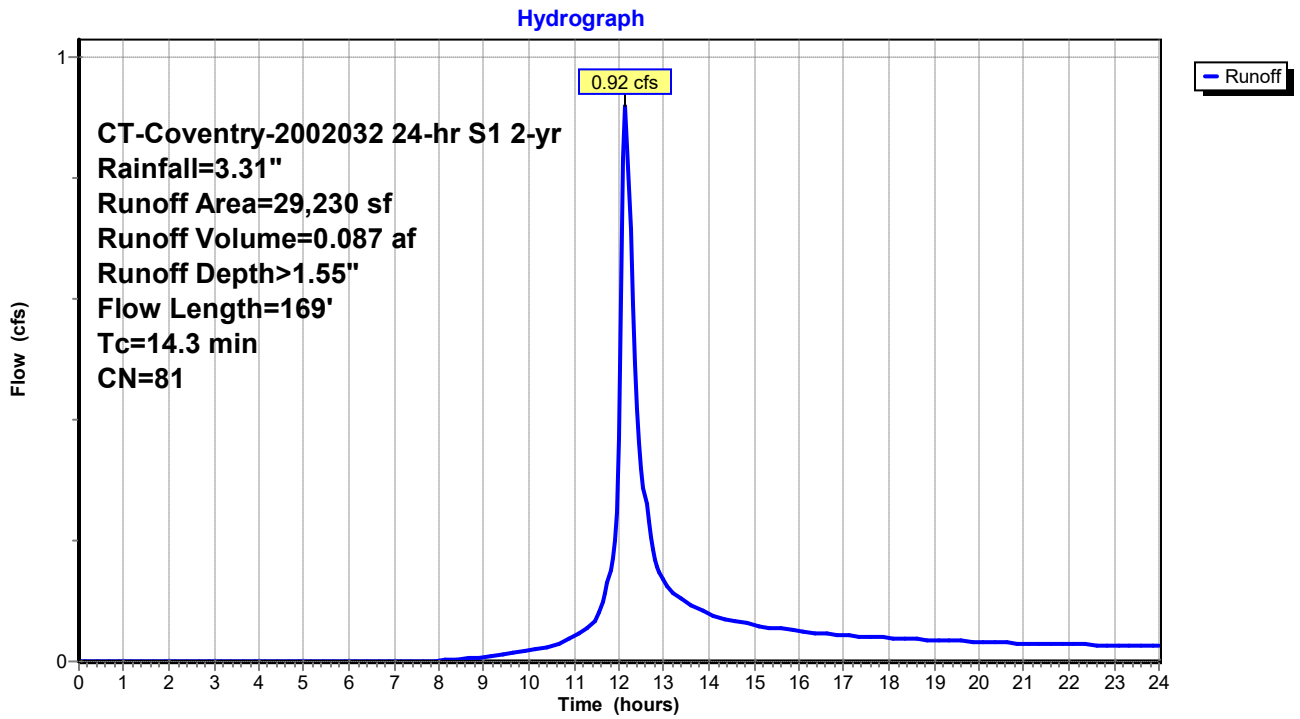
Runoff = 0.92 cfs @ 12.15 hrs, Volume= 0.087 af, Depth> 1.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"

Area (sf)	CN	Description
2,335	69	50-75% Grass cover, Fair, HSG B
22,290	79	50-75% Grass cover, Fair, HSG C
0	60	Woods, Fair, HSG B
0	73	Woods, Fair, HSG C
185	98	Paved parking, HSG B
4,420	98	Paved parking, HSG C
29,230	81	Weighted Average
24,625		84.25% Pervious Area
4,605		15.75% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.9	100	0.0080	0.12		Sheet Flow, Grass: Short n= 0.150 P2= 3.31"
0.2	16	0.0284	1.18		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.1	15	0.0100	2.03		Shallow Concentrated Flow, Paved Kv= 20.3 fps
0.1	38	0.0185	7.16	8.79	Pipe Channel, 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.013 Corrugated PE, smooth interior
14.3	169	Total			

Subcatchment EDA-20: Area to Ex. CBs in Driveway

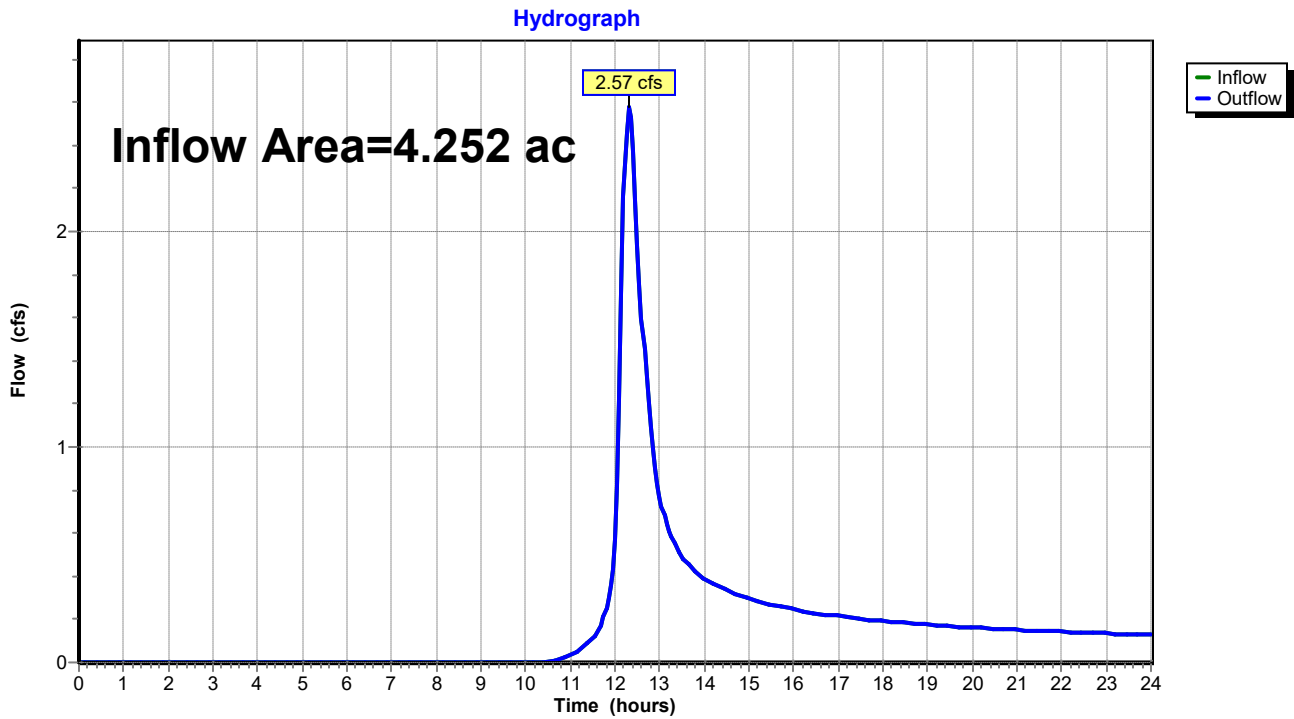


Summary for Reach DP-1: Wetland to Northeast

Inflow Area = 4.252 ac, 6.55% Impervious, Inflow Depth > 0.99" for 2-yr event
Inflow = 2.57 cfs @ 12.31 hrs, Volume= 0.350 af
Outflow = 2.57 cfs @ 12.31 hrs, Volume= 0.350 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach DP-1: Wetland to Northeast

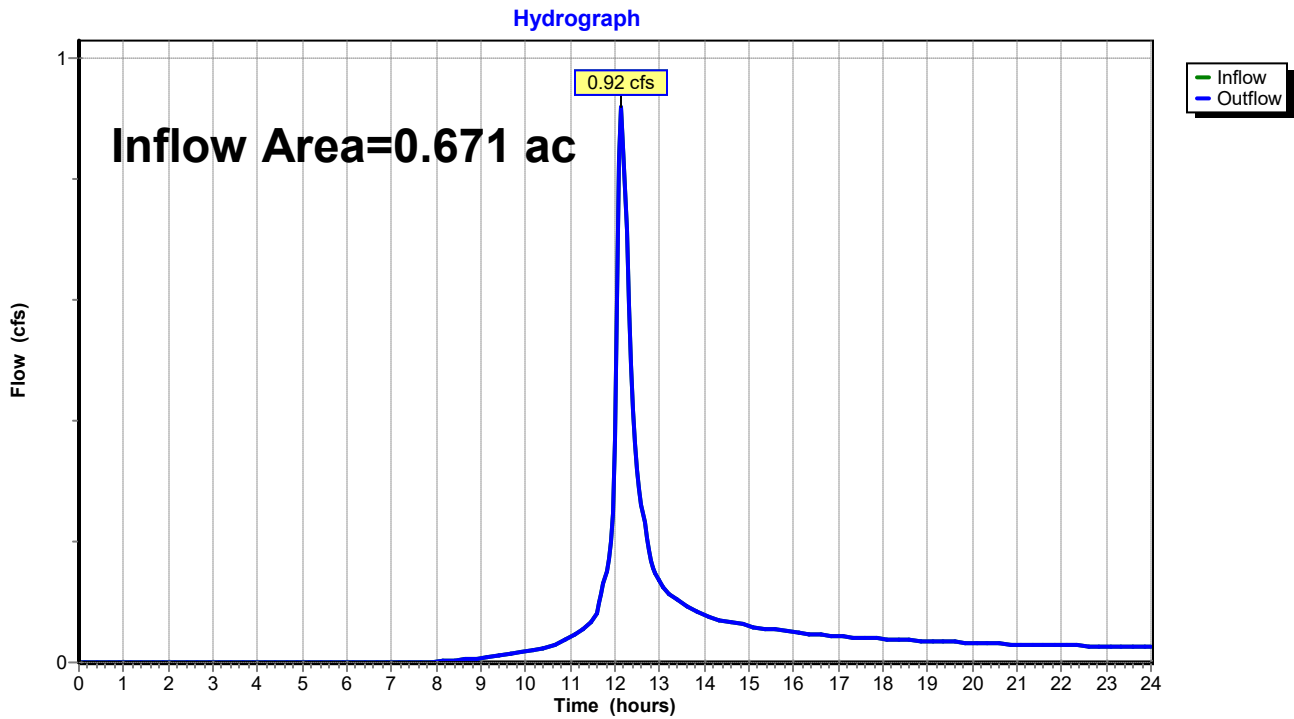


Summary for Reach DP-2: Ex. CBs in Driveway

Inflow Area = 0.671 ac, 15.75% Impervious, Inflow Depth > 1.55" for 2-yr event
Inflow = 0.92 cfs @ 12.15 hrs, Volume= 0.087 af
Outflow = 0.92 cfs @ 12.15 hrs, Volume= 0.087 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach DP-2: Ex. CBs in Driveway



C-DAT-2002032-EXISTING HYDROLOGY CT-Coventry-2002032 24-hr S1 10-yr Rainfall=5.08"

Prepared by {enter your company name here}

Printed 5/4/2021

HydroCAD® 10.00-25 s/n 01334 © 2019 HydroCAD Software Solutions LLC

Page 8

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-10: Area to Wetland to Runoff Area=185,210 sf 6.55% Impervious Runoff Depth>2.24"
Flow Length=136' Slope=0.0145 '/' Tc=25.0 min CN=72 Runoff=6.27 cfs 0.794 af

Subcatchment EDA-20: Area to Ex. CBs in Runoff Area=29,230 sf 15.75% Impervious Runoff Depth>3.04"
Flow Length=169' Tc=14.3 min CN=81 Runoff=1.82 cfs 0.170 af

Reach DP-1: Wetland to Northeast

Inflow=6.27 cfs 0.794 af
Outflow=6.27 cfs 0.794 af

Reach DP-2: Ex. CBs in Driveway

Inflow=1.82 cfs 0.170 af
Outflow=1.82 cfs 0.170 af

Total Runoff Area = 4.923 ac Runoff Volume = 0.964 af Average Runoff Depth = 2.35"
92.19% Pervious = 4.539 ac 7.81% Impervious = 0.384 ac

Summary for Subcatchment EDA-10: Area to Wetland to the Northeast

Runoff = 6.27 cfs @ 12.30 hrs, Volume= 0.794 af, Depth> 2.24"

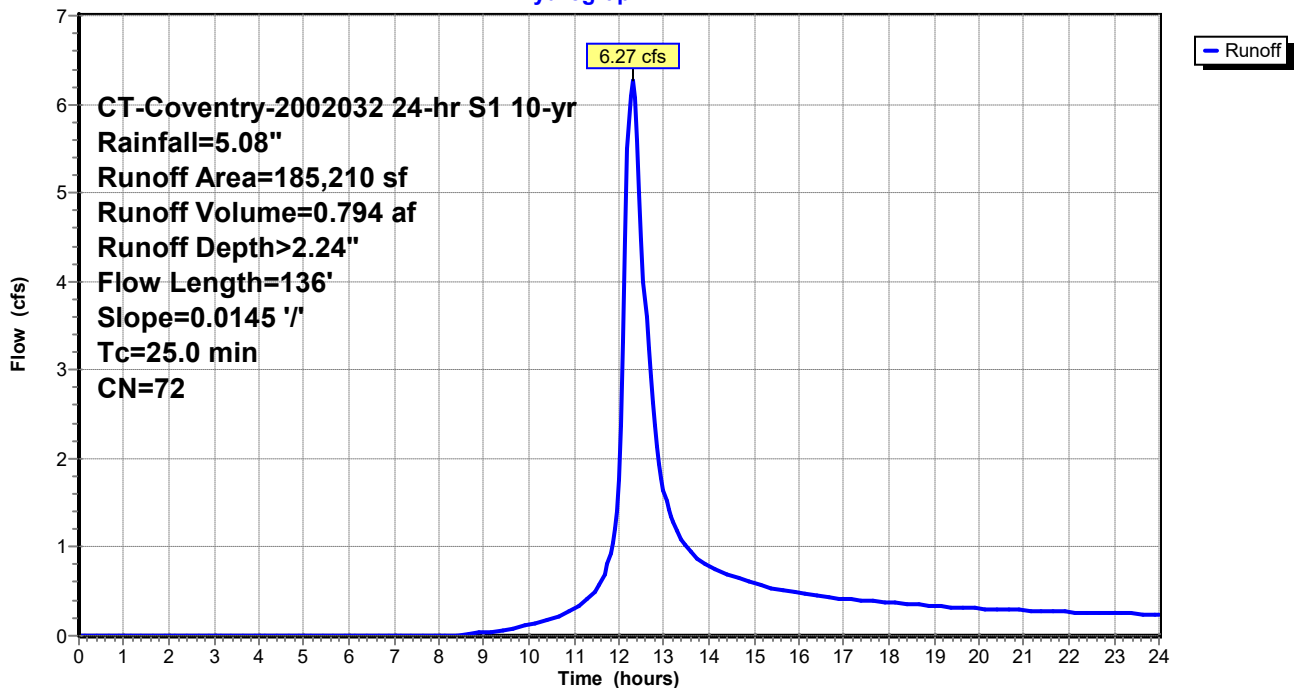
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 CT-Coventry-2002032 24-hr S1 10-yr Rainfall=5.08"

Area (sf)	CN	Description
134,225	69	50-75% Grass cover, Fair, HSG B
15,340	79	50-75% Grass cover, Fair, HSG C
2,445	60	Woods, Fair, HSG B
21,065	73	Woods, Fair, HSG C
12,135	98	Paved parking, HSG B
0	98	Paved parking, HSG C
185,210	72	Weighted Average
173,075		93.45% Pervious Area
12,135		6.55% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.0	100	0.0145	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.31"
1.0	36	0.0145	0.60		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
25.0	136	Total			

Subcatchment EDA-10: Area to Wetland to the Northeast

Hydrograph



Summary for Subcatchment EDA-20: Area to Ex. CBs in Driveway

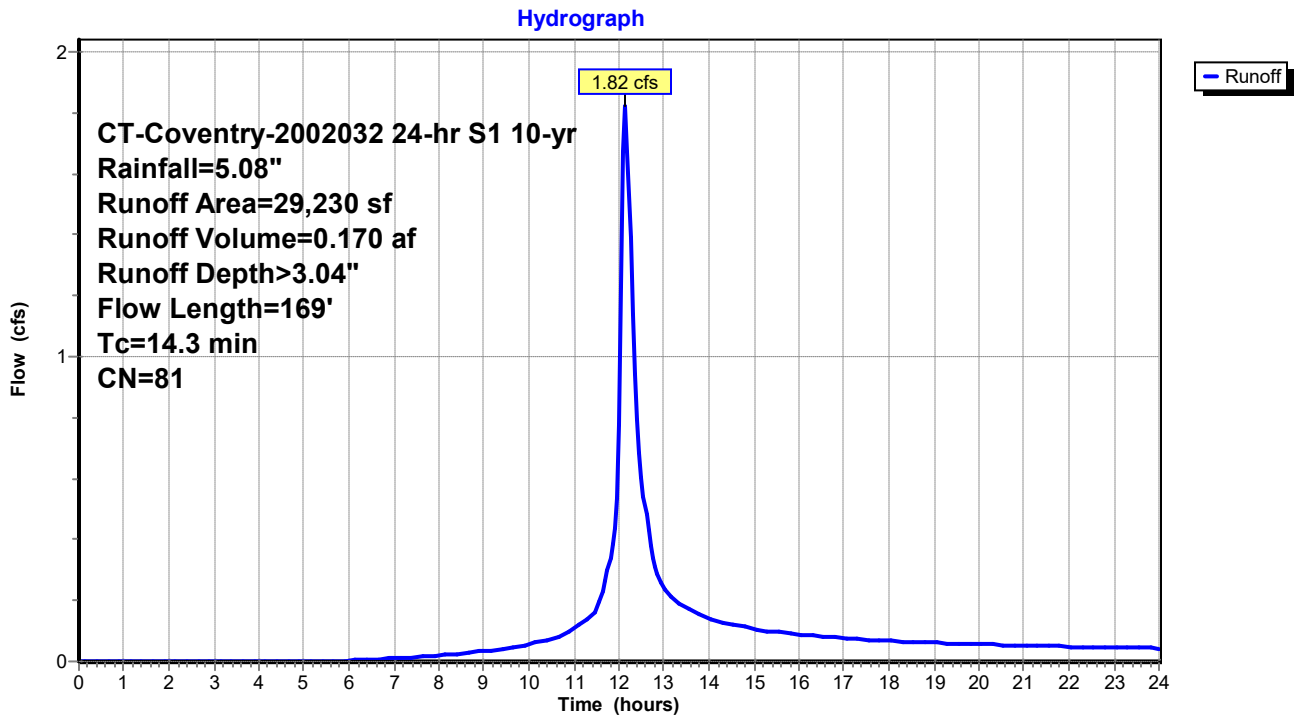
Runoff = 1.82 cfs @ 12.15 hrs, Volume= 0.170 af, Depth> 3.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 CT-Coventry-2002032 24-hr S1 10-yr Rainfall=5.08"

Area (sf)	CN	Description
2,335	69	50-75% Grass cover, Fair, HSG B
22,290	79	50-75% Grass cover, Fair, HSG C
0	60	Woods, Fair, HSG B
0	73	Woods, Fair, HSG C
185	98	Paved parking, HSG B
4,420	98	Paved parking, HSG C
29,230	81	Weighted Average
24,625		84.25% Pervious Area
4,605		15.75% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.9	100	0.0080	0.12		Sheet Flow, Grass: Short n= 0.150 P2= 3.31"
0.2	16	0.0284	1.18		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.1	15	0.0100	2.03		Shallow Concentrated Flow, Paved Kv= 20.3 fps
0.1	38	0.0185	7.16	8.79	Pipe Channel, 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.013 Corrugated PE, smooth interior
14.3	169	Total			

Subcatchment EDA-20: Area to Ex. CBs in Driveway

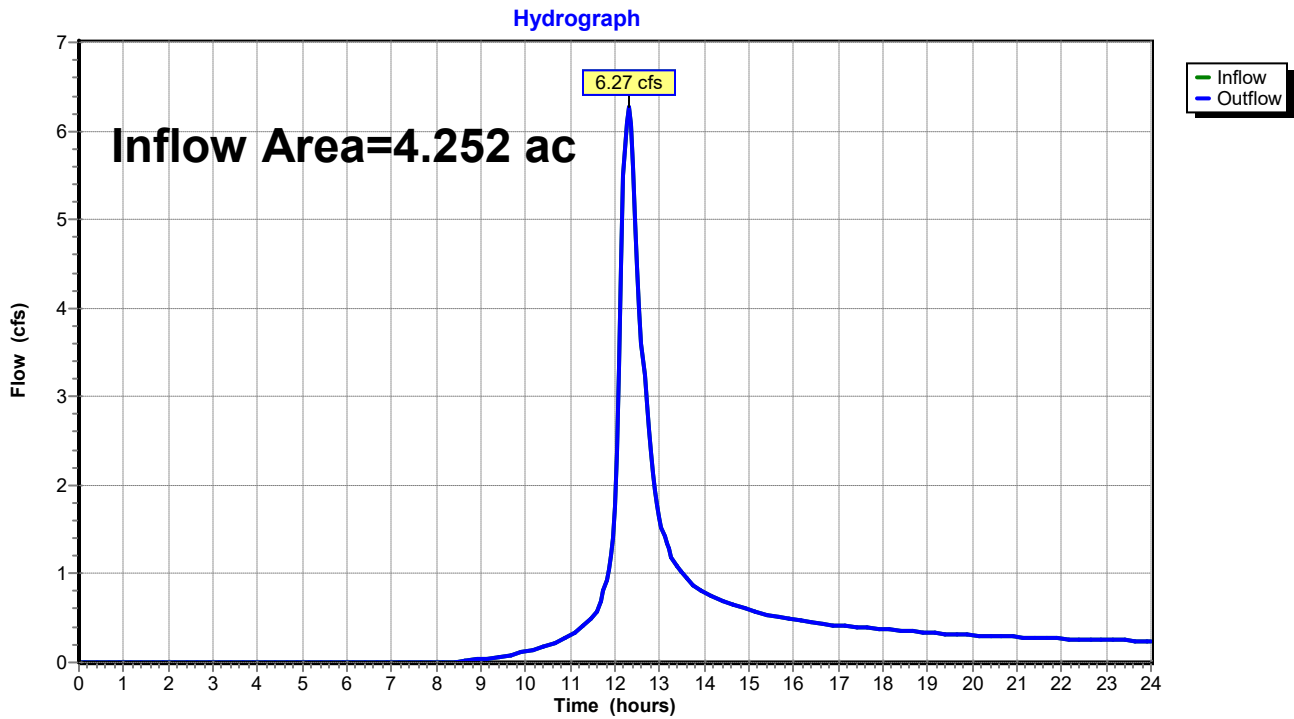


Summary for Reach DP-1: Wetland to Northeast

Inflow Area = 4.252 ac, 6.55% Impervious, Inflow Depth > 2.24" for 10-yr event
Inflow = 6.27 cfs @ 12.30 hrs, Volume= 0.794 af
Outflow = 6.27 cfs @ 12.30 hrs, Volume= 0.794 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach DP-1: Wetland to Northeast

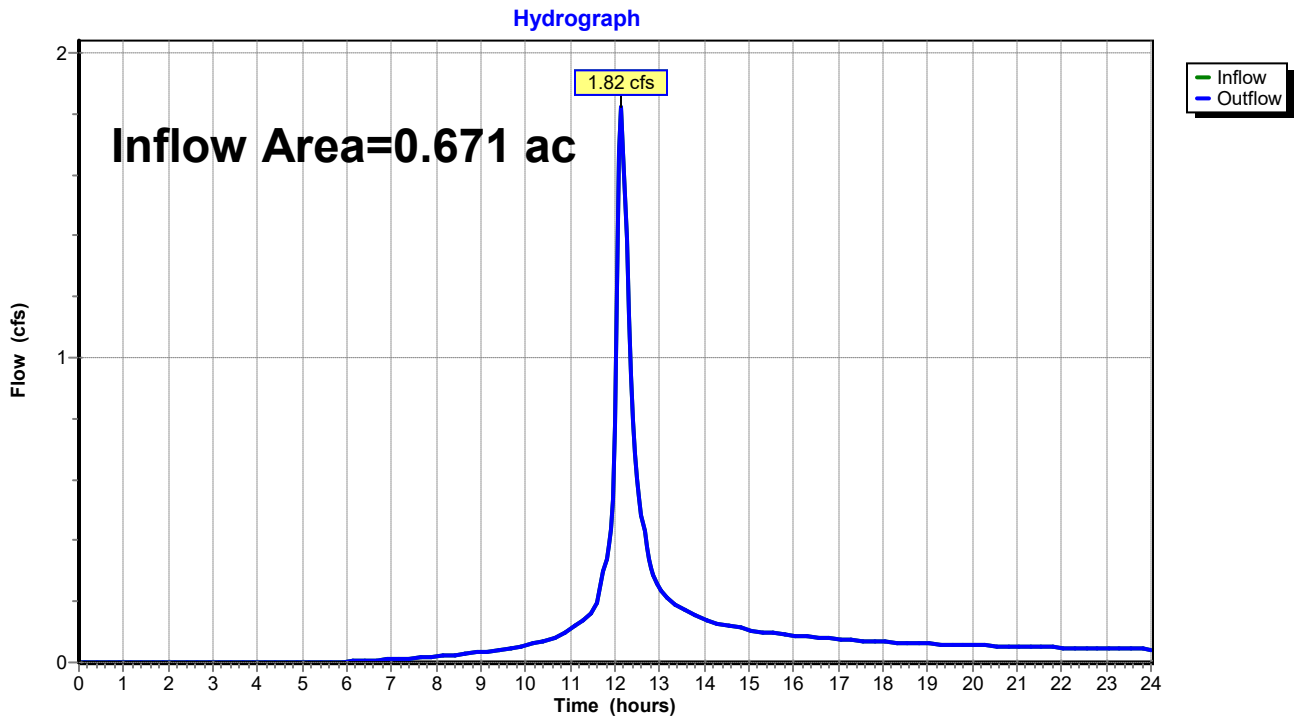


Summary for Reach DP-2: Ex. CBs in Driveway

Inflow Area = 0.671 ac, 15.75% Impervious, Inflow Depth > 3.04" for 10-yr event
Inflow = 1.82 cfs @ 12.15 hrs, Volume= 0.170 af
Outflow = 1.82 cfs @ 12.15 hrs, Volume= 0.170 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach DP-2: Ex. CBs in Driveway



C-DAT-2002032-EXISTING HYDROLOG CT-Coventry-2002032 24-hr S1 100-yr Rainfall=7.90"

Prepared by {enter your company name here}

Printed 5/4/2021

HydroCAD® 10.00-25 s/n 01334 © 2019 HydroCAD Software Solutions LLC

Page 14

Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment EDA-10: Area to Wetland to Runoff Area=185,210 sf 6.55% Impervious Runoff Depth>4.57"
Flow Length=136' Slope=0.0145 '/' Tc=25.0 min CN=72 Runoff=12.93 cfs 1.620 af

Subcatchment EDA-20: Area to Ex. CBs in Runoff Area=29,230 sf 15.75% Impervious Runoff Depth>5.63"
Flow Length=169' Tc=14.3 min CN=81 Runoff=3.30 cfs 0.315 af

Reach DP-1: Wetland to Northeast

Inflow=12.93 cfs 1.620 af
Outflow=12.93 cfs 1.620 af

Reach DP-2: Ex. CBs in Driveway

Inflow=3.30 cfs 0.315 af
Outflow=3.30 cfs 0.315 af

Total Runoff Area = 4.923 ac Runoff Volume = 1.934 af Average Runoff Depth = 4.72"
92.19% Pervious = 4.539 ac 7.81% Impervious = 0.384 ac

Summary for Subcatchment EDA-10: Area to Wetland to the Northeast

Runoff = 12.93 cfs @ 12.29 hrs, Volume= 1.620 af, Depth> 4.57"

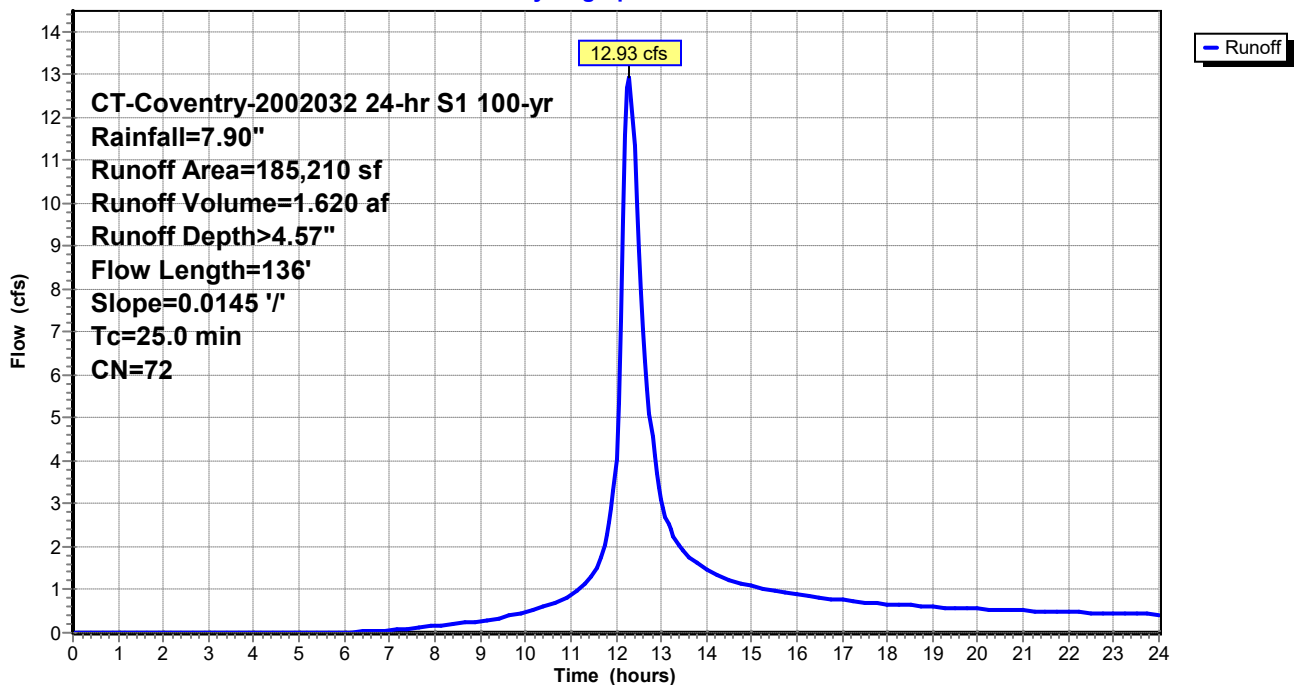
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 CT-Coventry-2002032 24-hr S1 100-yr Rainfall=7.90"

Area (sf)	CN	Description
134,225	69	50-75% Grass cover, Fair, HSG B
15,340	79	50-75% Grass cover, Fair, HSG C
2,445	60	Woods, Fair, HSG B
21,065	73	Woods, Fair, HSG C
12,135	98	Paved parking, HSG B
0	98	Paved parking, HSG C
185,210	72	Weighted Average
173,075		93.45% Pervious Area
12,135		6.55% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.0	100	0.0145	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.31"
1.0	36	0.0145	0.60		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
25.0	136	Total			

Subcatchment EDA-10: Area to Wetland to the Northeast

Hydrograph



Summary for Subcatchment EDA-20: Area to Ex. CBs in Driveway

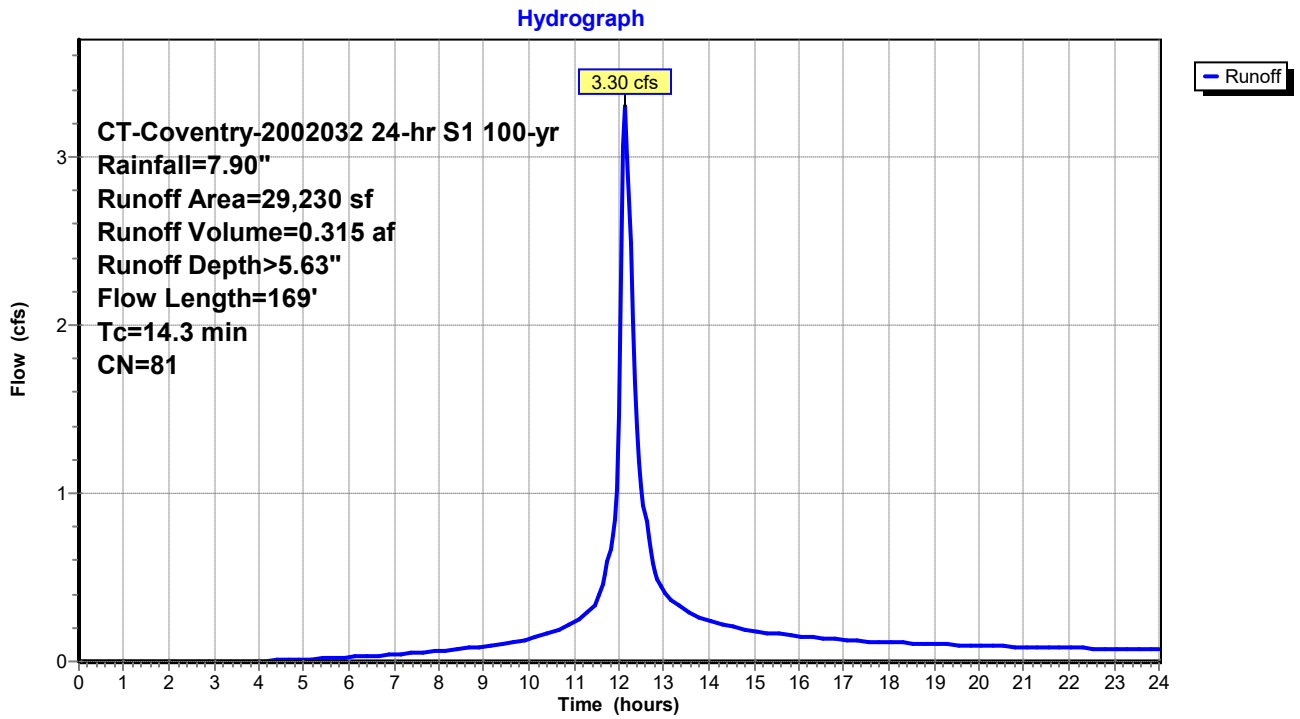
Runoff = 3.30 cfs @ 12.15 hrs, Volume= 0.315 af, Depth> 5.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs
 CT-Coventry-2002032 24-hr S1 100-yr Rainfall=7.90"

Area (sf)	CN	Description
2,335	69	50-75% Grass cover, Fair, HSG B
22,290	79	50-75% Grass cover, Fair, HSG C
0	60	Woods, Fair, HSG B
0	73	Woods, Fair, HSG C
185	98	Paved parking, HSG B
4,420	98	Paved parking, HSG C
29,230	81	Weighted Average
24,625		84.25% Pervious Area
4,605		15.75% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.9	100	0.0080	0.12		Sheet Flow, Grass: Short n= 0.150 P2= 3.31"
0.2	16	0.0284	1.18		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.1	15	0.0100	2.03		Shallow Concentrated Flow, Paved Kv= 20.3 fps
0.1	38	0.0185	7.16	8.79	Pipe Channel, 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.013 Corrugated PE, smooth interior
14.3	169	Total			

Subcatchment EDA-20: Area to Ex. CBs in Driveway

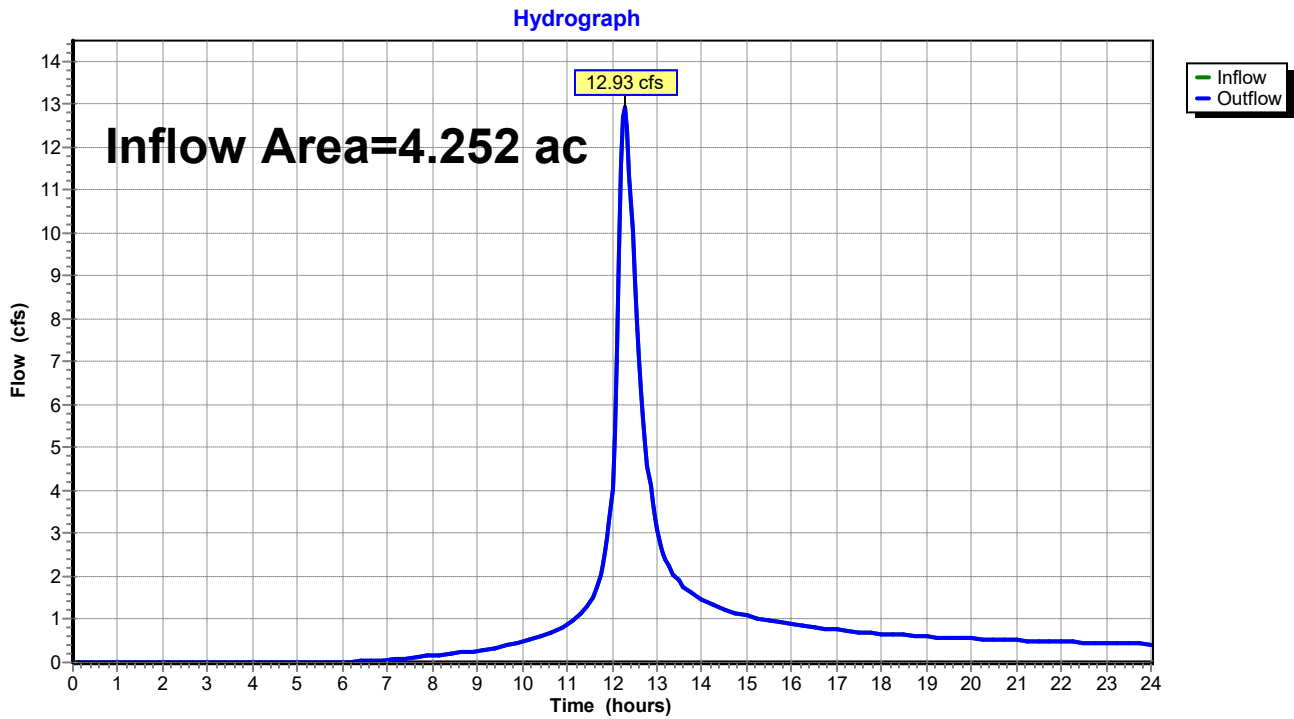


Summary for Reach DP-1: Wetland to Northeast

Inflow Area = 4.252 ac, 6.55% Impervious, Inflow Depth > 4.57" for 100-yr event
 Inflow = 12.93 cfs @ 12.29 hrs, Volume= 1.620 af
 Outflow = 12.93 cfs @ 12.29 hrs, Volume= 1.620 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach DP-1: Wetland to Northeast

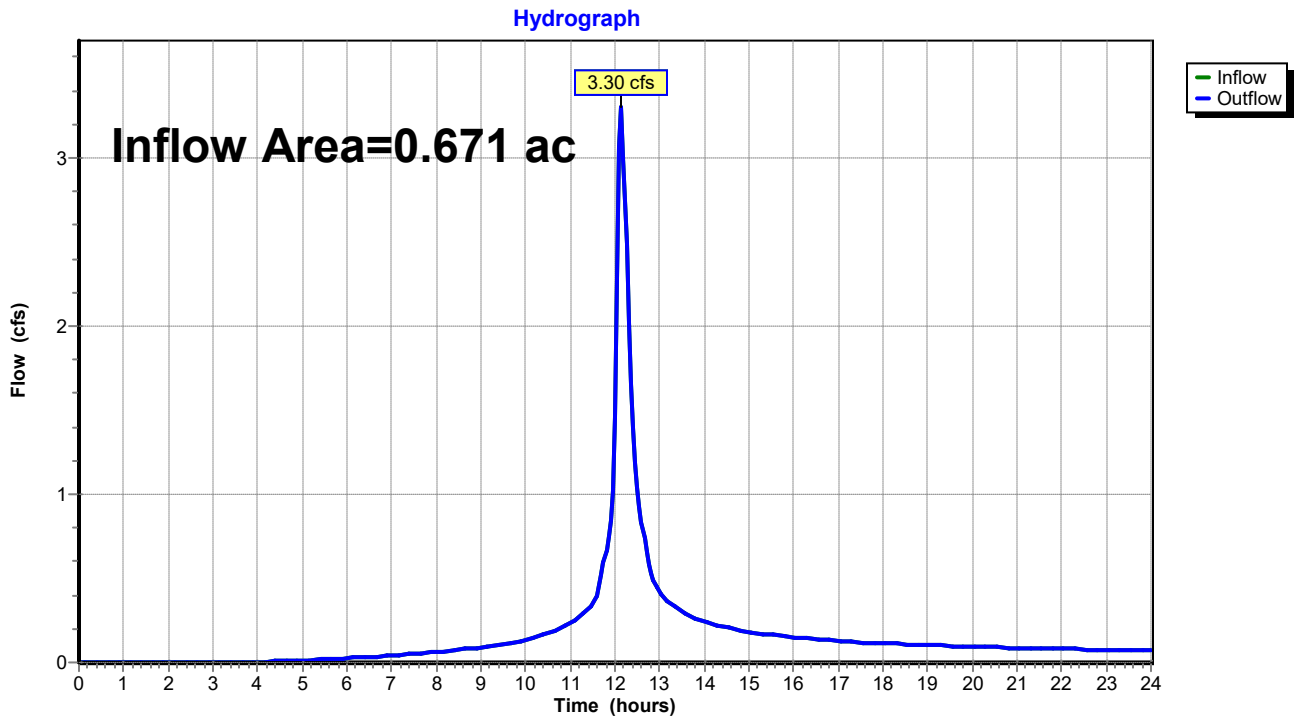


Summary for Reach DP-2: Ex. CBs in Driveway

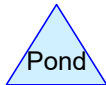
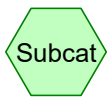
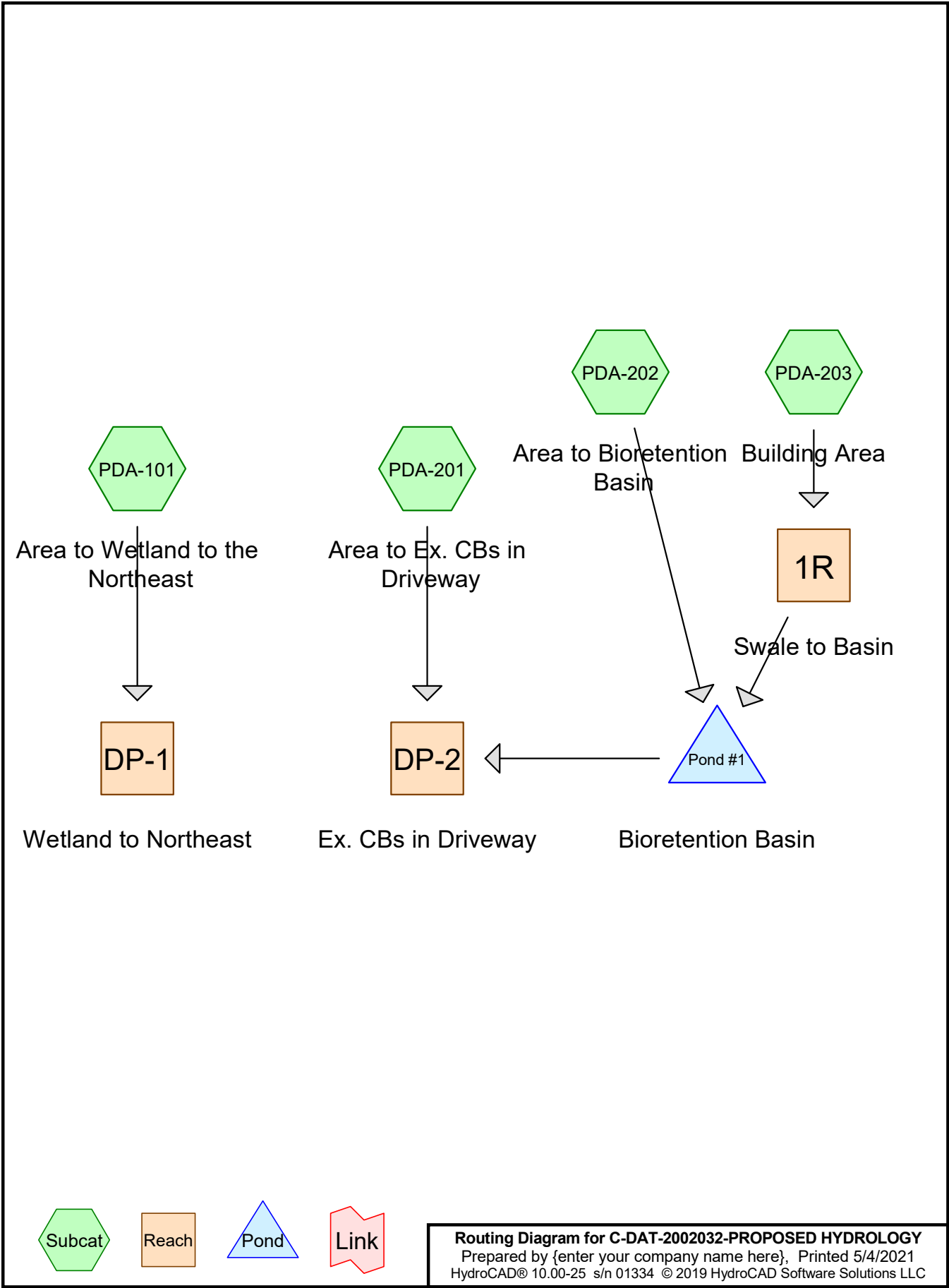
Inflow Area = 0.671 ac, 15.75% Impervious, Inflow Depth > 5.63" for 100-yr event
Inflow = 3.30 cfs @ 12.15 hrs, Volume= 0.315 af
Outflow = 3.30 cfs @ 12.15 hrs, Volume= 0.315 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

Reach DP-2: Ex. CBs in Driveway



APPENDIX C
POST-DEVELOPMENT HYDROLOGY



Routing Diagram for C-DAT-2002032-PROPOSED HYDROLOGY
 Prepared by {enter your company name here}, Printed 5/4/2021
 HydroCAD® 10.00-25 s/n 01334 © 2019 HydroCAD Software Solutions LLC

C-DAT-2002032-PROPOSED HYDROLOG CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"

Prepared by {enter your company name here}

Printed 5/4/2021

HydroCAD® 10.00-25 s/n 01334 © 2019 HydroCAD Software Solutions LLC

Page 2

Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PDA-101: Area to Wetland Runoff Area=127,015 sf 12.04% Impervious Runoff Depth=1.11"
Flow Length=136' Slope=0.0145 '/' Tc=25.0 min CN=74 Runoff=2.03 cfs 0.270 af

Subcatchment PDA-201: Area to Ex. CBs Runoff Area=22,250 sf 26.83% Impervious Runoff Depth=1.70"
Flow Length=161' Tc=9.8 min CN=83 Runoff=0.94 cfs 0.072 af

Subcatchment PDA-202: Area to Runoff Area=54,405 sf 50.91% Impervious Runoff Depth=1.85"
Flow Length=250' Slope=0.0100 '/' Tc=16.3 min CN=85 Runoff=1.94 cfs 0.193 af

Subcatchment PDA-203: Building Area Runoff Area=10,770 sf 100.00% Impervious Runoff Depth=3.08"
Tc=5.0 min CN=98 Runoff=0.96 cfs 0.063 af

Reach 1R: Swale to Basin Avg. Flow Depth=0.31' Max Vel=1.73 fps Inflow=0.96 cfs 0.063 af
n=0.030 L=370.0' S=0.0105 '/' Capacity=9.44 cfs Outflow=0.81 cfs 0.063 af

Reach DP-1: Wetland to Northeast Inflow=2.03 cfs 0.270 af
Outflow=2.03 cfs 0.270 af

Reach DP-2: Ex. CBs in Driveway Inflow=0.94 cfs 0.072 af
Outflow=0.94 cfs 0.072 af

Pond Pond #1: Bioretention Basin Peak Elev=659.88' Storage=5,907 cf Inflow=2.64 cfs 0.256 af
Discarded=0.10 cfs 0.256 af Primary=0.00 cfs 0.000 af Outflow=0.10 cfs 0.256 af

Total Runoff Area = 4.923 ac Runoff Volume = 0.598 af Average Runoff Depth = 1.46"
72.15% Pervious = 3.552 ac 27.85% Impervious = 1.371 ac

Summary for Subcatchment PDA-101: Area to Wetland to the Northeast

Runoff = 2.03 cfs @ 12.31 hrs, Volume= 0.270 af, Depth= 1.11"

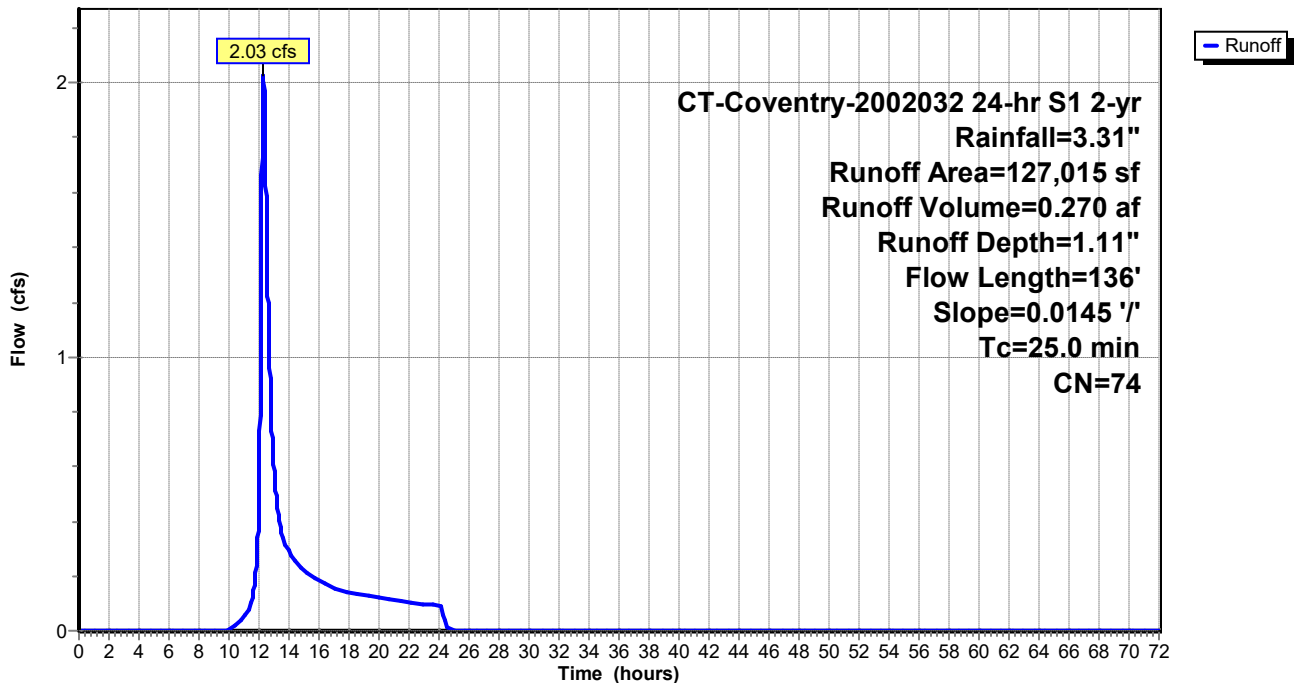
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"

Area (sf)	CN	Description
78,455	69	50-75% Grass cover, Fair, HSG B
10,060	79	50-75% Grass cover, Fair, HSG C
2,445	60	Woods, Fair, HSG B
20,760	73	Woods, Fair, HSG C
13,830	98	Paved parking, HSG B
1,465	98	Paved parking, HSG C
127,015	74	Weighted Average
111,720		87.96% Pervious Area
15,295		12.04% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.0	100	0.0145	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.31"
1.0	36	0.0145	0.60		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
25.0	136	Total			

Subcatchment PDA-101: Area to Wetland to the Northeast

Hydrograph



Summary for Subcatchment PDA-201: Area to Ex. CBs in Driveway

Runoff = 0.94 cfs @ 12.08 hrs, Volume= 0.072 af, Depth= 1.70"

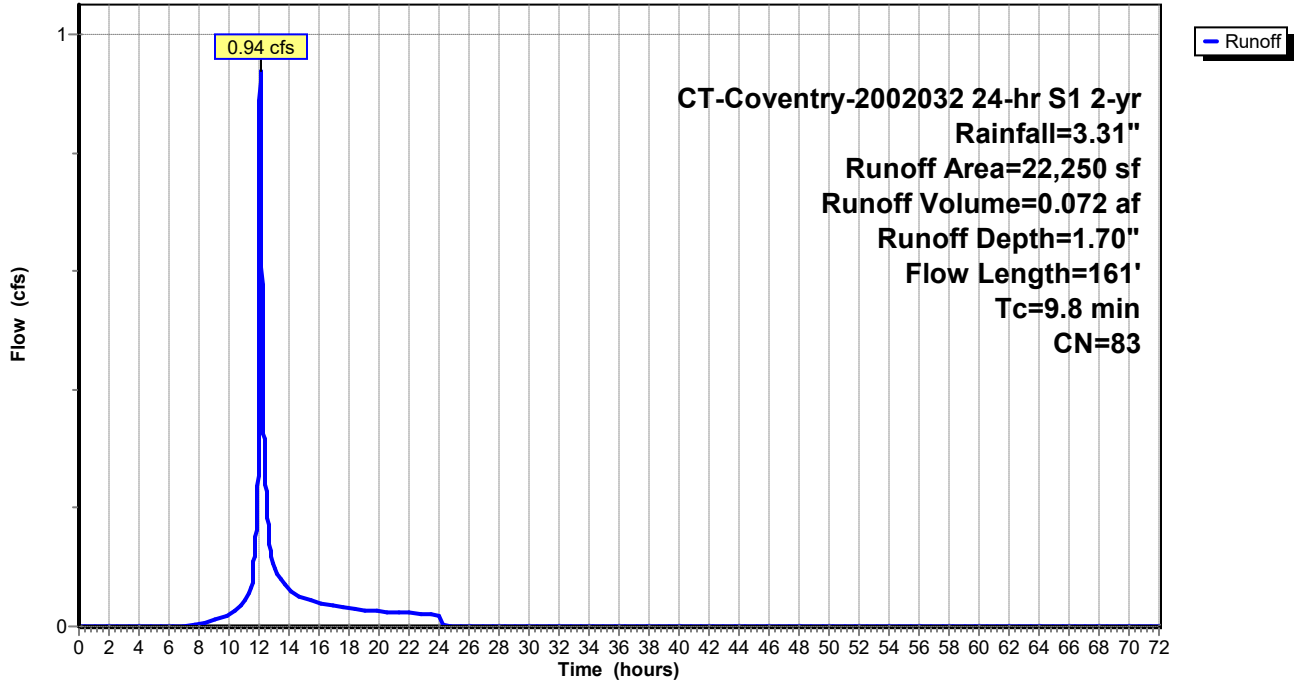
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"

Area (sf)	CN	Description
2,580	69	50-75% Grass cover, Fair, HSG B
13,700	79	50-75% Grass cover, Fair, HSG C
0	60	Woods, Fair, HSG B
0	73	Woods, Fair, HSG C
910	98	Paved parking, HSG B
5,060	98	Paved parking, HSG C
22,250	83	Weighted Average
16,280		73.17% Pervious Area
5,970		26.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.3	100	0.0220	0.18		Sheet Flow, Grass: Short n= 0.150 P2= 3.31"
0.4	23	0.0174	0.92		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.1	38	0.0185	7.16	8.79	Pipe Channel, 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.013 Corrugated PE, smooth interior
9.8	161	Total			

Subcatchment PDA-201: Area to Ex. CBs in Driveway

Hydrograph



Summary for Subcatchment PDA-202: Area to Bioretention Basin

Runoff = 1.94 cfs @ 12.17 hrs, Volume= 0.193 af, Depth= 1.85"

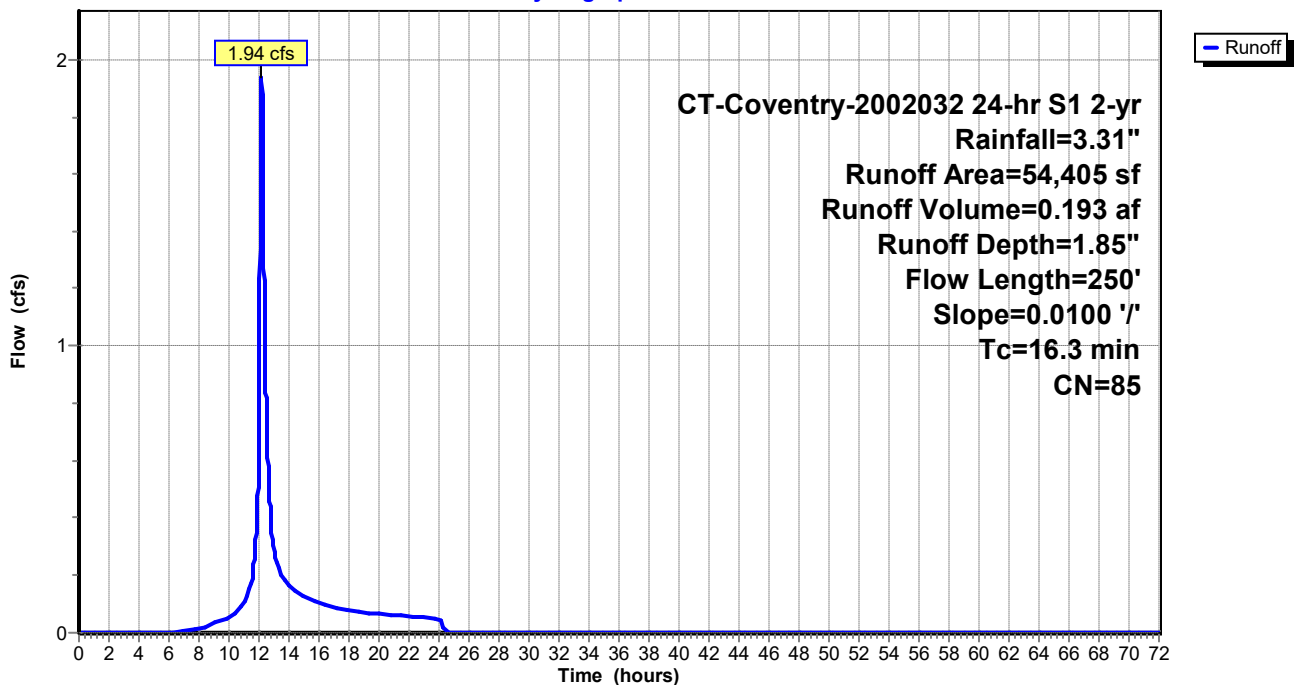
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"

Area (sf)	CN	Description
20,135	69	50-75% Grass cover, Fair, HSG B
6,575	79	50-75% Grass cover, Fair, HSG C
0	60	Woods, Fair, HSG B
0	73	Woods, Fair, HSG C
22,195	98	Paved parking, HSG B
5,500	98	Paved parking, HSG C
54,405	85	Weighted Average
26,710		49.09% Pervious Area
27,695		50.91% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7	100	0.0100	0.13		Sheet Flow, Grass: Short n= 0.150 P2= 3.31"
3.6	150	0.0100	0.70		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
16.3	250	Total			

Subcatchment PDA-202: Area to Bioretention Basin

Hydrograph



Summary for Subcatchment PDA-203: Building Area

Runoff = 0.96 cfs @ 12.03 hrs, Volume= 0.063 af, Depth= 3.08"

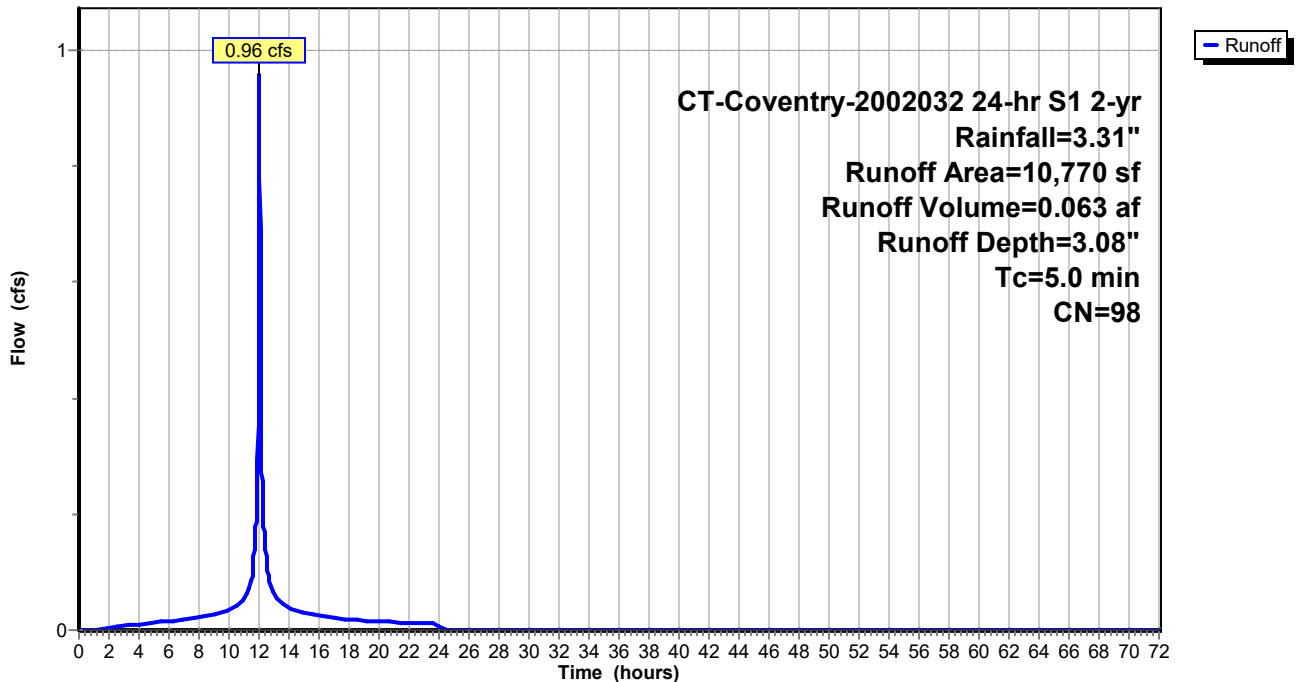
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 CT-Coventry-2002032 24-hr S1 2-yr Rainfall=3.31"

Area (sf)	CN	Description
0	69	50-75% Grass cover, Fair, HSG B
0	79	50-75% Grass cover, Fair, HSG C
0	60	Woods, Fair, HSG B
0	73	Woods, Fair, HSG C
10,770	98	Paved parking, HSG B
0	98	Paved parking, HSG C
10,770	98	Weighted Average
10,770		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Subcatchment PDA-203: Building Area

Hydrograph



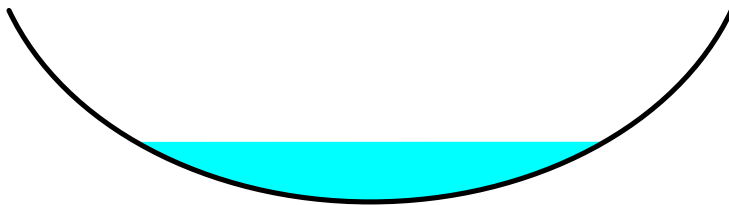
Summary for Reach 1R: Swale to Basin

Inflow Area = 0.247 ac, 100.00% Impervious, Inflow Depth = 3.08" for 2-yr event
 Inflow = 0.96 cfs @ 12.03 hrs, Volume= 0.063 af
 Outflow = 0.81 cfs @ 12.12 hrs, Volume= 0.063 af, Atten= 15%, Lag= 5.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 Max. Velocity= 1.73 fps, Min. Travel Time= 3.6 min
 Avg. Velocity = 0.54 fps, Avg. Travel Time= 11.3 min

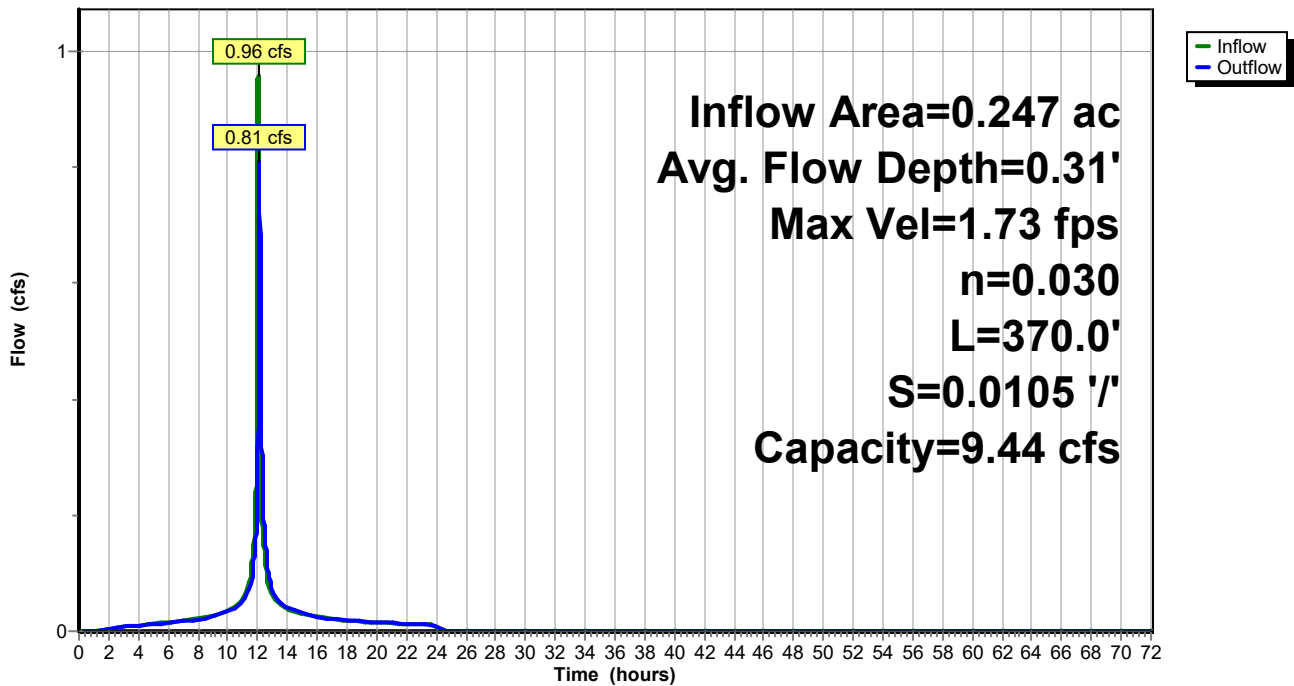
Peak Storage= 173 cf @ 12.06 hrs
 Average Depth at Peak Storage= 0.31'
 Bank-Full Depth= 1.00' Flow Area= 2.7 sf, Capacity= 9.44 cfs

4.00' x 1.00' deep Parabolic Channel, n= 0.030 Earth, grassed & winding
 Length= 370.0' Slope= 0.0105 '/'
 Inlet Invert= 665.90', Outlet Invert= 662.00'



Reach 1R: Swale to Basin

Hydrograph



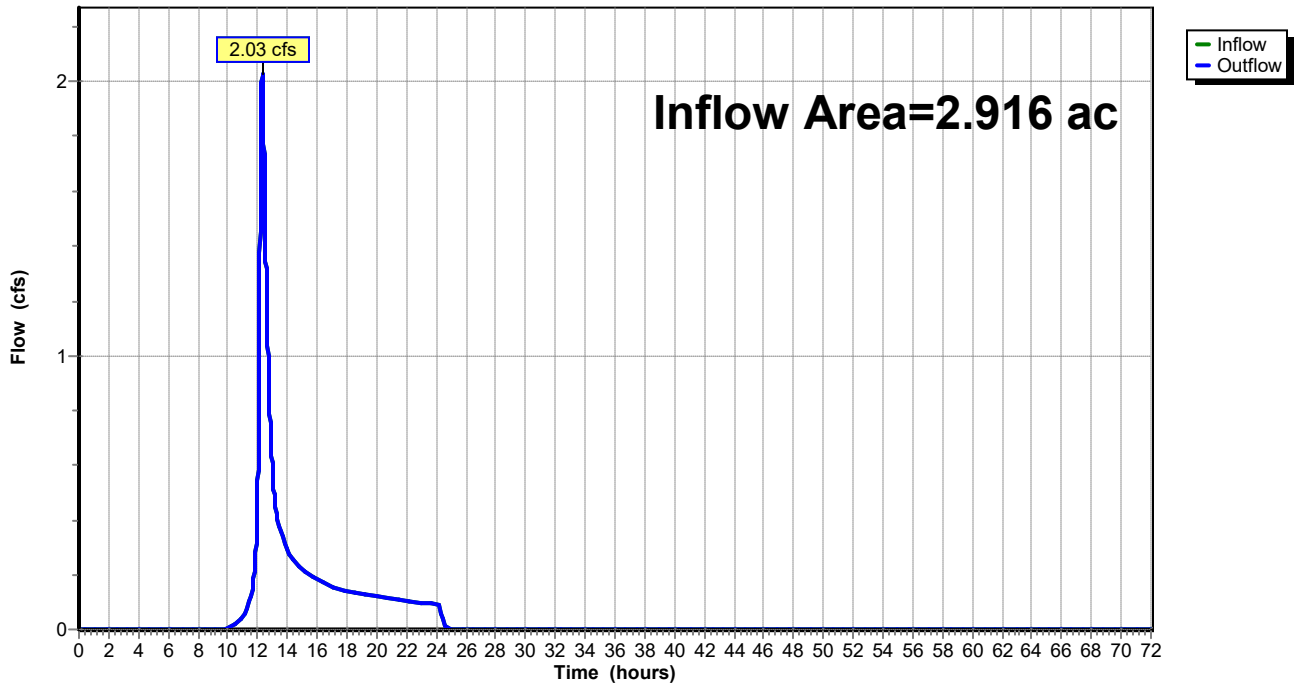
Summary for Reach DP-1: Wetland to Northeast

Inflow Area = 2.916 ac, 12.04% Impervious, Inflow Depth = 1.11" for 2-yr event
Inflow = 2.03 cfs @ 12.31 hrs, Volume= 0.270 af
Outflow = 2.03 cfs @ 12.31 hrs, Volume= 0.270 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Reach DP-1: Wetland to Northeast

Hydrograph



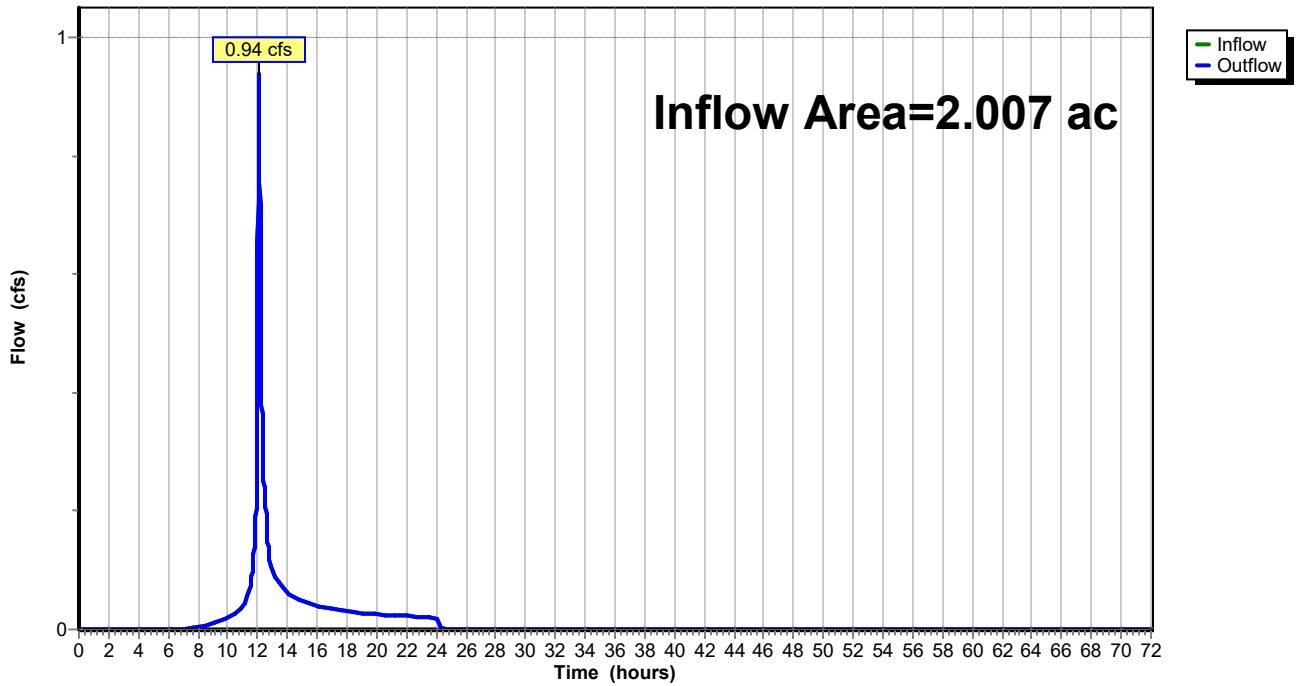
Summary for Reach DP-2: Ex. CBs in Driveway

Inflow Area = 2.007 ac, 50.83% Impervious, Inflow Depth = 0.43" for 2-yr event
Inflow = 0.94 cfs @ 12.08 hrs, Volume= 0.072 af
Outflow = 0.94 cfs @ 12.08 hrs, Volume= 0.072 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Reach DP-2: Ex. CBs in Driveway

Hydrograph



Summary for Pond Pond #1: Bioretention Basin

Inflow Area = 1.496 ac, 59.02% Impervious, Inflow Depth = 2.05" for 2-yr event
 Inflow = 2.64 cfs @ 12.14 hrs, Volume= 0.256 af
 Outflow = 0.10 cfs @ 10.54 hrs, Volume= 0.256 af, Atten= 96%, Lag= 0.0 min
 Discarded = 0.10 cfs @ 10.54 hrs, Volume= 0.256 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 659.88' @ 17.72 hrs Surf.Area= 4,366 sf Storage= 5,907 cf

Plug-Flow detention time= 578.0 min calculated for 0.256 af (100% of inflow)
 Center-of-Mass det. time= 578.0 min (1,412.2 - 834.2)

Volume	Invert	Avail.Storage	Storage Description
#1	656.50'	6,986 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 17,464 cf Overall x 40.0% Voids
#2	660.50'	15,465 cf	Custom Stage Data (Prismatic) Listed below (Recalc) -Impervious
		22,450 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
656.50	4,366	0	0
660.50	4,366	17,464	17,464

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
660.50	4,366	0	0
661.00	4,888	2,314	2,314
662.00	6,042	5,465	7,779
663.00	7,775	6,909	14,687
663.10	7,775	778	15,465

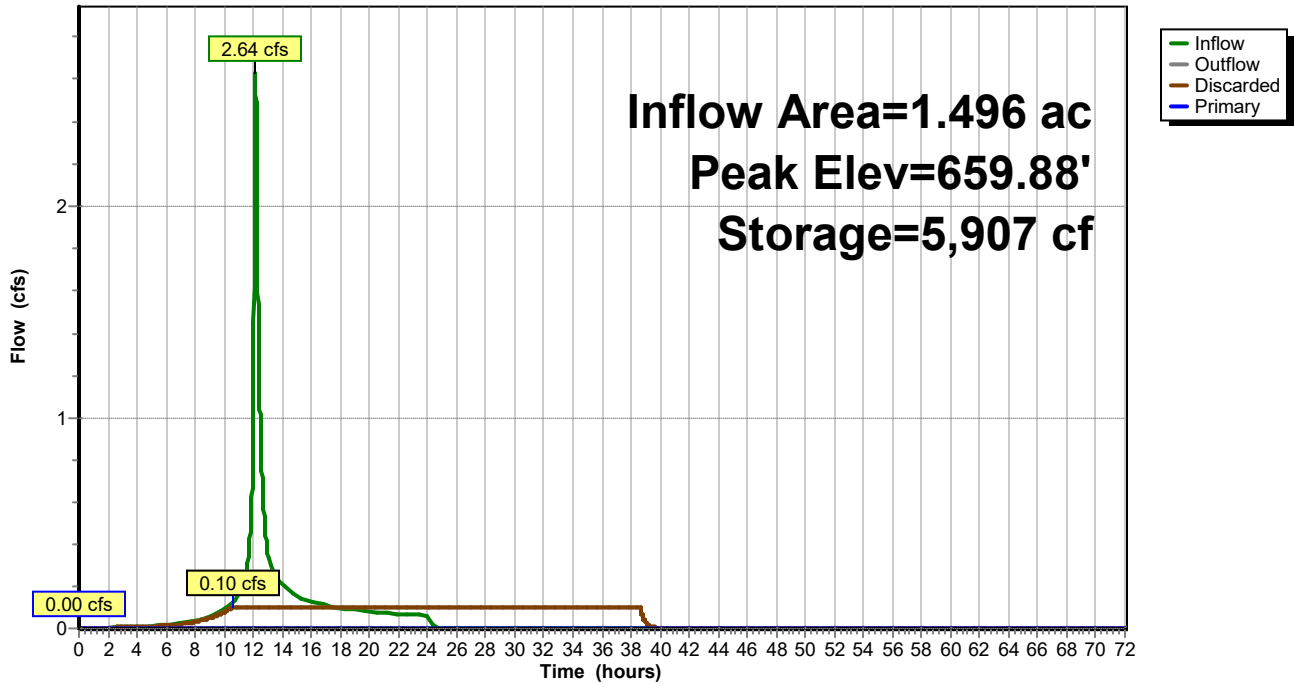
Device	Routing	Invert	Outlet Devices
#1	Discarded	656.50'	1.000 in/hr Exfiltration over Surface area
#2	Primary	658.25'	12.0" Round Culvert L= 69.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 658.25' / 657.90' S= 0.0051 '/ Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	661.87'	24.0" x 36.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.10 cfs @ 10.54 hrs HW=656.57' (Free Discharge)
 ↑1=Exfiltration (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=656.50' (Free Discharge)
 ↑2=Culvert (Controls 0.00 cfs)
 ↑3=Orifice/Grate (Controls 0.00 cfs)

Pond Pond #1: Bioretention Basin

Hydrograph



Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PDA-101: Area to Wetland Runoff Area=127,015 sf 12.04% Impervious Runoff Depth=2.43"
Flow Length=136' Slope=0.0145 '/' Tc=25.0 min CN=74 Runoff=4.67 cfs 0.590 af

Subcatchment PDA-201: Area to Ex. CBs Runoff Area=22,250 sf 26.83% Impervious Runoff Depth=3.25"
Flow Length=161' Tc=9.8 min CN=83 Runoff=1.79 cfs 0.138 af

Subcatchment PDA-202: Area to Runoff Area=54,405 sf 50.91% Impervious Runoff Depth=3.44"
Flow Length=250' Slope=0.0100 '/' Tc=16.3 min CN=85 Runoff=3.57 cfs 0.358 af

Subcatchment PDA-203: Building Area Runoff Area=10,770 sf 100.00% Impervious Runoff Depth=4.84"
Tc=5.0 min CN=98 Runoff=1.47 cfs 0.100 af

Reach 1R: Swale to Basin Avg. Flow Depth=0.39' Max Vel=1.98 fps Inflow=1.47 cfs 0.100 af
n=0.030 L=370.0' S=0.0105 '/' Capacity=9.44 cfs Outflow=1.28 cfs 0.100 af

Reach DP-1: Wetland to Northeast Inflow=4.67 cfs 0.590 af
Outflow=4.67 cfs 0.590 af

Reach DP-2: Ex. CBs in Driveway Inflow=1.79 cfs 0.138 af
Outflow=1.79 cfs 0.138 af

Pond Pond #1: Bioretention Basin Peak Elev=661.76' Storage=13,330 cf Inflow=4.60 cfs 0.458 af
Discarded=0.10 cfs 0.458 af Primary=0.00 cfs 0.000 af Outflow=0.10 cfs 0.458 af

Total Runoff Area = 4.923 ac Runoff Volume = 1.186 af Average Runoff Depth = 2.89"
72.15% Pervious = 3.552 ac 27.85% Impervious = 1.371 ac

Summary for Subcatchment PDA-101: Area to Wetland to the Northeast

Runoff = 4.67 cfs @ 12.30 hrs, Volume= 0.590 af, Depth= 2.43"

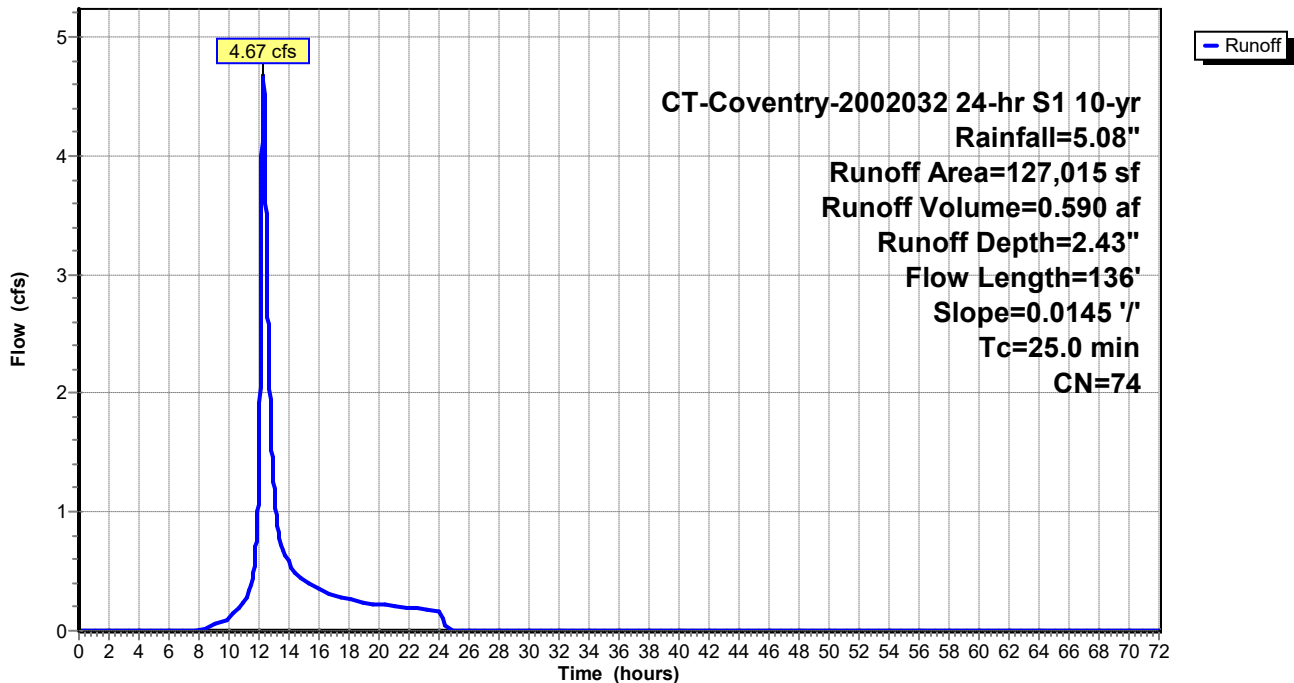
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 CT-Coventry-2002032 24-hr S1 10-yr Rainfall=5.08"

Area (sf)	CN	Description
78,455	69	50-75% Grass cover, Fair, HSG B
10,060	79	50-75% Grass cover, Fair, HSG C
2,445	60	Woods, Fair, HSG B
20,760	73	Woods, Fair, HSG C
13,830	98	Paved parking, HSG B
1,465	98	Paved parking, HSG C
127,015	74	Weighted Average
111,720		87.96% Pervious Area
15,295		12.04% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.0	100	0.0145	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.31"
1.0	36	0.0145	0.60		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
25.0	136	Total			

Subcatchment PDA-101: Area to Wetland to the Northeast

Hydrograph



Summary for Subcatchment PDA-201: Area to Ex. CBs in Driveway

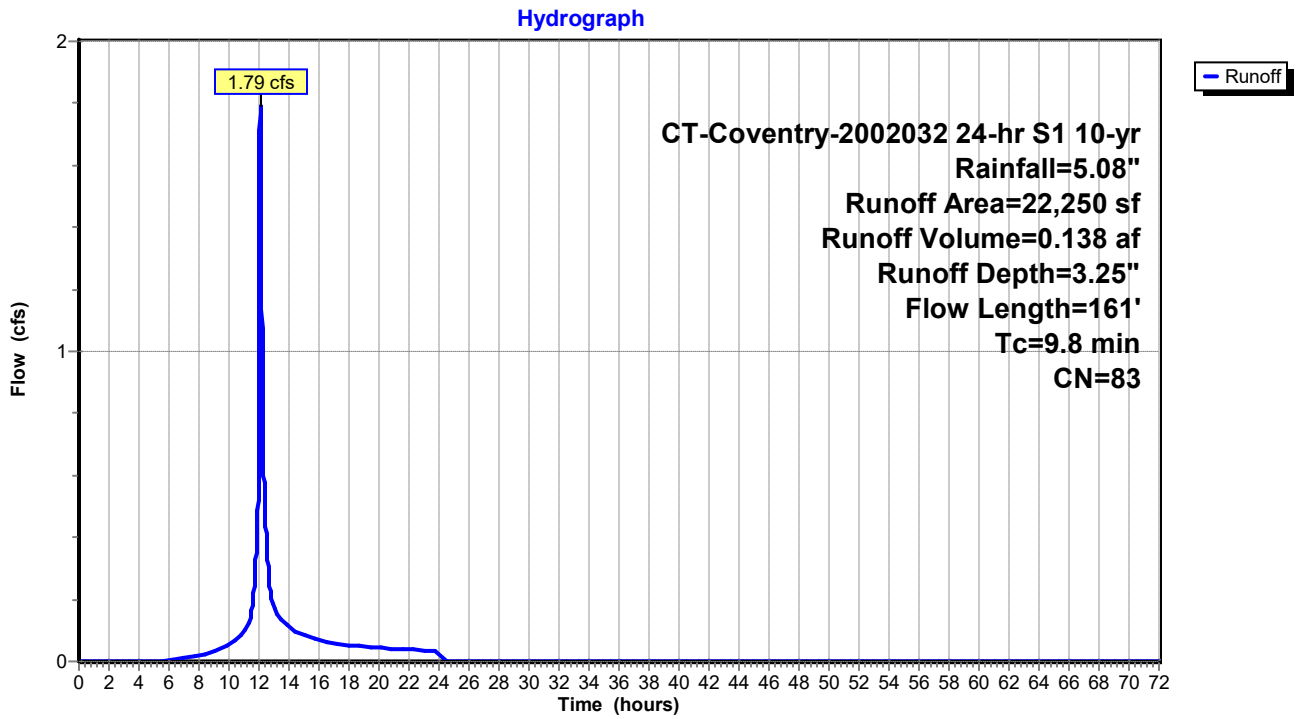
Runoff = 1.79 cfs @ 12.08 hrs, Volume= 0.138 af, Depth= 3.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 CT-Coventry-2002032 24-hr S1 10-yr Rainfall=5.08"

Area (sf)	CN	Description
2,580	69	50-75% Grass cover, Fair, HSG B
13,700	79	50-75% Grass cover, Fair, HSG C
0	60	Woods, Fair, HSG B
0	73	Woods, Fair, HSG C
910	98	Paved parking, HSG B
5,060	98	Paved parking, HSG C
22,250	83	Weighted Average
16,280		73.17% Pervious Area
5,970		26.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.3	100	0.0220	0.18		Sheet Flow, Grass: Short n= 0.150 P2= 3.31"
0.4	23	0.0174	0.92		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.1	38	0.0185	7.16	8.79	Pipe Channel, 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.013 Corrugated PE, smooth interior
9.8	161	Total			

Subcatchment PDA-201: Area to Ex. CBs in Driveway



Summary for Subcatchment PDA-202: Area to Bioretention Basin

Runoff = 3.57 cfs @ 12.17 hrs, Volume= 0.358 af, Depth= 3.44"

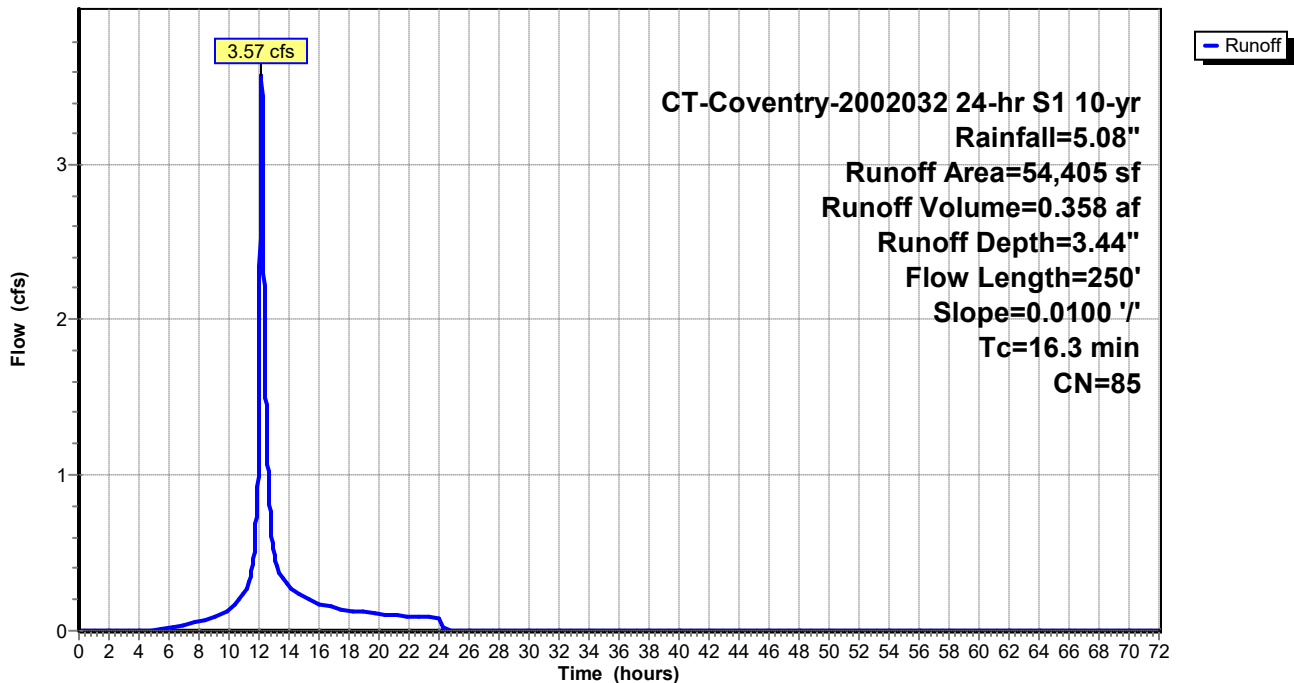
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 CT-Coventry-2002032 24-hr S1 10-yr Rainfall=5.08"

Area (sf)	CN	Description
20,135	69	50-75% Grass cover, Fair, HSG B
6,575	79	50-75% Grass cover, Fair, HSG C
0	60	Woods, Fair, HSG B
0	73	Woods, Fair, HSG C
22,195	98	Paved parking, HSG B
5,500	98	Paved parking, HSG C
54,405	85	Weighted Average
26,710		49.09% Pervious Area
27,695		50.91% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7	100	0.0100	0.13		Sheet Flow, Grass: Short n= 0.150 P2= 3.31"
3.6	150	0.0100	0.70		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
16.3	250	Total			

Subcatchment PDA-202: Area to Bioretention Basin

Hydrograph



Summary for Subcatchment PDA-203: Building Area

Runoff = 1.47 cfs @ 12.03 hrs, Volume= 0.100 af, Depth= 4.84"

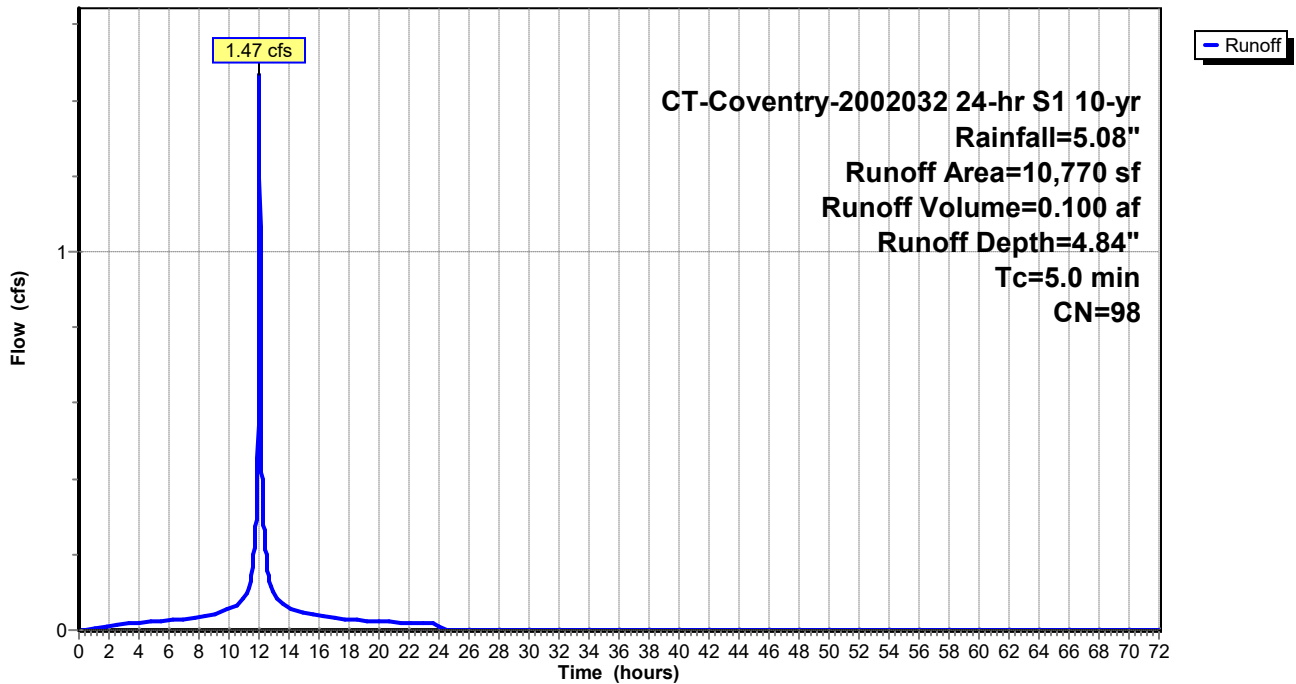
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 CT-Coventry-2002032 24-hr S1 10-yr Rainfall=5.08"

Area (sf)	CN	Description
0	69	50-75% Grass cover, Fair, HSG B
0	79	50-75% Grass cover, Fair, HSG C
0	60	Woods, Fair, HSG B
0	73	Woods, Fair, HSG C
10,770	98	Paved parking, HSG B
0	98	Paved parking, HSG C
10,770	98	Weighted Average
10,770		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Subcatchment PDA-203: Building Area

Hydrograph



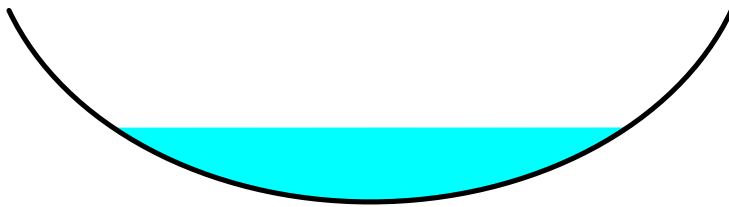
Summary for Reach 1R: Swale to Basin

Inflow Area = 0.247 ac, 100.00% Impervious, Inflow Depth = 4.84" for 10-yr event
 Inflow = 1.47 cfs @ 12.03 hrs, Volume= 0.100 af
 Outflow = 1.28 cfs @ 12.11 hrs, Volume= 0.100 af, Atten= 13%, Lag= 4.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 Max. Velocity= 1.98 fps, Min. Travel Time= 3.1 min
 Avg. Velocity = 0.62 fps, Avg. Travel Time= 9.9 min

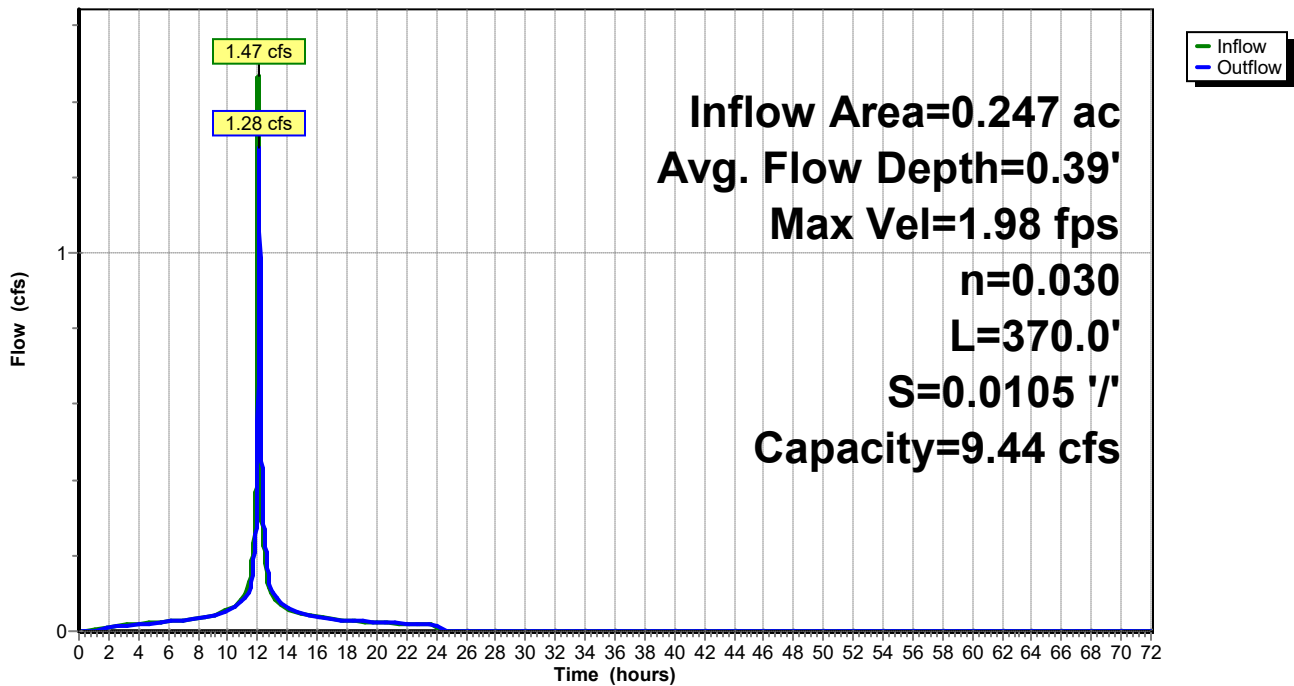
Peak Storage= 239 cf @ 12.05 hrs
 Average Depth at Peak Storage= 0.39'
 Bank-Full Depth= 1.00' Flow Area= 2.7 sf, Capacity= 9.44 cfs

4.00' x 1.00' deep Parabolic Channel, n= 0.030 Earth, grassed & winding
 Length= 370.0' Slope= 0.0105 '/'
 Inlet Invert= 665.90', Outlet Invert= 662.00'



Reach 1R: Swale to Basin

Hydrograph



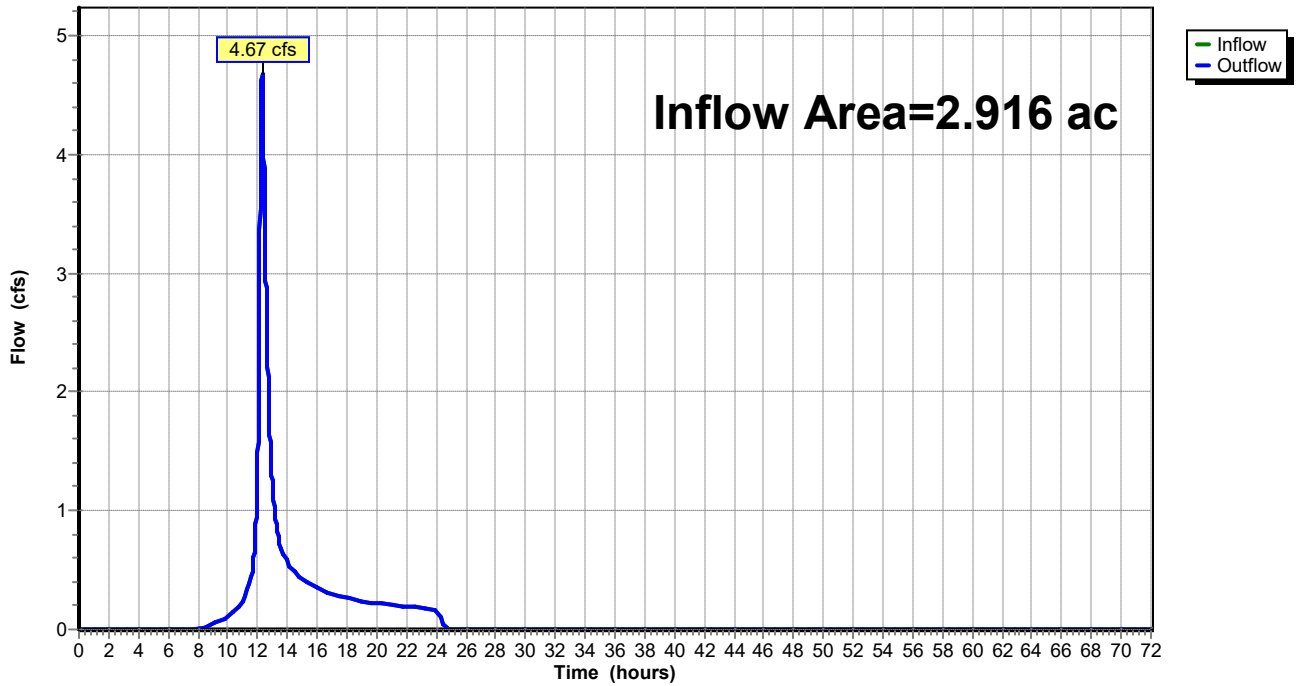
Summary for Reach DP-1: Wetland to Northeast

Inflow Area = 2.916 ac, 12.04% Impervious, Inflow Depth = 2.43" for 10-yr event
 Inflow = 4.67 cfs @ 12.30 hrs, Volume= 0.590 af
 Outflow = 4.67 cfs @ 12.30 hrs, Volume= 0.590 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Reach DP-1: Wetland to Northeast

Hydrograph

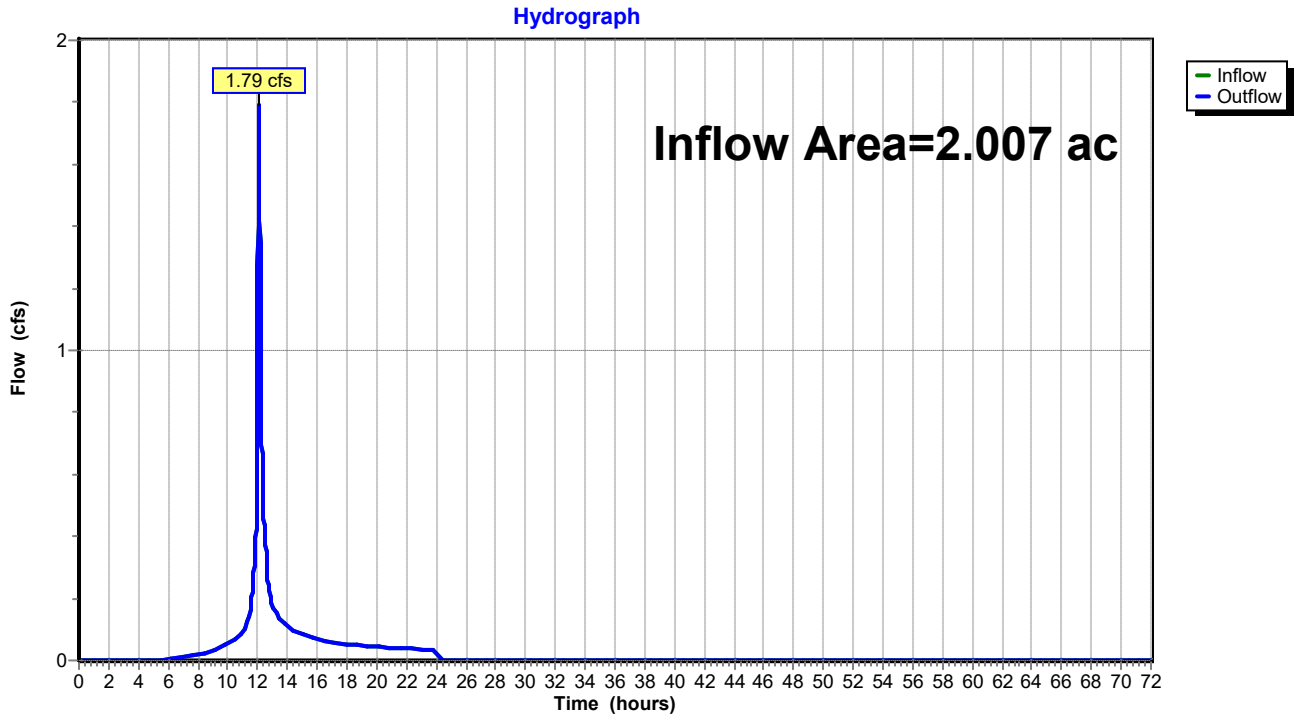


Summary for Reach DP-2: Ex. CBs in Driveway

Inflow Area = 2.007 ac, 50.83% Impervious, Inflow Depth = 0.83" for 10-yr event
Inflow = 1.79 cfs @ 12.08 hrs, Volume= 0.138 af
Outflow = 1.79 cfs @ 12.08 hrs, Volume= 0.138 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Reach DP-2: Ex. CBs in Driveway



Summary for Pond Pond #1: Bioretention Basin

Inflow Area = 1.496 ac, 59.02% Impervious, Inflow Depth = 3.67" for 10-yr event
 Inflow = 4.60 cfs @ 12.14 hrs, Volume= 0.458 af
 Outflow = 0.10 cfs @ 8.55 hrs, Volume= 0.458 af, Atten= 98%, Lag= 0.0 min
 Discarded = 0.10 cfs @ 8.55 hrs, Volume= 0.458 af
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 661.76' @ 24.04 hrs Surf.Area= 4,366 sf Storage= 13,330 cf

Plug-Flow detention time= 1,190.4 min calculated for 0.458 af (100% of inflow)
 Center-of-Mass det. time= 1,190.4 min (2,006.8 - 816.4)

Volume	Invert	Avail.Storage	Storage Description
#1	656.50'	6,986 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 17,464 cf Overall x 40.0% Voids
#2	660.50'	15,465 cf	Custom Stage Data (Prismatic) Listed below (Recalc) -Impervious
		22,450 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
656.50	4,366	0	0
660.50	4,366	17,464	17,464

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
660.50	4,366	0	0
661.00	4,888	2,314	2,314
662.00	6,042	5,465	7,779
663.00	7,775	6,909	14,687
663.10	7,775	778	15,465

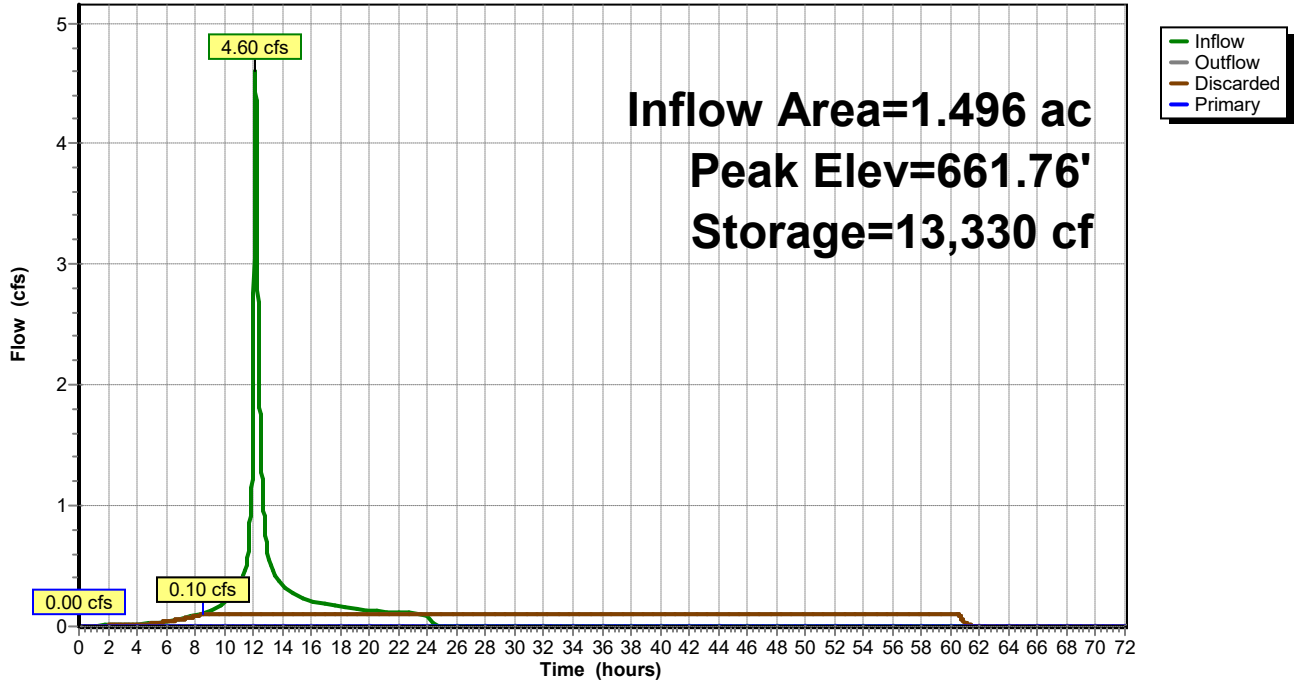
Device	Routing	Invert	Outlet Devices
#1	Discarded	656.50'	1.000 in/hr Exfiltration over Surface area
#2	Primary	658.25'	12.0" Round Culvert L= 69.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 658.25' / 657.90' S= 0.0051 '/ Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	661.87'	24.0" x 36.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.10 cfs @ 8.55 hrs HW=656.57' (Free Discharge)
 ↑1=Exfiltration (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=656.50' (Free Discharge)
 ↑2=Culvert (Controls 0.00 cfs)
 ↑3=Orifice/Grate (Controls 0.00 cfs)

Pond Pond #1: Bioretention Basin

Hydrograph



Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment PDA-101: Area to Wetland Runoff Area=127,015 sf 12.04% Impervious Runoff Depth=4.84"
Flow Length=136' Slope=0.0145 '/' Tc=25.0 min CN=74 Runoff=9.32 cfs 1.175 af

Subcatchment PDA-201: Area to Ex. CBs Runoff Area=22,250 sf 26.83% Impervious Runoff Depth=5.88"
Flow Length=161' Tc=9.8 min CN=83 Runoff=3.17 cfs 0.250 af

Subcatchment PDA-202: Area to Runoff Area=54,405 sf 50.91% Impervious Runoff Depth=6.12"
Flow Length=250' Slope=0.0100 '/' Tc=16.3 min CN=85 Runoff=6.17 cfs 0.637 af

Subcatchment PDA-203: Building Area Runoff Area=10,770 sf 100.00% Impervious Runoff Depth=7.66"
Tc=5.0 min CN=98 Runoff=2.28 cfs 0.158 af

Reach 1R: Swale to Basin Avg. Flow Depth=0.48' Max Vel=2.28 fps Inflow=2.28 cfs 0.158 af
n=0.030 L=370.0' S=0.0105 '/' Capacity=9.44 cfs Outflow=2.03 cfs 0.158 af

Reach DP-1: Wetland to Northeast Inflow=9.32 cfs 1.175 af
Outflow=9.32 cfs 1.175 af

Reach DP-2: Ex. CBs in Driveway Inflow=3.18 cfs 0.552 af
Outflow=3.18 cfs 0.552 af

Pond Pond #1: Bioretention Basin Peak Elev=662.05' Storage=15,071 cf Inflow=7.69 cfs 0.794 af
Discarded=0.10 cfs 0.493 af Primary=2.52 cfs 0.302 af Outflow=2.62 cfs 0.794 af

Total Runoff Area = 4.923 ac Runoff Volume = 2.220 af Average Runoff Depth = 5.41"
72.15% Pervious = 3.552 ac 27.85% Impervious = 1.371 ac

Summary for Subcatchment PDA-101: Area to Wetland to the Northeast

Runoff = 9.32 cfs @ 12.30 hrs, Volume= 1.175 af, Depth= 4.84"

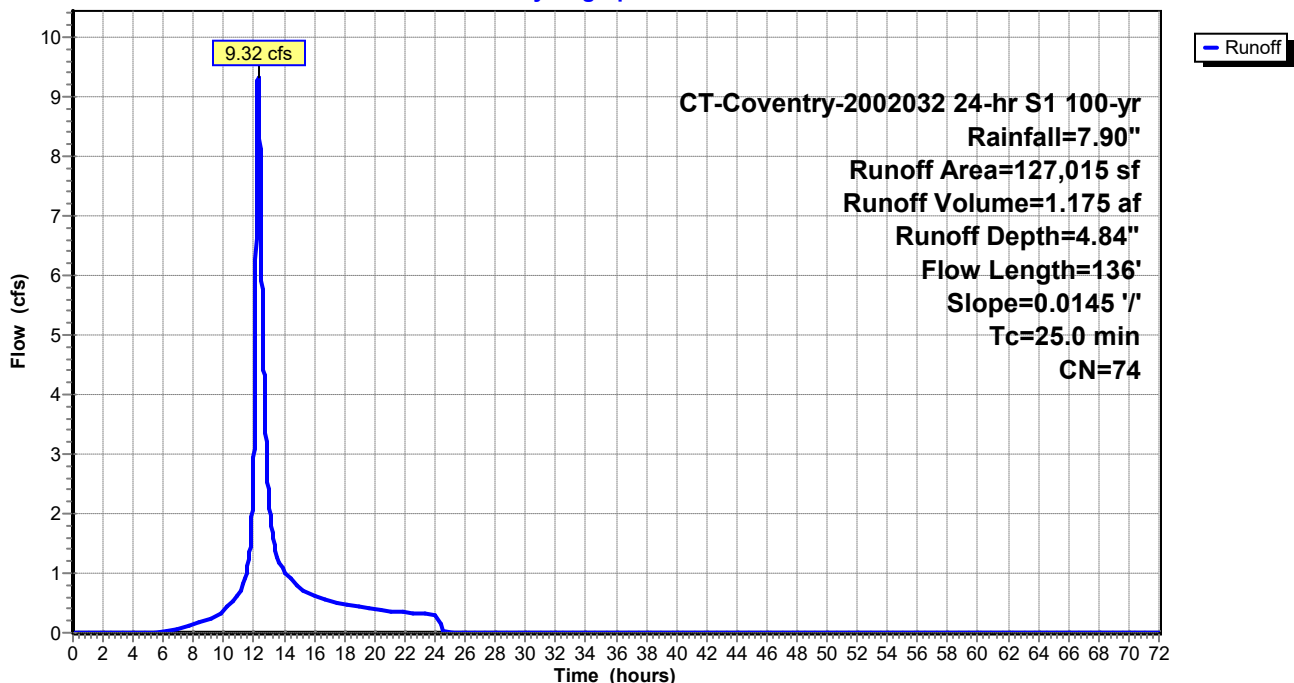
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 CT-Coventry-2002032 24-hr S1 100-yr Rainfall=7.90"

Area (sf)	CN	Description
78,455	69	50-75% Grass cover, Fair, HSG B
10,060	79	50-75% Grass cover, Fair, HSG C
2,445	60	Woods, Fair, HSG B
20,760	73	Woods, Fair, HSG C
13,830	98	Paved parking, HSG B
1,465	98	Paved parking, HSG C
127,015	74	Weighted Average
111,720		87.96% Pervious Area
15,295		12.04% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
24.0	100	0.0145	0.07		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.31"
1.0	36	0.0145	0.60		Shallow Concentrated Flow, Woodland Kv= 5.0 fps
25.0	136	Total			

Subcatchment PDA-101: Area to Wetland to the Northeast

Hydrograph



Summary for Subcatchment PDA-201: Area to Ex. CBs in Driveway

Runoff = 3.17 cfs @ 12.08 hrs, Volume= 0.250 af, Depth= 5.88"

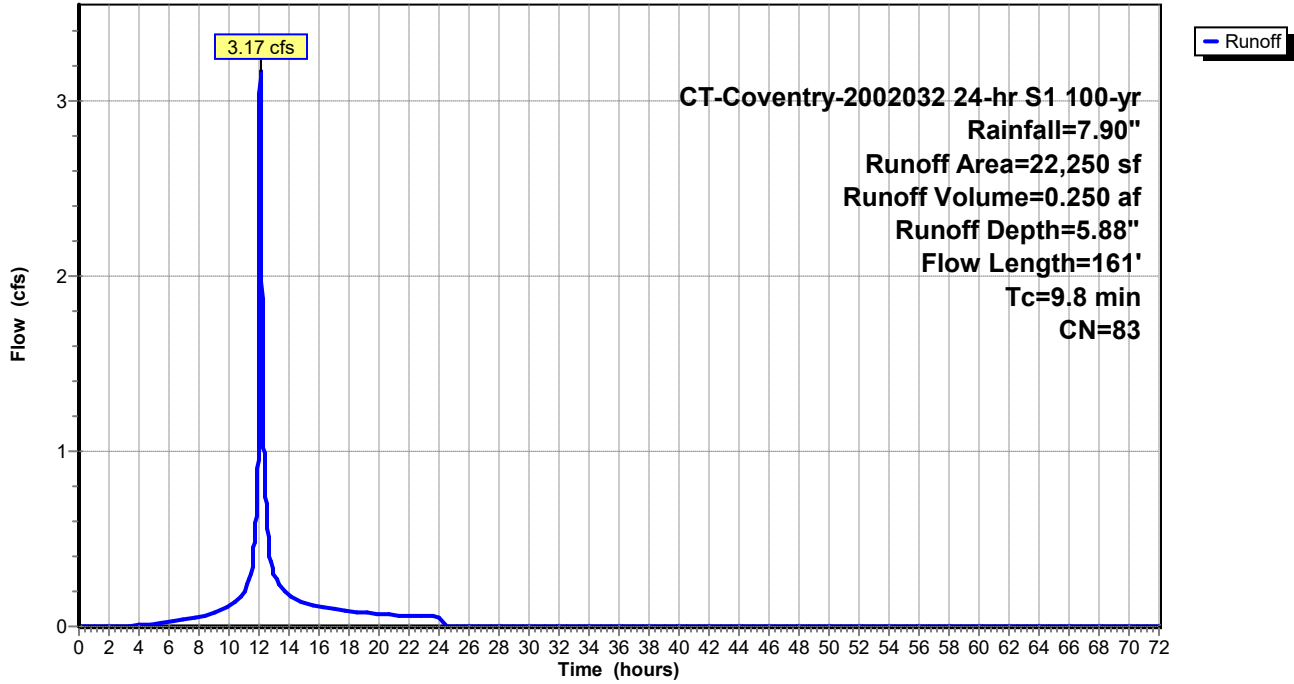
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 CT-Coventry-2002032 24-hr S1 100-yr Rainfall=7.90"

Area (sf)	CN	Description
2,580	69	50-75% Grass cover, Fair, HSG B
13,700	79	50-75% Grass cover, Fair, HSG C
0	60	Woods, Fair, HSG B
0	73	Woods, Fair, HSG C
910	98	Paved parking, HSG B
5,060	98	Paved parking, HSG C
22,250	83	Weighted Average
16,280		73.17% Pervious Area
5,970		26.83% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.3	100	0.0220	0.18		Sheet Flow, Grass: Short n= 0.150 P2= 3.31"
0.4	23	0.0174	0.92		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.1	38	0.0185	7.16	8.79	Pipe Channel, 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.013 Corrugated PE, smooth interior
9.8	161	Total			

Subcatchment PDA-201: Area to Ex. CBs in Driveway

Hydrograph



Summary for Subcatchment PDA-202: Area to Bioretention Basin

Runoff = 6.17 cfs @ 12.17 hrs, Volume= 0.637 af, Depth= 6.12"

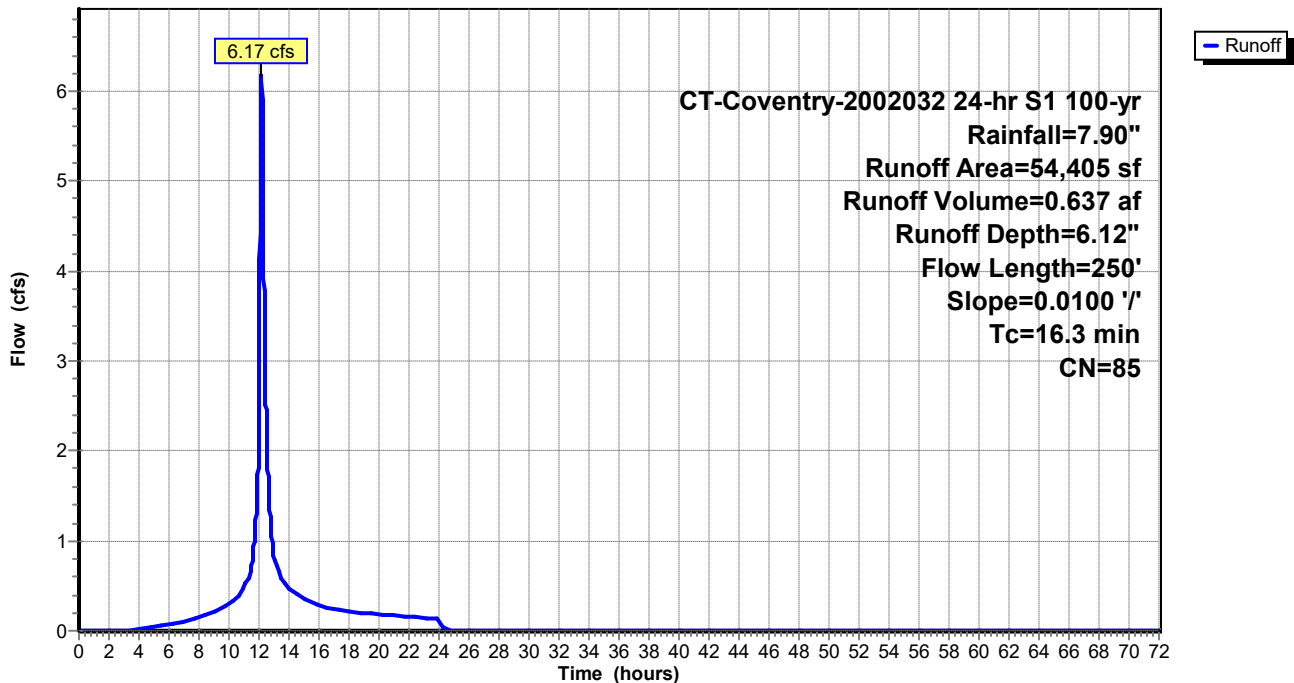
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 CT-Coventry-2002032 24-hr S1 100-yr Rainfall=7.90"

Area (sf)	CN	Description
20,135	69	50-75% Grass cover, Fair, HSG B
6,575	79	50-75% Grass cover, Fair, HSG C
0	60	Woods, Fair, HSG B
0	73	Woods, Fair, HSG C
22,195	98	Paved parking, HSG B
5,500	98	Paved parking, HSG C
54,405	85	Weighted Average
26,710		49.09% Pervious Area
27,695		50.91% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.7	100	0.0100	0.13		Sheet Flow, Grass: Short n= 0.150 P2= 3.31"
3.6	150	0.0100	0.70		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
16.3	250	Total			

Subcatchment PDA-202: Area to Bioretention Basin

Hydrograph



Summary for Subcatchment PDA-203: Building Area

Runoff = 2.28 cfs @ 12.03 hrs, Volume= 0.158 af, Depth= 7.66"

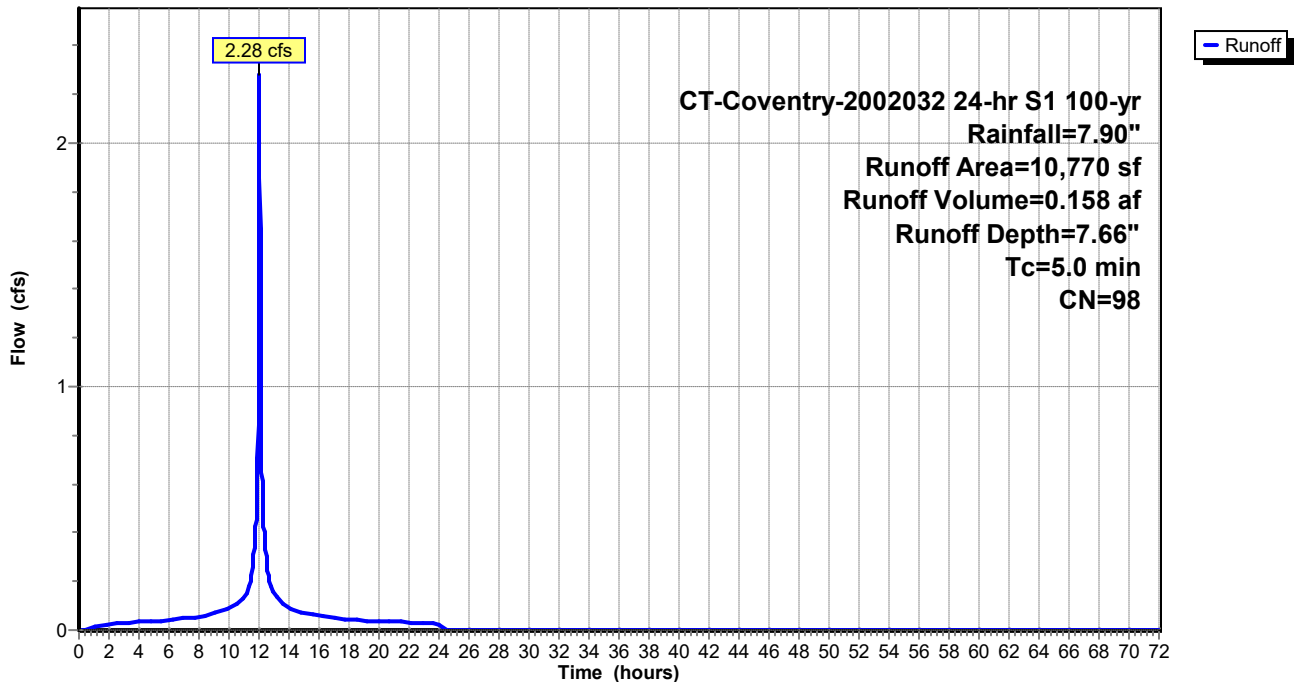
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 CT-Coventry-2002032 24-hr S1 100-yr Rainfall=7.90"

Area (sf)	CN	Description
0	69	50-75% Grass cover, Fair, HSG B
0	79	50-75% Grass cover, Fair, HSG C
0	60	Woods, Fair, HSG B
0	73	Woods, Fair, HSG C
10,770	98	Paved parking, HSG B
0	98	Paved parking, HSG C
10,770	98	Weighted Average
10,770		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

Subcatchment PDA-203: Building Area

Hydrograph



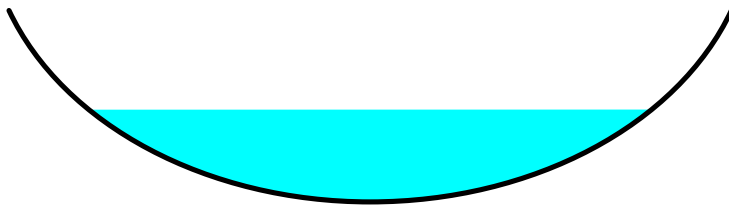
Summary for Reach 1R: Swale to Basin

Inflow Area = 0.247 ac, 100.00% Impervious, Inflow Depth = 7.66" for 100-yr event
 Inflow = 2.28 cfs @ 12.03 hrs, Volume= 0.158 af
 Outflow = 2.03 cfs @ 12.10 hrs, Volume= 0.158 af, Atten= 11%, Lag= 4.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
 Max. Velocity= 2.28 fps, Min. Travel Time= 2.7 min
 Avg. Velocity = 0.71 fps, Avg. Travel Time= 8.6 min

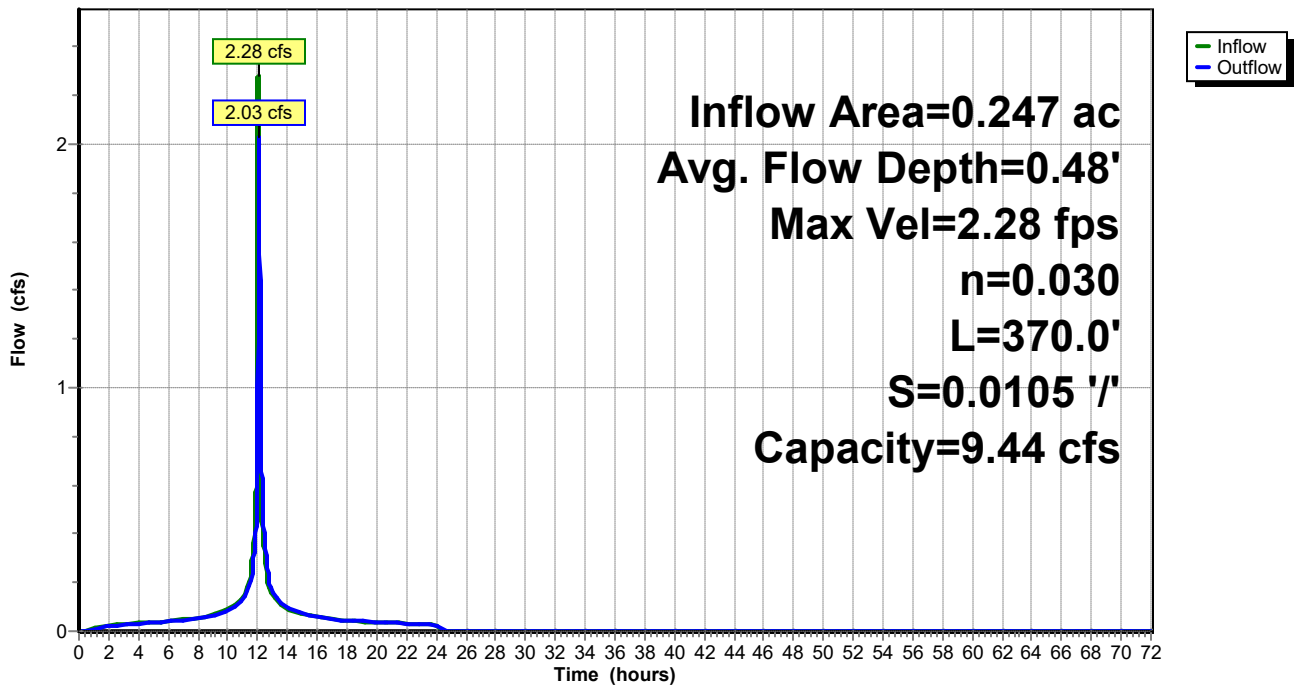
Peak Storage= 331 cf @ 12.05 hrs
 Average Depth at Peak Storage= 0.48'
 Bank-Full Depth= 1.00' Flow Area= 2.7 sf, Capacity= 9.44 cfs

4.00' x 1.00' deep Parabolic Channel, n= 0.030 Earth, grassed & winding
 Length= 370.0' Slope= 0.0105 '/'
 Inlet Invert= 665.90', Outlet Invert= 662.00'



Reach 1R: Swale to Basin

Hydrograph



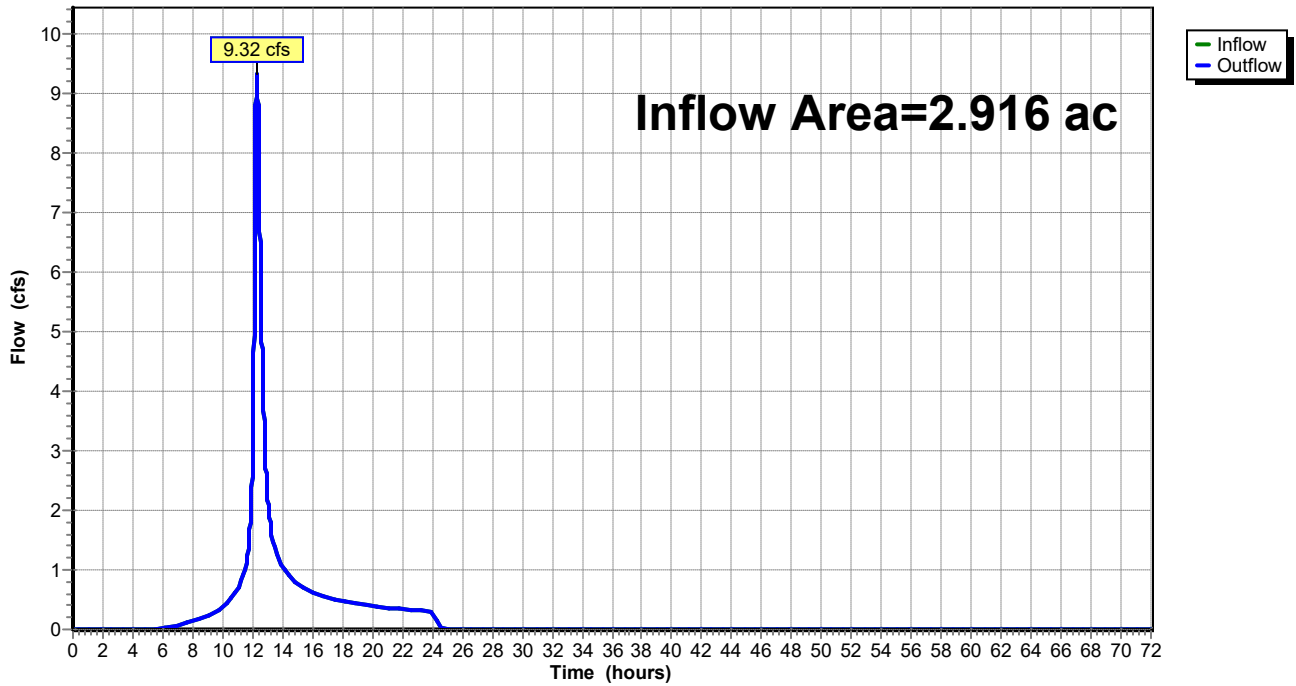
Summary for Reach DP-1: Wetland to Northeast

Inflow Area = 2.916 ac, 12.04% Impervious, Inflow Depth = 4.84" for 100-yr event
 Inflow = 9.32 cfs @ 12.30 hrs, Volume= 1.175 af
 Outflow = 9.32 cfs @ 12.30 hrs, Volume= 1.175 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Reach DP-1: Wetland to Northeast

Hydrograph



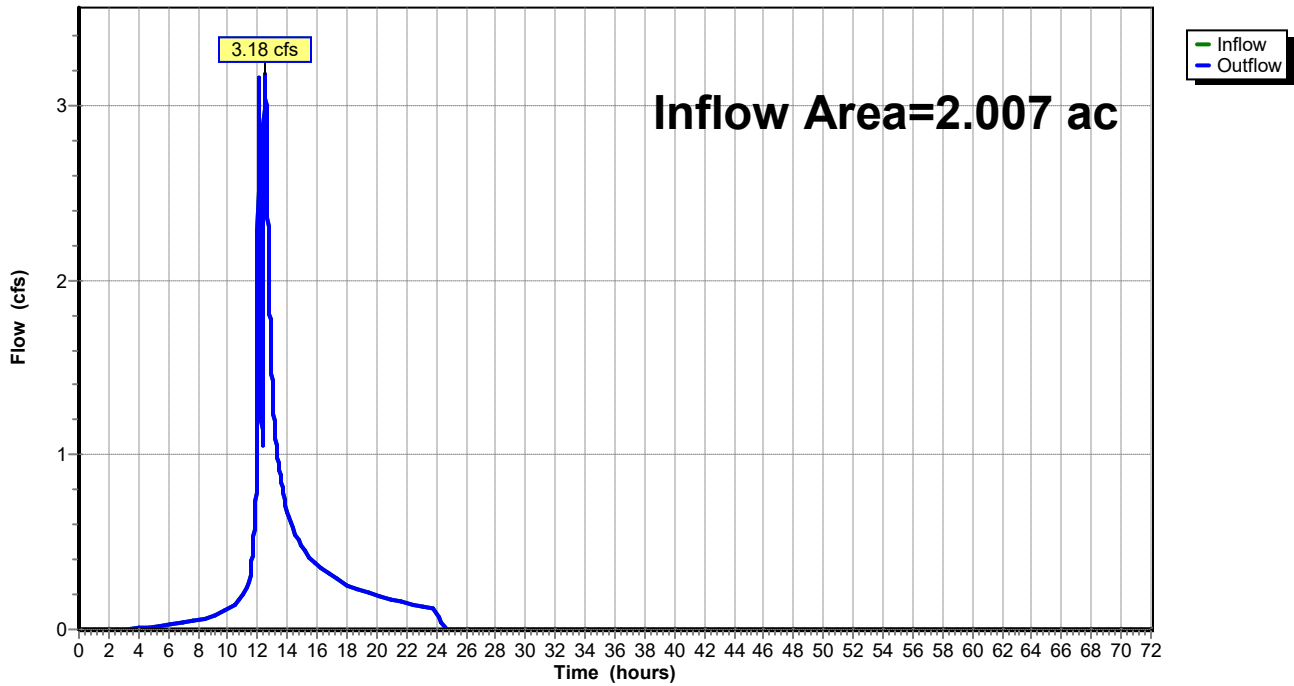
Summary for Reach DP-2: Ex. CBs in Driveway

Inflow Area = 2.007 ac, 50.83% Impervious, Inflow Depth = 3.30" for 100-yr event
Inflow = 3.18 cfs @ 12.49 hrs, Volume= 0.552 af
Outflow = 3.18 cfs @ 12.49 hrs, Volume= 0.552 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

Reach DP-2: Ex. CBs in Driveway

Hydrograph



Summary for Pond Pond #1: Bioretention Basin

Inflow Area = 1.496 ac, 59.02% Impervious, Inflow Depth = 6.37" for 100-yr event
 Inflow = 7.69 cfs @ 12.14 hrs, Volume= 0.794 af
 Outflow = 2.62 cfs @ 12.50 hrs, Volume= 0.794 af, Atten= 66%, Lag= 21.7 min
 Discarded = 0.10 cfs @ 5.84 hrs, Volume= 0.493 af
 Primary = 2.52 cfs @ 12.50 hrs, Volume= 0.302 af

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 2
 Peak Elev= 662.05' @ 12.50 hrs Surf.Area= 4,366 sf Storage= 15,071 cf

Plug-Flow detention time= 786.5 min calculated for 0.794 af (100% of inflow)
 Center-of-Mass det. time= 786.5 min (1,586.2 - 799.7)

Volume	Invert	Avail.Storage	Storage Description
#1	656.50'	6,986 cf	Custom Stage Data (Prismatic) Listed below (Recalc) 17,464 cf Overall x 40.0% Voids
#2	660.50'	15,465 cf	Custom Stage Data (Prismatic) Listed below (Recalc) -Impervious
		22,450 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
656.50	4,366	0	0
660.50	4,366	17,464	17,464

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
660.50	4,366	0	0
661.00	4,888	2,314	2,314
662.00	6,042	5,465	7,779
663.00	7,775	6,909	14,687
663.10	7,775	778	15,465

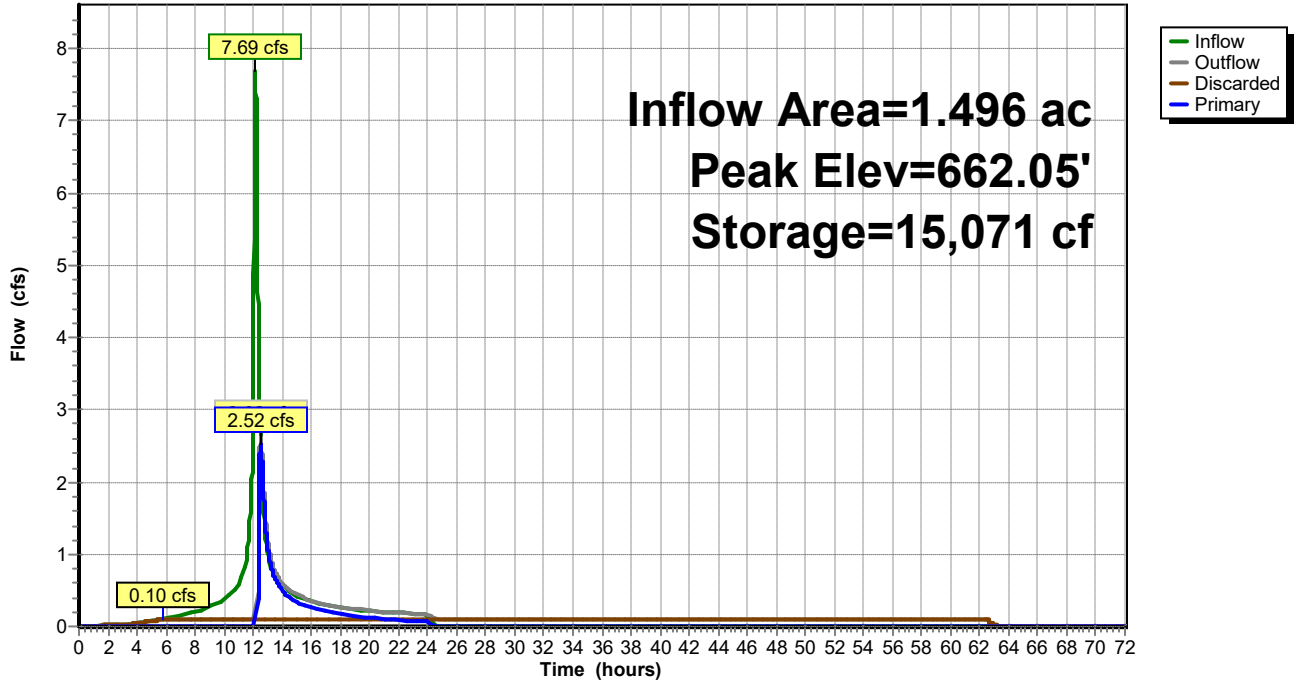
Device	Routing	Invert	Outlet Devices
#1	Discarded	656.50'	1.000 in/hr Exfiltration over Surface area
#2	Primary	658.25'	12.0" Round Culvert L= 69.0' CPP, end-section conforming to fill, Ke= 0.500 Inlet / Outlet Invert= 658.25' / 657.90' S= 0.0051 '/ Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#3	Device 2	661.87'	24.0" x 36.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.10 cfs @ 5.84 hrs HW=656.57' (Free Discharge)
 ↑1=Exfiltration (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=2.50 cfs @ 12.50 hrs HW=662.05' (Free Discharge)
 ↑2=Culvert (Passes 2.50 cfs of 5.84 cfs potential flow)
 ↑3=Orifice/Grate (Weir Controls 2.50 cfs @ 1.39 fps)

Pond Pond #1: Bioretention Basin

Hydrograph



APPENDIX D

WATER QUALITY CALCUATIONS

CTDEEP Water Quality Volume Calculations

Bioretention Basin Calculation

Groundwater Recharge Calculation

Treatment Train Efficiency Worksheet

Water Quality Calculations

Determine Water Quality Volume

From CT 2004 Stormwater Quality Manual:

$$WQV = \frac{(I)(R)(A)}{12}$$

$$R = 0.05 + 0.009(I)$$

WQV = water quality volume (ac-ft)

R = volumetric runoff coefficient

I = percent impervious cover

A = site area in acres

WQv = Calculated Water Quality Volume

Area	ID	Total Area		Impervious Area		Impervious Cover	Volumetric Runoff Coefficient	Water Quality Volume (WQV)		Proposed Water Quality Volume (WQV)	
		ac	ft ²	ac	ft ²	%	R	acre-feet	ft ³	acre-feet	ft ³
Area to Bioretention Basin	PDA 202/PDA 203	1.496	65,175	0.883	38,465	59.02	0.581	0.072	3,136	0.117	5,079

Bioretention Basin Calculations

Surface area of the Bioretention System
 $SA = (WQv) / hf$

WQv = Calculated Water Quality Volume
 hf = depth of ponding above soil media in feet)

		Water Quality Volume Required (CF)	Depth of Ponding (FT)	Required Surface Area (SF)	Surface Area Provided (SF)	WQV Provided in Ponded Depth (CF)
Bioretention Basin #1	PDA 202/PDA 203	3,136	1.37	2,289	4,366	5,079

Groundwater Recharge Volume Calculations

Groundwater Recharge Volume

From CT 2004 Stormwater Quality Manual:

$$GVR = \frac{(D)(A)(I)}{12}$$

GRV = Groundwater Recharge Volume (ac-ft)
 D = Depth of Runoff to be Recharged (table 7-4)
 A = site area in acres
 I = impervious cover (decimal)
 WQv = Calculated Water Quality Volume

1.37

4366

	A	Site Area by NRCS Hydrologic Soil Group				Impervious Cover by NRCS Hydrologic Soil Group				Site Imperviousness (Decimal) by NRCS Hydrologic Soil Group				GRV Required (ac-ft)	Potential Recharge Pond Volumes Proposed (ac-ft)
	Total Site Area (AC)	A	B	C	D	A	B	C	D	A	B	C	D		
	1.87	0.00	1.46	0.41	0.00	0.00	0.76	0.13	0.00	0.00	0.41	0.07	0.00		

**Table 7-4
Groundwater Recharge Depth**

NRCS Hydrologic Soil Group	Average Annual Recharge	Groundwater Recharge Depth (D)
A	18 inches/year	0.4 inches
B	12 inches/year	0.25 inches
C	6 inches/year	0.10 inches
D	3 inches/year	0 inches (waived)

Source: MADRP, 1997.
 NRCS – Natural Resources Conservation Service

Best Management Practice (BMP) Treatment Train Efficiency Worksheet

Prepared for:
Proposed Retail Development
1100 Boston Turnpike
Bolton, Connecticut

Prepared by:
BL Companies
100 Constitution Plaza, 10th Floor
Hartford Connecticut

Date prepared:
April 2, 2021

Overall Site Treatment Train Efficiency

Ei=[1-(1-E1)(1-E2)(1-E3)(1-E4)(1-E?)]*100	<u>BMP</u>	<u>BMP Description</u>	<u>Type of Treatment</u>	<u>Efficiency Rate %</u>
		E1	Impervious Surface Sweeping***	Secondary (conventional)
	E2	Bioretention Basin	Primary	90

Overall Treatment Train Efficiency (Et)= **91 % Total Suspended Solids (TSS) Remova**

* 80% require per CT DEP
 ** Manufacturers claim 80% TSS removal
 *** Schueler 1996 & EPA 1993
 **** University of New Hampshire

<u>BMP</u>	<u>Type of Treatment</u>	<u>TSS Removal Rate</u>	<u>Starting TSS Load</u>	<u>Amount Removed</u>	<u>Remaining Load</u>
		0.10	1.00	0.10	0.90
0.9	0.90	0.81	0.09		

Overall Treatment Train Efficiency (%) **91**

TSS Removal Rates (adapted from Schueler, 1996, & EPA, 1993)

BMP List	Design Rate	Range of Average TSS Removal Rates	Brief Design Requirements
Extended Detention Pond	70%	60-80%	Sediment forebay
Wet Pond (a)	70%	60-80%	Sediment forebay
Constructed Wetland (b)	80%	65-80%	Designed to infiltrate or retain
Water Quality Swale	70%	60-80%	Designed to infiltrate or retain
Infiltration Trench	80%	75-80%	Pretreatment critical
Infiltration Basin	80%	75-80% (predicted)	Pretreatment critical
Dry Well	80%	80% (predicted)	Rooftop runoff (uncontaminated only)
Sand Filter (c)	80%	80%	Pretreatment
Organic Filter (d)	80%	80%+	Pretreatment
Water Quality Inlet	25%	15-35% w/ cleanout	Off-line only; 0.1" minimum Water Quality Volume (WQV) storage
Sediment Trap (Forebay)	25%	25% w/ cleanout	Storm flows for 2-year event must not cause erosion; 0.1" minimum WQV storage
Drainage Channel	25%	25%	Check dams; non-erosive for 2-yr.
Deep Sump and Hooded Catch Basin	25%	25% w/ cleanout	Deep sump general rule = 4 x pipe diameter or 4.0' for pipes 18" or less
Street Sweeping	10%	10%	Discretionary non-structural credit, must be part of approved plan

APPENDIX E

SUBSURFACE SOIL INVESTIGATION LOGS

Test Pit Logs

Falling Head Permeability Test Logs



**Proposed Retail Development - 1100 Boston Turnpike
Bolton, CT**

TP-1
BL Project #2002032
May 4, 2021

TEST PIT FIELD LOG

PERSONNEL PRESENT	EXCAVATION EQUIPMENT	
Cody L'Heureux- BL Companies	Contractor _____	Ground Surface Elevation <u>662.50</u>
	Operator _____	Datum <u>NAVD 88</u>
	Make _____ Model _____	Temperature <u>54</u>
	Bucket Capacity _____ Reach _____	Weather <u>Cloudy w/ Rain</u>

Depth	SOIL DESCRIPTION	Excav. Effort	Cobble and Boulder Data	Remark No.
0"-2"	Topsoil	E		
2"-60"	Dark brown coarse sand with trace cobbles	E	TR C	
60"-120"	Dark brown silty sand	E		1
Bottom of Test Pit at 120" (10')				

REMARKS:

1. Ground water was observed at 8'.
2. Bedrock was not observed.

TEST PIT PLAN	LEGEND			
<p>North</p>	COBBLES AND BOULDERS Size Range Letter Classification Designation 3" - 12" Cobble (C) 12" - 24 Small (S) 24" - 36" Medium (M) 36" and Larger Large (L)	PROPORTIONS USED (QUANTITATIVE TERMS) TRACE (TR) 0-10% LITTLE (LI) 10-20% SOME (SO) 20-35% MANY (MA) 35-50%	QUALITATIVE TERMS OCCASIONAL FEW FREQUENT NUMEROUS	EXCAVATION EFFORT E - Easy M - Moderate D - Difficult Observed Depth to Groundwater



**Proposed Retail Development - 1100 Boston Turnpike
Bolton, CT**

TP-2
BL Project #2002032
May 4, 2021

TEST PIT FIELD LOG

PERSONNEL PRESENT	EXCAVATION EQUIPMENT	
Cody L'Heureux- BL Companies	Contractor _____	Ground Surface Elevation <u>662.30</u>
	Operator _____	Datum <u>NAVD 88</u>
	Make _____ Model _____	Temperature <u>54</u>
	Bucket Capacity _____ Reach _____	Weather <u>Cloudy w/ Rain</u>

Depth	SOIL DESCRIPTION	Excav. Effort	Cobble and Boulder Data	Remark No.
0"-6"	Topsoil	E		
6"-72"	Dark brown coarse sand with trace cobbles	E	TR C	
72"-120"	Dark brown silty sand	E		1
Bottom of Test Pit at 120" (10')				

REMARKS:

1. Ground water was observed at 8'.
2. Bedrock was not observed.

TEST PIT PLAN	LEGEND			
	COBBLES AND BOULDERS	PROPORTIONS USED (QUANTITATIVE TERMS)	QUALITATIVE TERMS	EXCAVATION EFFORT
	Size Range Letter Classification Designation 3" - 12" Cobble (C) 12" - 24" Small (S) 24" - 36" Medium (M) 36" and Larger Large (L)	TRACE (TR) 0-10% LITTLE (LI) 10-20% SOME (SO) 20-35% MANY (MA) 35-50%	OCCASIONAL FEW FREQUENT NUMEROUS	E - Easy M - Moderate D - Difficult ▼ Observed Depth to Groundwater



**Proposed Retail Development - 1100 Boston Turnpike
Bolton, CT**

TP-3
BL Project #2002032
May 4, 2021

TEST PIT FIELD LOG

PERSONNEL PRESENT	EXCAVATION EQUIPMENT			
Cody L'Heureux- BL Companies	Contractor _____	Ground Surface Elevation	661.90	
	Operator _____	Datum	NAVD 88	
	Make _____ Model _____	Temperature	54	
	Bucket Capacity _____ Reach _____	Weather	Cloudy w/ Rain	

Depth	SOIL DESCRIPTION	Excav. Effort	Cobble and Boulder Data	Remark No.
0"-6"	Topsoil	E		
6"-66"	Dark brown coarse sand with trace cobbles	E	TR C	
66"-120"	Dark brown silty sand	E		1
Bottom of Test Pit at 120" (10')				

REMARKS:

1. Ground water was observed at 7'.
2. Bedrock was not observed.

TEST PIT PLAN	LEGEND																																
	COBBLES AND BOULDERS	PROPORTIONS USED (QUANTITATIVE TERMS)	QUALITATIVE TERMS	EXCAVATION EFFORT																													
<p>North</p>	<table border="0"> <tr> <td>Size Range</td> <td>Letter</td> </tr> <tr> <td>Classification</td> <td>Designation</td> </tr> <tr> <td>3" - 12"</td> <td>Cobble (C)</td> </tr> <tr> <td>12" - 24"</td> <td>Small (S)</td> </tr> <tr> <td>24" - 36"</td> <td>Medium (M)</td> </tr> <tr> <td>36" and Larger</td> <td>Large (L)</td> </tr> </table>	Size Range	Letter	Classification	Designation	3" - 12"	Cobble (C)	12" - 24"	Small (S)	24" - 36"	Medium (M)	36" and Larger	Large (L)	<table border="0"> <tr> <td>TRACE (TR)</td> <td>0-10%</td> </tr> <tr> <td>LITTLE (LI)</td> <td>10-20%</td> </tr> <tr> <td>SOME (SO)</td> <td>20-35%</td> </tr> <tr> <td>MANY (MA)</td> <td>35-50%</td> </tr> </table>	TRACE (TR)	0-10%	LITTLE (LI)	10-20%	SOME (SO)	20-35%	MANY (MA)	35-50%	<table border="0"> <tr> <td>OCCASIONAL</td> </tr> <tr> <td>FEW</td> </tr> <tr> <td>FREQUENT</td> </tr> <tr> <td>NUMEROUS</td> </tr> </table>	OCCASIONAL	FEW	FREQUENT	NUMEROUS	<table border="0"> <tr> <td>E - Easy</td> </tr> <tr> <td>M - Moderate</td> </tr> <tr> <td>D - Difficult</td> </tr> <tr> <td align="center">▼</td> <td>Observed Depth to Groundwater</td> </tr> </table>	E - Easy	M - Moderate	D - Difficult	▼	Observed Depth to Groundwater
Size Range	Letter																																
Classification	Designation																																
3" - 12"	Cobble (C)																																
12" - 24"	Small (S)																																
24" - 36"	Medium (M)																																
36" and Larger	Large (L)																																
TRACE (TR)	0-10%																																
LITTLE (LI)	10-20%																																
SOME (SO)	20-35%																																
MANY (MA)	35-50%																																
OCCASIONAL																																	
FEW																																	
FREQUENT																																	
NUMEROUS																																	
E - Easy																																	
M - Moderate																																	
D - Difficult																																	
▼	Observed Depth to Groundwater																																

FALLING HEAD PERMEABILITY TEST

PROJECT: Proposed Retail Development
Bolton, CT

PROJECT #2002032
DATE: 5/4/2021

BY: C.J.L.

TEST PIT # 1

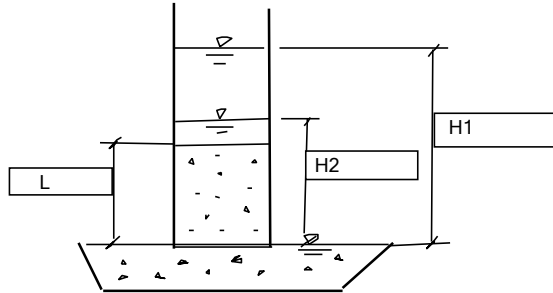
SAMPLE TP-1

SAMPLE LENGTH: 4.50 in.

SAMPLE DEPTH (BELOW EG): 6.00 ft

presoak start: 10:00 am

presoak finish: 10:30 am



$$K = \frac{(H1 - H2) \times L}{t \times (H1 + H2)/2}$$

Time (min.)	H1 (in.)	H2 (in.)	H1 - H2	(H1 + H2)/2	K (in/min.)	K (ft./day)
0.000	6.500	6.500	0.000	6.500	-	-
5.000	6.500	6.260	0.240	6.380	0.034	4.063
10.000	6.500	5.960	0.540	6.230	0.039	4.681
15.000	6.500	5.720	0.780	6.110	0.038	4.596
20.000	6.500	5.540	0.960	6.020	0.036	4.306
25.000	6.500	5.420	1.080	5.960	0.033	3.914
30.000	6.500	5.300	1.200	5.900	0.031	3.661
35.000	6.500	5.060	1.440	5.780	0.032	3.844
40.000	6.500	4.880	1.620	5.690	0.032	3.844
45.000	6.500	4.680	1.820	5.590	0.033	3.907
50.000	6.500	4.500	2.000	5.500	0.033	3.927
					Average=	4.074 ft/day
					or	2.04 in/hr

FALLING HEAD PERMEABILITY TEST

PROJECT: Proposed Retail Development
Bolton, CT

PROJECT #2002032
DATE: 5/4/2021

BY: C.J.L.

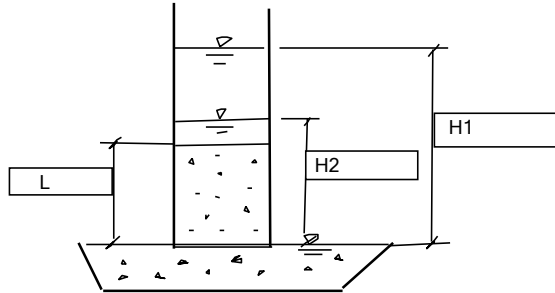
TEST PIT # 1

SAMPLE TP-2

SAMPLE LENGTH: 4.50 in.

SAMPLE DEPTH (BELOW EG): 6.00 ft

presoak start: 10:00 am
presoak finish: 10:30 am



$$K = \frac{(H1 - H2) \times L}{t \times (H1 + H2)/2}$$

Time (min.)	H1 (in.)	H2 (in.)	H1 - H2	(H1 + H2)/2	K (in/min.)	K (ft./day)
0.000	6.500	6.500	0.000	6.500	-	-
5.000	6.500	6.170	0.330	6.335	0.047	5.626
10.000	6.500	5.880	0.620	6.190	0.045	5.409
15.000	6.500	5.650	0.850	6.075	0.042	5.037
20.000	6.500	5.300	1.200	5.900	0.046	5.492
25.000	6.500	5.060	1.440	5.780	0.045	5.381
30.000	6.500	4.680	1.820	5.590	0.049	5.860
35.000	6.500	4.500	2.000	5.500	0.047	5.610
40.000	6.500	4.300	2.200	5.400	0.046	5.500
45.000	6.500	4.000	2.500	5.250	0.048	5.714
					Average=	5.514 ft/day
					or	2.76 in/hr

FALLING HEAD PERMEABILITY TEST

PROJECT: Proposed Retail Development
Bolton, CT

PROJECT #2002032
DATE: 5/4/2021

BY: C.J.L.

TEST PIT # 1

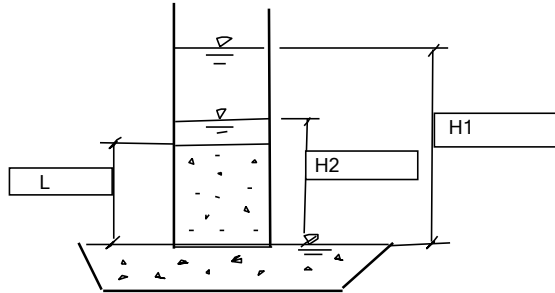
SAMPLE TP-3

SAMPLE LENGTH: 4.00 in.

SAMPLE DEPTH (BELOW EG): 6.00 ft

presoak start: 10:00 am

presoak finish: 10:30 am



$$K = \frac{(H1 - H2) \times L}{t \times (H1 + H2)/2}$$

Time (min.)	H1 (in.)	H2 (in.)	H1 - H2	(H1 + H2)/2	K (in/min.)	K (ft./day)
0.000	6.500	6.500	0.000	6.500	-	-
5.000	6.500	6.180	0.320	6.340	0.040	4.845
10.000	6.500	5.540	0.960	6.020	0.064	7.654
15.000	6.500	5.060	1.440	5.780	0.066	7.972
20.000	6.500	4.280	2.220	5.390	0.082	9.885
25.000	6.500	4.220	2.280	5.360	0.068	8.167
30.000	6.500	4.000	2.500	5.250	0.063	7.619
					Average=	7.691 ft/day
					or	3.85 in/hr

May 5, 2021

Patrice L. Carson, AICP, Director of Community Development
Town of Bolton
222 Bolton Center Road
Bolton, CT 06043

Re: Special Permit Application
2-Lot Subdivision Application
Proposed Retail Development
1100 Boston Turnpike

Dear Ms. Carson:

We are in receipt of engineering comments dated April 26, 2021, from Nathan L. Jacobson & Associates, Inc., and your comments dated May 3, 2021, regarding the project referenced above. Our responses below are shown in ***bold italic*** text. We have provided the following additional information documenting our responses:

- Land Development Plans, revised to May 5, 2021
- Stormwater Management Narrative and Hydrologic Calculations, dated May 5, 2021

NATHAN L. JACOBSON & ASSOCIATES, INC.

1. Referral should be made to the Connecticut Department of Transportation (CTDOT) for work proposed within the State right-of-way.

Response: Reference to CTDOT requirements for permanent pavement repair in Route 44 has been included on the revised Site Plan sheet SP-1. The CTDOT standard details for trench repair are included on Details Sheet DN-6.

2. An application should be made to the Bolton Lakes Regional Water Pollution Authority (BLRWPCA) prior to the installation of the pressure line and sewage pump station.

Response: Acknowledged. An application to the BLRWPCA has been initiated; coordination is ongoing.

3. We would recommend that an underdrain be placed along the centerline of the proposed water quality basin to help to drain the basin between rain events and prevent problems with standing water.

Response: A 4" perforated HDPE underdrain has been added to the water quality basin. A raised underdrain system is being utilized to create an internal water storage zone within the bioretention soil media. This configuration will allow for a 6" depth of submerged bioretention soil to provide enhanced nitrogen removal. Please refer to the updated Grading and Drainage Plan sheet GD-1 and Details Sheet DN-3.

4. An existing conditions model should be provided for the subarea that contributes to the water quality basin on the Bolton Dental site. From the data provided in the Stormwater Management Report, it appears that flows being sent to the existing basin will be minimal, but it should be quantified.

Response: Acknowledged. A supplementary Stormwater Management Narrative and Hydrologic Calculations letter has been provided with this submission. The letter details and compares the existing and proposed condition peak flows directed offsite. As indicated in the letter, peak flow directed to the Bolton Dental site is matched in the 2, 10, and 100-year storm events.

5. The available Water Quality Volume (WQv) should be calculated as the volume of the basin up to the lowest proposed outlet elevation. In this regard, the available storage should be calculated only up to the top of frame of the proposed outlet structure. The data provided in the Stormwater Management Report indicates that the basin still provides adequate WQv even with the reduced available storage. The "Proposed Water Quality Volume (WQv)" column of the WQv table should be revised for accuracy.

Response: The Bioretention Basin Calculations sheet has been revised to indicate the correct depth of ponding of 1.37' between the revised outlet structure frame (661.87') and the revised bottom of the bioretention basin (660.50'). The WQV provided in the ponded depth is 5,079 CF. The updated Stormwater Quality Calculations can be seen in Appendix D of the supplementary Stormwater Management Narrative and Hydrologic Calculations letter included with this submission.

6. Test pits should be performed within the area of the proposed stormwater basin to determine if rock or high groundwater will conflict with the soil media and drainage layers below the proposed basin floor elevation.

Response: Test pits were performed on site on 5/4/2021. A total of three test pits, spaces 50' on center, were observed spanning the location of the proposed bioretention basin. Test pit locations have been added to the Land Development Plans included with this submission.

The test pits indicate that the site soils consist of dark brown sand within the upper 5' to 6' below existing grade, transitioning to sand with silt in below 6'. Groundwater was observed in all three pits ranging in depth from 7' to 8' below grade. Test pit 1 (TP-1) represents the highest potential groundwater table, with water located at elevation 654.50. To provide the 2' minimum required vertical separation from groundwater to

the bottom of the bioretention media, the bioretention basin elevation has been raised by 0.50'. The bottom of the ¾" stone layer is now proposed at elevation 656.50, and the bottom of the above-ground basin storage is now 660.50.

Falling head soil permeability testing was also performed for each test pit. Results of permeability testing ranged from 2.04 in/hr to 3.85 in/hr. The infiltration rate of 1 in/hr used in the hydrologic modeling conservatively represents the minimum field-measured rate of 2.04 in/hr reduced by half to accommodate a clogging factor of safety.

COMMUNITY DEVELOPMENT STAFF ANALYSIS – 2-LOT SUBDIVISION APPLICATION

1. PZC determined no Public Hearing would be held and was not required by CGS.

Response: Acknowledged.

2. A feasibility plan needs to be shown for lot #2 which would include proposed locations of a building, parking, and general site layout.

Response: A feasibility plan indicating potential locations of a building, parking and drive aisle areas, and other general site layout items has been provided in the Land Development Plans. Please refer to Master Plan sheet MP-1.

3. Once Street Numbers are assigned and approved by the Town, they should be shown on the Plan.

Response: Acknowledged, appropriate street numbers will be added to the plans upon approval.

4. Section 16A.3.x. – Buildings and Structures: Architectural and Design Requirements & Section 16B.4.1. – Architectural Character, Historic Preservation, Site Design. The Commission needs to determine if the design of the proposed building is adequate to meet these standards. If the Commission's intention along this corridor is to preserve the residential-type character and create transitions to existing residential neighborhoods, this proposal seems to accomplish that. Staff feels the applicant has paid particular attention to keeping all activity (no lighting, windows, etc.) away from the west side of the building to keep from interfering with the residences on North Road.

Response: Acknowledged, please note that additional brick banding and faux windows have been added to the northern side of the building in an effort to increase visual aesthetics while mitigating disturbance to the residential abutters. Please refer to the updated architectural elevations provided in the Land Development Plan set included with this submission.

5. Subdivision Regulations Section 4 – Open Space – The Commission needs to declare on the record that the conservation easement on the west side of the property satisfies the Open Space requirement of the Subdivision Regulations.

Response: Acknowledged.

COMMUNITY DEVELOPMENT STAFF ANALYSIS – SPECIAL PERMIT APPLICATION

The plans appear to meet Town Regulations. The following items may require additional information:

1. The PZC will hold a Public Hearing on May 5, 2021 as required by CGS.

Response: Acknowledged.

2. Once Street Numbers are assigned and approved by the Town, they should be shown on the Plan.

Response: Acknowledged, appropriate street numbers will be added to the plans upon approval.

3. The Town Engineer’s comments need to be addressed.

Response: The Town Engineer’s comments have been addressed, please refer to the Nathan L. Jacobson & Associates, Inc. portion of this response letter.

4. Section 16A.3.x. – Buildings and Structures: Architectural and Design Requirements & Section 16B.4.1. – Architectural Character, Historic Preservation, Site Design. The Commission needs to determine if the design of the proposed building is adequate to meet these standards. If the Commission’s intention along this corridor is to preserve the residential-type character and create transitions to existing residential neighborhoods, this proposal seems to accomplish that. Staff feels the applicant has paid particular attention to keeping all activity (no lighting, windows, etc.) away from the west side of the building to keep from interfering with the residences on North Road.

Response: Acknowledged, please note that additional brick banding and faux windows have been added to the northern side of the building in an effort to increase visual aesthetics while mitigating disturbance to the residential abutters. Please refer to the updated architectural elevations provided in the Land Development Plan set included with this submission.

5. Section 16A.4.d. – Notices – Statutory notices have been published on the town’s website, and the applicant has been provided with a sign for posting. The applicant has provided an affidavit for the posting of a sign.

Response: Acknowledged.

General Revision Notes

1. A conduit sleeve under the proposed development driveway, within the access and utility easement area, is now shown on the revised Site Utility Plan sheet SU-1. The conduit sleeve may be used for future sign conduit installation to Lot 2.
2. Additional brick banding and faux windows have been added to the northern side of the building in an effort to increase visual aesthetics while mitigating disturbance to the residential abutters. Please refer to the updated Conceptual Elevations sheet by BKA Architects dated May 3, 2021 provided in the Land Development Plan set included with this submission.
3. Screening vegetation to be planted between the proposed building and the residential abutters has been relocated to align with the top of the existing earthen berm, rather than being planted on the sloped portion of the berm, to maximize screening efficiency. Please refer to the updated Landscape Plan sheet LL-1.
4. The sanitary sewer service connection has been relocated to the force main along Boston Turnpike, greatly reducing the length of sanitary lateral, at the recommendation of the BLRWPCA. The Land Development Plans have been updated to indicate approximate connection invert to the main and required lateral kit installation. The plans also indicate the location of a proposed 20' wide sanitary sewer easement in favor of the BLRWPCA on the project parcel up to and including the sanitary sewer pump station. Please refer to the revised Site Utility Plan sheet SU-1.
5. The developer will provide a site work completion bond and sewer bond as required.
6. The property owner's signature will be provided on required applications upon receipt.
7. Responses to comments from the Eastern Highlands Health district received on 5/5/2021 are forthcoming.

We trust this addresses your concerns. Should you require additional information, please contact me at 860-760-1908.

Sincerely,



Kimberly M. Masiuk, P.E.
Senior Project Manager